Engineering Watershed Study Report

# Highway 236/Dennett Road Hydrologic Watershed Study

Town of Kittery, Maine

Prepared for

# **Town of Kittery**

200 Rogers Road Kittery, Maine 03904

August 2023



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# **Prepared by:**

Barton & Loguidice, LLC Portland, Maine 383 U.S. Route 1, Suite 2A Scarborough, ME 04074



Streamworks, PLLC PO Box 6578 31 Gorham Road Scarborough, ME 04070



**FB Environmental Associates** 97A Exchange Street, Suite 203 Portland, ME 04101



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#### List of Acronyms

**B&L:** Barton & Loguidice **BMPs: Best Management Practices** CFS: Cubic Feet per Second **DEM:** Digital Elevation Model FEMA: Federal Emergency Management Agency FIRM: Flood Insurance Rate Map gSURRGO: Gridded Soil Survey Geographic Database h:min Hours and Minutes HSG: Hydrologic Soil Group I-95: Interstate 95 Lbs.: Pounds MMW: Model My Watershed NCSD: National Soil Characterization Database NOAA: National Oceanic and Atmospheric Administration **Qpk:** Peak Discharge Rate Rt. 236: Maine State Route 236 Tc: Time of Concentration USDA: United States Department of Agriculture USEPA: United States Environmental Protection Agency

USGS: United States Geological Survey

#### **EXECUTIVE SUMMARY**

The Town of Kittery, Maine, progressed with an engineering study in the area bounded by Interstate 95 (I-95) to the south, Dennett Road to the west, Martin Road to the north and Highway 236 (Rt-236) to the east. The purpose of the study was to evaluate existing localized flood concerns, along with developing drainage and stormwater management considerations associated with potential future development in commercially zoned (Business Park and Rt-236 corridor) areas. The purpose of the study is to assess the existing causes of flood damage, the impact potential development would have on flood frequency and magnitude, and the development of alternatives to mitigate flood damage for existing and future conditions. In addition to minimizing the frequency and extent of localized flooding, the Town also aims to provide water quality improvements to natural resources within, and downstream of, the Town. Additionally, identification of potential future funding resources available to implement the recommended mitigation alternatives was developed.

Increased concern with stormwater quantity and quality within the Town has elevated the need for building community resiliency and protecting community assets from stormwater impacts. All concerned have the desire to mitigate the potential impacts of future storm events, minimize localized flooding, and provide water quality improvements to receiving waters. This Stormwater Management Study Report provides an overview of the site investigation and design process conducted by Barton & Loguidice, D.P.C. (B&L) and partners Streamworks and FB Environmental. Provided within is an existing conditions assessment including a summary of previous reports utilized and data collection activities, a stormwater system capacity evaluation (hydrologic and hydraulic modeling), a nutrient/pollutant loading evaluation, an evaluation of mitigation alternatives, an expanded analysis of six (6) potential water quality/flood mitigation projects, an evaluation of conservation/enhancement opportunities, and an evaluation of potential funding sources for implementation of the recommended projects.

Prior to the start of work, B&L reviewed previous studies and reports provided by the Town. Throughout the course of the project, additional information as described in the report was obtained from Town residents through a public outreach effort consisting of public meetings, community survey, and use of an interactive mapper where residents could identify drainage concerns and potential opportunity areas. The initial public meeting was held on December 15, 2022 and project update public meetings were held on March 22 and August 9, 2023. Community input received at each of these meetings were incorporated into the analysis and report. A final project presentation was held on \_\_\_\_\_\_, 2023 at a Town Board meeting.

Field data collection was used to develop a hydrologic and hydraulic model utilizing HydroCAD<sup>®</sup> that represents existing conditions to evaluate the stormwater system capacity and identify existing infrastructure elements within the community at risk for flood damage. Separate model runs were developed to evaluate potential build-out scenarios (50% and 100%) for non-residential zoned areas. Additionally, a pollutant load evaluation was conducted for the study area to evaluate phosphorus, nitrogen and sediment loads from subwatershed within the Study Area.

A retrofit opportunity matrix was developed to evaluate potential stormwater mitigation alternatives. The alternatives were based on information obtained from field data collection activities, hydrologic and hydraulic modeling scenarios, pollutant load model results, and public input. The potential alternatives comprise a wide range of practices for flood mitigation and water quality improvement. The projects were ranked based on criteria associated with stormwater benefits (quantity and quality), constructability, cost and co-benefits. The project advisory team utilized this matrix and recommendations from the B&L project team to select the six projects to progress to more detailed analysis.

The goal for selection of the six projects was to include a diverse collection of projects. The projects selected, therefore, were not necessarily ranked as the six highest overall scores. The projects were selected based on developing a diverse collection of potential projects ranging in scale on cost, location, and retrofit practice. The purpose was to utilize this matrix as a template that can be repeated by the Town to progress additional projects as future funding becomes available.

The projects selected for further evaluation include:

- 1. Right-Sizing Critical Infrastructure culvert and drainage system modifications
- 2. Seep Collars along Martin Road Sewer Line
- 3. Upstream Detention/Wetland Expansion (above Martin Rd.)
- 4. Identification of potential conservation areas (requires easements or land acquisition)
- 5. Expanding stormwater storage at "98 Dennett" Parcel
- 6. Providing Low Impact Development considerations for future Build-Out scenarios

This document provides an in-depth discussion and comparison of the aforementioned projects. Design considerations and cost estimates (based on 2023 dollars) for each project are also included for implementation as future funding becomes available.

#### 1.0 PROJECT BACKGROUND & HISTORY

# 1.1 Project Background

Recurrent flooding at residential properties in the Town of Kittery, Maine, in combination with potential development of the area north of Interstate 95 (I-95) between Highway 236 (Rt-236) and Dennett Road, has led to the Town's need for a hydrologic watershed study to assess the existing causes of flood damage, the impact potential development would have on flood frequency and magnitude, and the development of alternatives to mitigate flood damage for existing and future conditions. In addition to minimizing the frequency and extent of localized flooding, the Town aims to provide water quality improvements to natural resources within, and downstream of, the Town. The primary focus area for the study includes the area bounded I-95 to the south, Rt-236 to the east, Martin Road to the north, and Dennett Road to the west. This area includes an existing developed commercial highway (Rt-236) corridor, several residential neighborhoods for which development has occurred since the 1960's, and a large undeveloped tract of forested and wetland area located centrally within the study area. A topographic location map showing the general location of the study area is provided as **Figure 1-1**. An aerial map showing the study area, as well as the larger contributing drainage area which encompass the focus study area, is included as **Figure 1-2**.



Figure 1-1: General Study Area Location



Figure 1-2: Aerial Study Area Map

The primary goals of the study are to provide an existing conditions assessment, evaluate stormwater system capacity, understand the nature and potential causes of flooding in the study area, analyze nutrient/pollutant loading, and provide recommendations for flood mitigation projects or initiatives. Additionally, identification of potential future funding resources available to implement the recommended mitigation alternatives was developed.

# 1.2 Environmental Setting

The study area comprises six separate drainage areas. Each drainage area ultimately directs stormwater runoff to Spinney Creek to the west, the Piscataqua River to the southwest, or Spruce Creek to the southeast. Drainage areas vary in geologic conditions (*e.g.,* soil type, depth to bedrock, groundwater level, and slope). Soils are classified into hydrologic soil groups (HSG) to indicate the minimum rate of infiltration, or rate at which water enters the soil at soil surface, for bare soil after prolonged wetting. HSG's consist of Groups A, B, C, and D soils. In general, Group A soils have the lowest runoff potential and highest infiltration rates, whereas Group D soils have the highest runoff potential is the interaction between surface hydrology and groundwater. For example, even HSG A soils may have excessive runoff potential during instances of a high groundwater table. The interaction between depth of groundwater, surface hydrology, and resulting runoff potential will be further discussed as part of this study. Soil properties and qualities are summarized for each drainage area in **Table 1-1**. Soil mapping is included as **Figure 1-3** and **Figure 1-4** illustrating the variance in HSG and drainage classifications

between drainage areas. A majority of the study area (62%) consists of Group C/D soils, which exhibit higher runoff potential and lower infiltration rates.

Table	1-1:	Study	Area	Soils	Data
-------	------	-------	------	-------	------

Soil				Depth to Water	Acres in	Percent of
Symbol	Soil Unit Name	HSG	Drainage Class	Table (ft.)	Study Area	Study Area
	Adams-Urban land		U			
	complex, 0 to 8 percent		Somewhat			
AgB	slopes	A	excessively drained	6.6	5.8	1.7%
_	Biddeford mucky peat, 0					
Bm	to 3 percent slopes	D	Very poorly drained	0.0	33.8	9.8%
	Brayton and Westbury					
	very stony fine sandy					
	loams, 0 to 8 percent					
BsB	slopes	D	Poorly drained	0.5	4.4	1.3%
	Colton gravelly sandy					
	loam, 0 to 8 percent					
СоВ	slopes	A	Excessively drained	6.6	23	6.6%
	Colton gravelly sandy					
	loam, 15 to 25 percent					
CoD	slopes	A	Excessively drained	6.6	0.8	0.2%
	Croghan loamy fine sand,					
	0 to 8 percent slopes,		Moderately well			
CrB	wooded	A	drained	2.0	38.1	11.0%
	Lyman loam, 3 to 8		Somewhat			
LnB	percent slopes, rocky	D	excessively drained	6.6	41.1	11.9%
	Lyman loam, 8 to 15	_	Somewhat			
LnC	percent slopes, rocky	D	excessively drained	6.6	46.1	13.3%
	Lyman-Rock outcrop					
	complex, 3 to 8 percent		Somewhat		10.0	F 70/
ГЛЯ	slopes	D	excessively drained	6.6	19.8	5.7%
	Lyman-Rock outcrop		Company			
1	complex, 8 to 15 percent		Somewnat		0.0	2.00/
LYC	siopes	D	excessively drained	0.0	9.8	2.8%
	Lyman-Rock outcrop		Somowhat			
LVE	nercent slopes		somewhat	6.6	03	0.1%
LYL	Marlow fine sandy loam	D		0.0	0.5	0.176
MrB	3 to 8 percent slopes	6	Well drained	6.6	0.8	0.2%
	Marlow fine sandy loam	C	Weir dramed	0.0	0.0	0.270
MrC2	8 to 15 percent slopes	C	Well drained	6.6	19	0.6%
Na	Naumburg sand		Poorly drained	0.6	30	8.7%
Na Da	Naumburg sanu	AJD		0.0	30	8.7%
Pg	Pits, gravel	A	Excessively drained	6.6	22.8	6.6%
6.	Scantic silt loam, 0 to 3		De e ulu i due in e d	0.5	6.5	1.00/
SC	Charme fine conductors	D	Poorly drained	0.5	0.5	1.9%
CLD	skerry fine sandy loam, 0	C/D	drained	17	8.0	2.6%
SKR	to a percent slopes	C/D	Mederately well	1./	8.9	2.0%
Ur	Urban land	D	drained	4.0	15.8	4.6%
w	Water bodies	Unknown	Unknown	Unknown	30.6	8.8%



Figure 1-3: Hydrologic Soil Groups



Figure 1-4: Soil Drainage Class Designations

Within the focus area, higher elevations are generally found in the northern and central areas along Martin Road. Additionally, steeper slopes are more prominent in the western half of the focus study area, directing runoff west and southwest toward Spinney Creek and the Piscataqua River, as shown on **Figure 1-1**. Alternatively, the central, south-central, and eastern portions of the focused study area are characterized by more moderate slopes directing stormwater flows to the east and southeast toward Spruce Creek.

The hydrologic and hydraulic evaluation of the study area, as further discussed in Section 2.4 below, is comprised of multiple factors including land use, land cover types, soil classifications, topography and drainage routes. Land cover is critical to drainage characteristics within a watershed, and also exerts considerable influence on the chemical, physical, and biological characteristics of waterbodies. Land cover classifies the vegetation (or lack thereof) covering the ground. Removal of natural vegetation can reduce the ability to lessen runoff rates, prevent erosion and filter potential nutrients and pollutants. Lack of natural vegetative cover typically results in increased amounts of runoff and potential nutrient transport. Within the Watershed Study Area, land cover varies with population density, where more impervious cover types are generally located within closer proximity to the roadways along the focused study area boundaries, while more pervious cover, primarily forest, is generally located centrally within the study area.

The groundwater level varies greatly between soil groups and topography, and has a significant impact upon runoff potential. Groundwater fills the interstitial (void) space of soils, leaving less room for infiltration of runoff. Developed properties located in areas characterized by a high seasonal groundwater table are more susceptible to localized flooding due to a lack of infiltration capacity during precipitation events. Further, properties developed within areas characterized by high seasonal groundwater tables may be subject to instances of inundation in the absence of precipitation and stormwater runoff, and may require mitigation alternatives that differ from conventional stormwater best management practices (BMPs). Approximate groundwater levels, as defined by Soil Survey Database (gSSURGO, 2016), are included within **Figure 1-5**.



Figure 1-5: Depth to Water Table

A floodplain, by definition, is a nearly flat plain near a surface waterbody that is naturally subject to flooding during normal (2-year) to extreme (100-year) precipitation events. Floodplains generally contribute to localized flooding, however, offer much needed nutrient filtration and downstream flood minimization. Floodplains exist within the study area, originating mostly within Drainage Area 1 to the northeast. The 100-year floodplain boundaries are shown on the 1984 Federal Emergency Management Agency (FEMA) Flood Rate Insurance Maps (FIRMs) included as **Appendix A**.

# 1.3 Land Ownership

Lands located within the focus study area consist primarily of privately owned lands; therefore, potential mitigation alternatives to alleviate localized flooding will require cooperation and collaboration with current landowners. Private landowner buy-in and partnerships will likely be required to increase the potential for funding assistance from state and/or federal grant programs for implementation of potential stormwater improvement projects.

Identifying select parcels that have the greatest potential to maximize water quality improvements and flood reduction benefits can accommodate potential future build-out while also mitigating existing flooding issues. The following factors were reviewed to identify priority areas for implementation of mitigation alternatives:

- Watershed Area
- Location
- Existing Land Use
- Ownership
- Zoning
- Hydrologic Soil Group

Larger watershed areas were identified as more favorable than smaller areas with similar characteristics for certain mitigation opportunities, such as conservation of natural areas or implementation of large retrofit practices. These larger watershed areas have greater potential to treat a larger volume of water quality and quantity from stormwater runoff when compared to smaller areas: basically the economy of scale. The existing land use compared to the Town's zoning was also reviewed. Current zoning for the Town of Kittery is included as **Appendix B**. Areas that have higher present day flood attenuation (*e.g.*, wooded areas) that are zoned to allow increased impervious area (*e.g.*, commercial) were identified as potential for build-out of adjacent areas while minimizing an increase in stormwater runoff volumes and rates. Proximity to receiving waters and position within the watershed were reviewed as well. Areas that are directly adjacent to receiving waters were considered more favorable as potential mitigation or conservation areas.

Areas that are publicly owned (*e.g.*, owned by the Town, County or other government organization) serve the easiest to implement a conservation or enhancement opportunity; however, the focus study area is primarily composed of privately owned parcels. Privately owned parcels meeting the criteria for viable conservation areas may be considered as a part of this study as well based on the potential for land acquisition.

# 2.0 EXISTING FACILITIES & PRESENT CONDITION

#### 2.1 Field Data Collection and Survey

Issues arriving from localized flooding and associated water quality degradation may be mitigated after better understanding the root causes and influencing factors of flooding. A field survey was completed in November, 2022 by members of the Barton & Loguidice (B&L) and Streamworks teams to collect supplemental information to assess the causes of flooding in the study area. Prior to the field reconnaissance efforts, B&L conducted a desktop-based delineation of subwatersheds using publicly available LIDAR mapping, the Town's online GIS database, and other desktop resources used to identify drainage divides, buried drainage facilities, and common outlet points. A pre-field reconnaissance meeting was conducted with Town officials to further document existing areas of concern. The goals of the preliminary watershed reconnaissance included:

- Catalog location, type, and condition data for critical stormwater infrastructure not included in the Town's GIS database, as well as for confirmation of existing data for infrastructure located in priority areas;
- Collect detailed elevation data at features critical to the accurate hydrologic and hydraulic modeling of areas subject to recurrent inundation with stormwater or groundwater; and
- Confirm and refine subwatershed delineations.

Additionally, members of FB Environmental Associates conducted a preliminary wetland delineation reconnaissance effort in fall 2022 to better understand the extent and functionality of hydrologic resources within the primary focus area. Wetland boundaries were identified based on the United States Army Corps of Engineers' (1987) Wetland Delineation Manual and the 2012 Regional Supplement to the Corps of Engineers Wetland Delineation Manual: Northcentral and Northeast Region, Version 2.0. Approximate boundaries were geolocated using a Garmin GPSMAP 78 handheld GPS unit with an accuracy of approximately 30 feet. Although performed at the reconnaissance level, preliminary mapping of wetland resources within the focus area plays a critical role in the identification of potential conservation areas and features that may affect hydrologic and hydraulic modeling of the study area (e.g., storage of flood areas). A map of wetlands and watercourses identified by FB Environmental during their 2022 wetland reconnaissance efforts is included as **Appendix C**.

# 2.2 Public Engagement

In order to better understand existing conditions and to collect supplemental information from various stakeholders, a series of public engagement meetings were conducted. In addition to public engagement meetings, multiple resources were provided to residents of the Town of Kittery to provide additional information for review and consideration, including an interactive mapper and a public survey. The interactive mapper consisted of an open-source website in which stakeholders could select specific areas for identification of existing deficiencies and

potential enhancement opportunities. The results of the interactive mapper, including graphical summaries of responses, are provided in **Appendix D.** 



Figure 2-1: Interactive Mapper Interface



Additionally, the public survey afforded a means for public stakeholders to provide additional background information through a series of guided questions. The results of the public survey are included as **Appendix D.** 

General themes from input received through the public outreach included:

Interactive Mapper Summary:

- Intersection of Martin Rd./236 historical flooding, impacts of high groundwater table, beaver dams implications, culvert sizing considerations
- Herb Parsons Pond changes in water level over time
- Happy Avenue historical flooding
- Dennett Road historical flooding

# Public Survey Summary:

- 62% of respondents (based on 16 responses) have experienced flooding at their property
- Combination of seasonal and ongoing flooding, with most respondents identifying increased flooding during the Spring season
- Multiple respondents have incurred costs associated with flooding
- 2/3 of respondents suspect flooding is more groundwater than surface water related
- Respondents believe increased development, wetland alteration and modifications to natural drainage paths are primary factors to localized flooding.

# 2.3 Priority Areas

Prior to development of hydrologic and hydraulic models for the focus area, priority areas were identified through correspondence with Town officials, engagement of the public during public meetings, field reconnaissance, and evaluation of existing literature and desktop resources. The following areas were identified as priority areas:

- Residential properties along Martin Road, primarily within close proximity to Rt-236;
- Herb Parson Pond;
- Central study area (potential future development areas);
- Residential properties along Dennett Road;
- Happy Avenue access road;
- Residential properties along Old Dennett Road, located west of Dennett Road; and
- Ongoing Dennett Road development near I-95 on-ramp.

Priority areas listed above are identified on Figure 2-2 and are further discussed in Section 8.0.



Figure 2-2: Priority Areas

# 2.4 Hydrologic and Hydraulic Evaluation

A hydrologic and hydraulic model was developed using HydroCAD<sup>®</sup> software to identify areas of localized flooding and predict anticipated peak flows (runoff rates and volumes) during specific storm events, and to provide an existing conditions model that may be modified to evaluate potential mitigation alternatives.

The focus study area was separated into six distinct drainage areas (Drainage Areas 1, 2, 3, 4, 5, and 6). Each drainage catchment within the study area has variable slopes directing stormwater from the outer extents of each subcatchment toward one of six modeled outfalls to surrounding receiving water bodies. Generally, the steeper the slope, the shorter the time of concentration is, which produces a higher peak flow. Drainage Areas 1 through 5 were comprised of smaller subcatchments, some that included ponds and reaches. Ponds (represented as triangles in Figure 2-3) were utilized within the model to either represent areas which provide notable storage and attenuation of stormwater flows. Reaches were utilized to represent channelized flow (shown as squares in Figure 2-3). Modeling assumed reaches operate under free discharge conditions based on normal open channel flow.

The HydroCAD<sup>®</sup> model develops runoff hydrographs from rainfall. The runoff reflects watershed precipitation, topography, soil type, land cover, and land use. The following data and corresponding sources were utilized to calibrate the model to the Watershed Study Area:

- Precipitation Data:
  - The 1-, 2-, 10-, 25-, 50-, 100-, and 500-year events were identified using National Oceanic and Atmospheric Administration (NOAA) Atlas 14 rainfall data (see Appendix E).
- Topography:
  - Elevation maps were developed to calculate slopes and to approximate inverts and flood elevations using 2019 USGS 1 meter digital elevation model (DEM) data (USGS, 2022). DEM data was geoprocessed and converted to contours at 2-foot intervals. This mapping was also utilized to evaluate the time of concentration (Tc), which is the time required for runoff to travel from the hydraulically most distant point in the watershed to the design point. Tc is an important input into the model to determine runoff timing.
- Soil Type:
  - Soil types were retrieved from the United States Department of Agriculture (USDA) National Resources Conservation Service (NRCS) SSURGO dataset (USDA, 2016).
- Land Cover and Land Use:
  - Acreages were retrieved from the 2019 National Land Cover Database (Dewitz, 2021).

When modeled runoff hydrograph water elevations encroach on, or inundate, private property or infrastructure, that location represents a flood risk. Graphic figures representing potential areas where flooding may occur during specific storm events are included in **Figure 2-3**, **Figure 2-4**, and **Figure 2-5**. Depending on the location in the study area, the model predicted localized flooding anywhere from a 1-year storm to a 100-year storm events.

Prior to analysis of future development scenarios, an existing conditions model was developed to determine current vulnerable locations and stormwater infrastructure components. Each subwatershed was evaluated for peak discharge rate (Qpk), time of the peak discharge (hours:minutes), and total runoff volume. Each of these parameters plays a key role in evaluation of how a subwatershed responds to various precipitation and development scenarios. Results from the existing conditions model for each subwatershed is provided in **Table 2-1**, below with the modeling reports provided as **Appendix F.** 

Figure 2-3: Flood Risk Areas – 1-Year Storm

(To be inserted in final draft)

Figure 2-4: Flood Risk Areas – 10-Year Storm

(To be inserted in final draft)

Figure 2-5: Flood Risk Areas – 100-Year Storm

(To be inserted in final draft)

Sub-	Peak	Discharge Rate	(CFS)	Runoff Volume (acre-feet)			
watershed	1-year	10-year	100-year	1-year	10-year	100-year	
DA-1-1	18.87	59.93	111.40	3.059	10.13	19.79	
DA-1-2	14.2	44.8	82.96	2.065	6.792	13.21	
DA-1-3	6.46	17.58	31.01	0.705	2.083	3.891	
DA-1-4	17.05	43.96	75.92	2.153	6.04	11.09	
DA-2-1	18.56	59.88	112.80	4.657	15.62	30.94	
DA-2-2	4.95	17.23	33.91	1.188	4.257	10.09	
DA-2-3	6.28	18.23	32.94	0.864	2.676	5.094	
DA-2-4	9.49	26.74	48.00	1.076	3.24	6.126	
DA-3-1	2.4	8.38	16.91	0.399	1.462	3.080	
DA-3-2	5.12	20.07	44.42	0.673	2.759	6.451	
DA-4-1	1.69	6.31	13.04	0.327	1.258	3.026	
DA-4-2	3.99	12.57	23.56	0.698	2.304	4.629	
DA-5-1	3.69	12.47	23.60	0.613 2.126		4.278	
DA-5-2	14.16	39.66	70.89	1.213	3.64	6.860	
DA-6-1	0.58	1.52	2.63	0.077	0.223	0.411	

Table 2-1: Existing Condition Hydrologic and Hydraulic Model Results Summary

# 2.5 Modeled Nutrient and Sediment Loads

The modeling analysis for the study was completed using the Model My Watershed (MMW) tool, a web-based watershed modeling application that includes a Watershed Multi-Year Model. Model My Watershed provides a continuous simulation model that evaluates stormwater quality impacts using the Generalized Watershed Loading Function Enhanced (GWLF-E) model. The GWLF-E model was initially developed by Barry M. Evans, Ph.D., and his team at Penn State University for use with the MapShed desktop modeling application. The MMW Multi-Year Model utilizes regional geospatial data layers embedded within the program's web interface and provides estimated annual nutrient loadings based on 30 years of simulated water, nutrient, and sediment fluxes over a user defined Study Area.

The GWLF-E estimates external nutrient and sediment loads as a function of precipitation data, land cover, topography, soil type, soil nutrients, groundwater nitrogen, baseflow, animal farming operations, and wastewater inputs. Sources for each required dataset are as follows:

- Precipitation data: USEPA's National Climate Data (USEPA, 2006)
- Land cover: 2011 National Land Cover Database (Homer et al., 2015)
- Soil type: USDA-NRCS GSSURGO (USDA, 2016)

- Soil nitrogen: USDA National Soil Characterization Database (NSCD) (Hargrove and Luxmoore, 1998)
- Soil phosphorus: USGS (Smith et al. 2014)
- Groundwater nitrogen: USGS (Nolan and Hitt, 2006)
- Base flow: USGS (Wolock, 2003)
- Topography: National Elevation Dataset (USGS, 2009)
- Animal farming operations: USDA (USDA, 2012)
- Streams: Continental US Medium Resolution Stream Network (NHD Plus V2, 2017)

Additionally, MMW serves as a valuable tool for assessing the effectiveness of various alternatives as compared to an established baseline condition. The following Tables present the annual sediment, phosphorus and nitrogen loading results under existing conditions for each of the main drainage areas within the study area.

Table 2-2. Annual Sediment Loading Summary									
Drainage Area	Area (ac)	Sediment Loading RatesSediment LoadingRatesSediment LoadingArea (ac)(Ib./ac)		Percent of Total Sediment Loading (%)					
DA-1	95.2	29.13	2,771.3	42.5%					
DA-2	146.97	19.87	2,921.6	44.9%					
DA-3	35.98	12.74	459.5	7.1%					
DA-4	27.87	3.92	110.4	1.7%					
DA-5	23.57	9.11	231.8	3.6%					
DA-6	0.74	26.21	19.4	0.2%					
Total	330.3	_	6,514	100%					

	Table 2-3. Annual Phosphorus Loading Summary									
Drainage Area	Area (ac)	Phosphorus Loading Rates (Ib./ac)	Phosphorus Loading (lbs.)	Percent of Total Phosphorus Loading (%)						
DA-1	95.2	0.06	5.7	30.5%						
DA-2	146.97	0.05	8	42.7%						
DA-3	35.98	0.06	2.2	11.8%						
DA-4	27.87	0.05	1.4	7.5%						
DA-5	23.57	0.06	1.3	7%						
DA-6	0.74	0.09	0.1	0.5%						
Total	330.3	-	18.7	100%						

Table 2-4. Annual Nitrogen Loading Summary									
Drainage Area	Area (ac)	Nitrogen Loading Rates (Ib./ac)	Nitrogen Loading (lbs.)	Percent of Total Nitrogen Loading (%)					
DA-1	95.2	1.49	141.3	27.7%					
DA-2	146.97	1.51	221.3	43.3%					
DA-3	35.98	1.74	62.8	12.3%					
DA-4	27.87	1.51	42.7	8.4%					
DA-5	23.57	1.77	41.6	8.1%					
DA-6	0.74	1.43	1.1	0.2%					
Total	330.3	-	510.8	100%					

This data is utilized to inform priority locations for potential water quality retrofit projects and will be incorporated into the analysis of the priority projects further discussed in Section 6.0. The Model My Watershed Pollutant Loading summary reports are provided as **Appendix G**.

#### 3.0 BUILD-OUT ANALYSIS

In addition to evaluating existing conditions and alternatives to alleviate existing localized flooding and impacts of high groundwater tables, a primary goal of the project is to evaluate hydrologic impacts of potential future development. In order to better understand what portions of the focus area would be most susceptible to negative hydrologic impacts as a result of future development, as well as what mitigation alternatives would be most effective in managing these impacts, a build-out analysis was completed. This build-out analysis consisted of development of HydroCAD models for three build-out alternatives: 1) No Build Alternative; 2) 50% Build-Out scenario; and 3) 100% Build-Out scenario. Existing zoning within the focus area is shown in **Figure 3-1** below.



Figure 3-1: Existing Zoning within Focus Area

The no-build alternative consisted of a simplification of the existing conditions HydroCAD model. The existing conditions model described in Section 2.4 utilized detailed land cover data provided through the 2019 NLCD. For land parcels located within areas zoned as Business Park (B-PK) or Commercial 2 (C-2), the no-build alternative was developed through designation of parcels as either build-out, or not built-out, in accordance with existing zoning regulations. For example, for areas located along Rt-236 that are zoned for commercial use, parcels containing commercial properties were identified as built-out, while forested parcels were designated as not built-out. Parcels containing mixed land use (i.e., half commercial and half forested) were bisected, and each sub-parcel was given the appropriate

designation. Following designation of each parcel and sub-parcel with the appropriate aggregate land use, associated runoff curve numbers were assigned to each parcel area. Simplification of parcel land uses for build-out analysis was not completed for areas zoned as Residential Suburban (R-S) as substantial build out is not anticipated in these areas. Therefore, existing NLCD data was retained in HydroCAD models for R-S zoned areas under each build out scenario. A visual representation of the No-Build modeling approach is shown in **Figure 3-2** below.



Figure 3-2: No-Build Model Scenario (To be revised omitting zone R-S parcels in final draft)

Following development of a HydroCAD model representing the no-build alternative, designated land use was modified for development of the build-out scenario models. This process consisted of converting 50% of the total area as not built-out to the built-out designation, and assigning the corresponding runoff curve numbers to the new area totals for each land use. For example, within the area zoned as Business Park in the southern extent of the focus area, 50% of the aggregate wooded area would be converted to commercial.

The final build-out scenario model, representing the 100% build-out scenario, was developed by converting all areas designated as not 'built-out' to the maximum built-out land use allowable by existing zoning. For example, in the eastern extent of the focus area currently zoned as 'Commercial 2 (C-2)', all areas designated as wooded, residential, etc. would be converted to commercial lands in the HydroCAD model and assigned the appropriate runoff curve number.

Results from the modeling efforts for each build-out scenario are summarized in the following Tables 3-1, 2, 3 and 4. The Build-Out scenario HydroCAD modeling summary reports are provided in **Appendices H (50% Build-Out) and I (100% Build-Out).** 

		1-Year event		10-Year event			1	LOO-Year even	ıt
	Qpk - No Build (CFS)	Q-pk - 50% Build (CFS)	% Increase from No Build	Qpk - No Build (CFS)	Q-pk - 50% Build (CFS)	% Increase from No Build	Qpk - No Build (CFS)	Q-pk - 50% Build (CFS)	% Increase from No Build
DA-1-1	18.87	18.87	0.0%	59.93	59.93	0.0%	111.4	111.4	0.0%
DA-1-2	13.31	14.39	8.1%	39.53	40.72	3.0%	71.87	72.79	1.3%
DA-1-3 POND	0.52	0.57	9.6%	2.23	2.28	2.2%	12.76	11.59	-9.2%
DA-1-3	22.23	25.23	13.5%	52.87	56.41	6.7%	89.6	92.97	3.8%
DA-1-4	18.5	22.05	19.2%	45.11	49.46	9.6%	77.14	81.38	5.5%
DA-2-1	17.96	18.85	5.0%	57.79	59.98	3.8%	108.81	110.03	1.1%
DA-2-2	4.74	7.38	55.7%	15.92	31.62	98.6%	31.1	67.76	117.9%
DA-2 POND	3.18	3.75	17.9%	10.06	24.03	138.9%	97.78	144.5	47.8%
DA-2-3	4.55	6.88	51.2%	13.4	16.4	22.4%	24.46	27.46	12.3%
DA-2-4	7.03	9.99	42.1%	19.48	23.32	19.7%	34.88	38.81	11.3%
DA-3-1	2.4	2.4	0.0%	8.38	8.38	0.0%	16.91	16.91	0.0%
DA-3-2	5.12	5.12	0.0%	20.07	20.07	0.0%	44.42	44.42	0.0%
DA-4-1	1.68	2.73	62.5%	6.02	10.15	68.6%	12.13	21.01	73.2%
DA-4-2	3.29	5.27	60.2%	10.15	14.34	41.3%	18.84	25.82	37.0%
DA-5-1	4.74	7.06	48.9%	13.75	17.67	28.5%	25.02	30.53	22.0%
DA-5-2	23.98	25.53	6.5%	49.83	51.79	3.9%	80.16	82.11	2.4%
DA-6-1	0.13	0.13	0.0%	0.26	0.26	0.0%	0.41	0.41	0.0%

Table 3-1: Peak	Discharge	Rates - 50%	6 Build	Out Scenario
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		1-Year event			10-Year event	:	100-Year event		
	Volume - No Build (ac-ft.)	Volume - 50% Build (ac-ft.)	% Increase from No Build	Volume - No Build (ac-ft.)	Volume - 50% Build (ac-ft.)	% Increase from No Build	Volume - No Build (CFS)	Volume - 50% Build (ac-ft.)	% Increase from No Build
DA-1-1	3.059	3.059	0.0%	10.129	10.129	0.0%	19.791	19.719	-0.4%
DA-1-2	1.914	2.052	7.2%	6.018	6.2	3.0%	11.516	11.653	1.2%
DA-1-3 POND	0.81	0.896	10.6%	3.658	3.784	3.4%	7.669	7.803	1.7%
DA-1-3	2.442	2.72	11.4%	6.442	6.777	5.2%	11.522	11.751	2.0%
DA-1-4	2.139	2.488	16.3%	5.756	6.233	8.3%	10.406	10.807	3.9%
DA-2-1	4.502	4.694	4.3%	15.075	15.387	2.1%	29.866	30.197	1.1%
DA-2-2	1.127	2.028	79.9%	3.944	8.122	105.9%	9.371	17.75	89.4%
DA-2 POND	4.846	5.717	18.0%	15.098	19.259	27.6%	34.722	43.378	24.9%
DA-2-3	0.639	0.911	42.6%	1.982	2.419	22.0%	3.791	4.276	12.8%
DA-2-4	0.803	1.084	35.0%	2.376	2.822	18.8%	4.47	4.956	10.9%
DA-3-1	0.399	0.399	0.0%	1.462	1.462	0.0%	3.08	3.075	-0.2%
DA-3-2	0.673	0.673	0.0%	2.759	2.758	0.0%	6.451	6.438	-0.2%
DA-4-1	0.319	0.554	73.7%	1.187	2.047	72.5%	2.865	4.482	56.4%
DA-4-2	0.578	0.909	57.3%	1.864	2.665	43.0%	3.729	4.992	33.9%
DA-5-1	0.778	1.118	43.7%	2.388	3.075	28.8%	4.612	5.57	20.8%
DA-5-2	2.203	2.117	-3.9%	4.804	4.851	1.0%	8.266	8.094	-2.1%
DA-6-1	0.017	0.18	958.8%	0.04	0.04	0.0%	0.067	0.066	-1.5%

Table 3-2: Runoff Volumes – 50% Build Out Scenario

		1-Year event			10-Year event	t 100-Year event			
	Qpk - No Build (CFS)	Q-pk - 100% Build (CFS)	% Increase from No Build	Qpk - No Build (CFS)	Q-pk - 100% Build (CFS)	% Increase from No Build	Qpk - No Build (CFS)	Q-pk - 100% Build (CFS)	% Increase from No Build
DA-1-1	18.87	18.87	0.0%	59.93	59.93	0.0%	111.4	111.4	0.0%
DA-1-2	13.31	14.7	10.4%	39.53	40.78	3.2%	71.87	72.89	1.4%
DA-1-3 POND	0.52	0.59	13.5%	2.23	2.29	2.7%	12.76	14.93	17.0%
DA-1-3	22.23	28.51	28.3%	52.87	59.18	11.9%	89.6	95.18	6.2%
DA-1-4	18.5	25.95	40.3%	45.11	53.04	17.6%	77.14	84.39	9.4%
DA-2-1	17.96	19.89	10.7%	57.79	59.94	3.7%	108.81	110.82	1.8%
DA-2-2	4.74	30.88	551.5%	15.92	63.87	301.2%	31.1	102.76	230.4%
DA-2 POND	3.18	6.43	102.2%	10.06	72.62	621.9%	97.78	195.65	100.1%
DA-2-3	4.55	9.58	110.5%	13.4	18.78	40.1%	24.46	29.39	20.2%
DA-2-4	7.03	13.49	91.9%	19.48	26.44	35.7%	34.88	41.38	18.6%
DA-3-1	2.4	2.4	0.0%	8.38	8.38	0.0%	16.91	16.91	0.0%
DA-3-2	5.12	5.12	0.0%	20.07	20.07	0.0%	44.42	44.42	0.0%
DA-4-1	1.68	8.63	413.7%	6.02	17.92	197.7%	12.13	29.28	141.4%
DA-4-2	3.29	10.32	213.7%	10.15	20.29	99.9%	18.84	31.79	68.7%
DA-5-1	4.74	11.31	138.6%	13.75	22.27	62.0%	25.02	34.91	39.5%
DA-5-2	23.98	27.3	13.8%	49.83	53.33	7.0%	80.16	83.36	4.0%
DA-6-1	0.13	0.13	0.0%	0.26	0.26	0.0%	0.41	0.41	0.0%

Table 3-3: Peak Discharge Rates – 100% Build Out Scenario

		1-Year event		:	10-Year event	:	1	00-Year even	t
	Volume - No Build (ac-ft.)	Volume - 100% Build (ac- ft.)	% Increase from No Build	Volume - No Build (ac-ft.)	Volume - 100% Build (ac- ft.)	% Increase from No Build	Volume - No Build (CFS)	Volume - 100% Build (ac- ft.)	% Increase from No Build
DA-1-1	3.059	3.059	0.0%	10.129	10.129	0.0%	19.791	19.719	-0.4%
DA-1-2	1.914	2.114	10.4%	6.018	6.239	3.7%	11.516	11.67	1.3%
DA-1-3 POND	0.81	0.936	15.6%	3.658	3.811	4.2%	7.669	7.82	2.0%
DA-1-3	2.442	3.095	26.7%	6.442	7.199	11.8%	11.522	12.153	5.5%
DA-1-4	2.139	2.942	37.5%	5.756	6.78	17.8%	10.406	11.349	9.1%
DA-2-1	4.502	4.961	10.2%	15.075	15.71	4.2%	29.866	30.519	2.2%
DA-2-2	1.127	7.046	525.2%	3.944	16.304	313.4%	9.371	27.783	196.5%
DA-2 POND	4.846	10.076	107.9%	15.098	27.657	83.2%	34.722	53.714	54.7%
DA-2-3	0.639	1.288	101.6%	1.982	2.857	44.1%	3.791	4.703	24.1%
DA-2-4	0.803	1.481	84.4%	2.376	3.288	38.4%	4.47	5.412	21.1%
DA-3-1	0.399	0.399	0.0%	1.462	1.461	-0.1%	3.08	3.075	-0.2%
DA-3-2	0.673	0.673	0.0%	2.759	2.758	0.0%	6.451	6.438	-0.2%
DA-4-1	0.319	1.538	382.1%	1.187	3.597	203.0%	2.865	6.344	121.4%
DA-4-2	0.578	1.733	199.8%	1.864	3.851	106.6%	3.729	6.342	70.1%
DA-5-1	0.778	1.785	129.4%	2.388	3.974	66.4%	4.612	6.55	42.0%
DA-5-2	2.203	2.269	3.0%	4.804	5.026	4.6%	8.266	8.264	0.0%
DA-6-1	0.017	0.018	5.9%	0.04	0.04	0.0%	0.067	0.066	-1.5%

Table 3-4: Runoff Volumes – 100% Build Out Scenario

#### 4.0 EXTREME PRECIPITATION SCENARIOS

Climate change will only exacerbate existing conditions, imparting more extreme weather events resulting in an increase of total stormwater runoff and peak flow. In turn, this will amplify existing instances of localized flooding, provide higher-than normal seasonal groundwater tables, and further impact the effects of future development of currently undeveloped areas. Additionally, an increase in the frequency and magnitude of extreme storm events will drive the transport of increasingly excessive amounts of nutrients and sediment to receiving waterbodies and resulting in degradation of water quality within, and downstream of, the focus area. In order to model extreme precipitation scenarios, the precipitation amounts were increased by 15% for each storm event. Tables 4-1 and 4-2 indicate the changes that may be anticipated with increased rainfall amounts and intensities. A figure identifying the potential flooding impacts associated with extreme precipitation scenarios is provided below {to be developed} and the Extreme Storm Event HydroCAD modeling reports are provided in **Appendix J**.

	1-Year				10-year					
	Qpk - No Build (CFS)	Q-pk - 50% Build (CFS)	% Increase from No Build	Q-pk - 100% Build (CFS)	% Increase from No Build	Qpk - No Build (CFS)	Q-pk - 50% Build (CFS)	% Increase from No Build	Q-pk - 100% Build (CFS)	% Increase from No Build
DA-1-1	18.87	22.69	20.2%	18.87	0.0%	59.93	59.93	0.0%	59.93	0.0%
DA-1-2	13.31	16.74	25.8%	14.7	10.4%	39.53	40.72	3.0%	40.78	3.2%
DA-1-3 POND	0.52	0.79	51.9%	0.59	13.5%	2.23	2.28	2.2%	2.3	3.1%
DA-1-3	22.23	27.54	23.9%	28.51	28.3%	52.87	56.41	6.7%	59.18	11.9%
DA-1-4	18.5	24.11	30.3%	25.95	40.3%	45.11	49.46	9.6%	53.04	17.6%
DA-2-1	17.96	22.65	26.1%	19.89	10.7%	57.79	58.98	2.1%	59.94	3.7%
DA-2-2	4.74	9.3	96.2%	30.88	551.5%	15.92	31.62	98.6%	63.87	301.2%
DA-2 POND	3.18	4.82	51.6%	6.43	102.2%	10.06	24.03	138.9%	73.57	631.3%
DA-2-3	4.55	7.66	68.4%	9.58	110.5%	13.4	16.4	22.4%	18.78	40.1%
DA-2-4	7.03	11.04	57.0%	13.49	91.9%	19.48	23.32	19.7%	26.44	35.7%
DA-3-1	2.4	2.93	22.1%	2.4	0.0%	8.38	8.38	0.0%	8.38	0.0%
DA-3-2	5.12	6.21	21.3%	5.12	0.0%	20.07	20.07	0.0%	20.07	0.0%
DA-4-1	1.68	3.31	97.0%	8.63	413.7%	6.02	10.15	68.6%	17.92	197.7%
DA-4-2	3.29	6.01	82.7%	10.32	213.7%	10.15	14.34	41.3%	20.29	99.9%
DA-5-1	4.74	7.92	67.1%	11.31	138.6%	13.75	17.67	28.5%	22.27	62.0%
DA-5-2	23.98	27.18	13.3%	27.3	13.8%	49.83	51.79	3.9%	53.33	7.0%
DA-6-1	0.13	0.14	7.7%	0.13	0.0%	0.26	0.26	0.0%	0.26	0.0%

	1-Year				10-year					
	Qpk - No Build (CFS)	Volume - 50% Build (ac-ft.)	% Increase from No Build	Volume - 100% Build (ac-ft.)	% Increase from No Build	Qpk - No Build (CFS)	Volume - 50% Build (ac-ft.)	% Increase from No Build	Volume - 100% Build (ac-ft.)	% Increase from No Build
DA-1-1	18.87	22.69	20.2%	18.87	0.0%	59.93	59.93	0.0%	59.93	0.0%
DA-1-2	13.31	16.74	25.8%	14.7	10.4%	39.53	40.72	3.0%	40.78	3.2%
DA-1-3 POND	0.81	1.237	52.7%	0.941	16.2%	3.658	3.784	3.4%	3.848	5.2%
DA-1-3	22.23	27.54	23.9%	28.51	28.3%	52.87	56.41	6.7%	59.18	11.9%
DA-1-4	18.5	24.11	30.3%	25.95	40.3%	45.11	49.46	9.6%	53.04	17.6%
DA-2-1	17.96	22.65	26.1%	19.89	10.7%	57.79	58.98	2.1%	59.94	3.7%
DA-2-2	4.74	9.3	96.2%	30.88	551.5%	15.92	31.62	98.6%	63.87	301.2%
DA-2 POND	4.846	7.391	52.5%	10.108	108.6%	15.098	19.259	27.6%	28.009	85.5%
DA-2-3	4.55	7.66	68.4%	9.58	110.5%	13.4	16.4	22.4%	18.78	40.1%
DA-2-4	7.03	11.04	57.0%	13.49	91.9%	19.48	23.32	19.7%	26.44	35.7%
DA-3-1	2.4	2.93	22.1%	2.4	0.0%	8.38	8.38	0.0%	8.38	0.0%
DA-3-2	5.12	6.21	21.3%	5.12	0.0%	20.07	20.07	0.0%	20.07	0.0%
DA-4-1	1.68	3.31	97.0%	8.63	413.7%	6.02	10.15	68.6%	17.92	197.7%
DA-4-2	3.29	6.01	82.7%	10.32	213.7%	10.15	14.34	41.3%	20.29	99.9%
DA-5-1	4.74	7.92	67.1%	11.31	138.6%	13.75	17.67	28.5%	22.27	62.0%
DA-5-2	23.98	27.18	13.3%	27.3	13.8%	49.83	51.79	3.9%	53.33	7.0%
DA-6-1	0.13	0.14	7.7%	0.13	0.0%	0.26	0.26	0.0%	0.26	0.0%

Table 4-2: Peak	<b>Discharge Rates</b>	– Extreme F	Precipitation	<b>Scenarios</b>
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# 5.0 CRITICAL INFRASTRUCURE CULVERT ANALYSIS

B&L assessed the capacity of 14 critical culvert locations using the results on the hydrologic models for the No Build Out, 50% Build Out and 100% Build Out scenarios. The culvert analysis focused on the headwater to depth ratios (HW/D) of culverts. Common hydraulic guidance used by many DOT agencies allows for a HW/D ratio up to 1.5 times the depth (or diameter) of culverts at peak discharge conditions. Higher ratios can commonly cause issues with water piping through the road subbase, potentially causing damage to embankments. B&L assumed culverts at state-owned road crossings need to be designed to safely convey the 50-year return storm peak discharge in accordance with Maine DOT design criteria. For local and county roads, B&L assumed a safe conveyance of a 10-year return storm peak discharge is adequate for many of crossing locations. Note that HW/D ratios outside design recommendations do not necessarily mean that local flooding occurs at the design storm event.

Based on the model analysis, B&L found that 8 of 14 culverts assessed are presently undersized as summarized in **Table 5-1 and on Figure 5-1**. Potential land development activities may exacerbate hydraulic conditions, but do not cause any additional culverts to exceed HW/D design recommendations in either the 50% or 100% scenarios. Preliminary culvert sizing recommendations are presented in **Table #** to meet HW/D ratio criteria for the 100% Build Out model scenario. We recommend conducting site-specific hydraulic analyzes to size replacement culverts so that other design criteria may be considered as necessary.

Table 5-1: Critical Infrastructure Culvert Analysis							
	HW/D Ratio Per Scenario @ Design Storm Peak						
		Existing	Discharge				
		Culvert Size			100% Build		
Model Node ID	Design Storm	(in dia.)	No Build Out	50% Build Out	Out		
1-P	10-Year	12	1.26	1.30	1.32		
2-P	10-Year	18	1.63	1.69	1.81		
CB-1-1*	10-Year	14	>2.0	>2.0	>2.0		
CB-1-3	10-Year	36	2.53	2.63	2.71		
CB-1-4	10-Year	24	8.74	10.61	12.28		
CB-2-3	50-Year	36	1.80	3.81	8.77		
CB-2-4	50-year	24	2.63	3.18	3.63		
CB-3-1**	10-Year	14	1.13	1.13	1.13		
CB-3-2	10-Year	24	1.84	1.84	1.84		
CB-4-1	10-Year	24	0.54	0.73	1.20		
CB-4-2	10-Year	24	0.73	0.94	1.40		
CB-5-1	10-Year	36	0.48	0.55	0.64		
CB-5-2	50-Year	32	3.71	4.06	4.38		
CB-6-1	10-Year	18	0.15	0.15	0.15		

\*Additional evaluation required to account for potential upstream storage.

\*\* Multiple culverts modeled at this node location. Largest diameter evaluated for hydraulic capacity. Locations in **BOLD** exceed a 1.5 HW/D ratio and additional hydraulic analysis may be warranted.

Table 5-2: Critical Infrastructure Culvert Analysis – Initial Sizing Considerations							
	Existing Culvert Size (in	<b>Recommended Culvert</b>	Preliminary HW/D				
Model Node ID	dia.)	Size (in dia.)	Ratio				
2-P	18	24	1.34				
CB-1-1	14	TBD	TBD				
CB-1-3	36	48	1.31				
CB-1-4	24	42	1.10				
CB-2-3	36	60	1.13				
CB-2-4	24	36	1.18				
CB-3-2	24	30	0.93				
CB-5-2	32	42	1.50				

Table 5-2 below provides initial sizing considerations to reduce the HW/D ratio to less than 1.5.



Figure 5-1: Culvert Sizing Analysis Locations
#### 6.0 ALTERNATIVES ANALYSIS

A primary goal of the project was to determine what projects have the greatest buy-in from both the Town and the general public, particularly due to public ownership of the majority of lands within the study area. A conceptual overview of the alternative selection process is described in detail below.

### 6.1 Alternative Matrix and Overview

A retrofit opportunity matrix was developed to evaluate potential stormwater mitigation alternatives based on information obtained from prior studies and field data collection activities. Potential alternatives include:

- Modifications to Existing Drainage System this practice focuses on right-sizing, repairing, or otherwise improving under-performing stormwater infrastructure, such as culverts or closed drainage systems (including inlets to such systems). These drainage system deficiencies are primarily identified through known failures or review of hydrologic and hydraulic modeling results. Examples include culvert replacements, drainage system modifications, inlet/outlet modifications, and daylighting of closed drainage systems (conversion to open channels).
- Installation of Stormwater Retrofits this practice focuses on providing localized storage to a drainage area to allow either detention and sedimentation or retention and infiltration, reducing downstream peak flows, runoff volume, total nutrients, and sediment loads. Examples include upstream stormwater/wetland retention facilities and stream restorations that provide connectivity to flood storage areas.
- Flood Damage Protection and Planning this practice focuses on localized improvements to structures located within areas with high frequencies of localized flooding or groundwater-related inundation. This practice may be completed at the individual homeowner level, or on a larger scale for protection of infrastructure. Examples range from installation of homeowner sump pumps, reduction of impervious areas, installing/maintaining homeowner drainage (gutters and downspouts), improving residential lot grading, backflow prevention, elevating utilities, installation of flood vents, and use of flood resistant materials.
- Groundwater Interception Trenches This practice focuses on interception and redirection of groundwater flows to avoid inundation of infrastructure located in areas characterized by high seasonal groundwater tables.
- Evaluation of Land Use Planning and Zoning This practice includes evaluation of existing policies and zoning areas to protect areas vulnerable to stormwater or groundwater related damages under existing conditions, as well as to avoid future development in areas viewed as vulnerable to the hydrologic effects of build-out where tangible mitigation practices are less feasible. Examples include incorporation of "Low Impact Development" considerations.

- Water Quality Treatment Practices and Green Infrastructure Opportunities this practice focuses on capture and treatment of stormwater at, or near, its source, including stormwater management while providing recreational and wildlife benefits. Generally, peak flows are reduced while providing nutrient treatment. Alternatives will also evaluate opportunities to replace existing or proposed impervious areas with permeable options that capture and infiltrate stormwater runoff. These techniques require careful evaluation of subsurface conditions (soils, depth to groundwater and bedrock) to ensure infiltration is feasible and will not contribute to localized groundwater impacts (i.e. increased basement flooding). Examples include:
  - Permeable pavements
  - Bioswales/vegetative swales
  - Rain gardens
  - Silva Cells<sup>®</sup>
  - Stormwater Tree Trenches
  - Infiltration basins
  - Rainwater harvesting
  - Rain barrels
  - Native plantings
  - Rooftop disconnects
- Conservation Opportunities this practice focuses on identification of areas best suited for conservation to assist with the off-set of future Build-Out conditions. Provisions of conservation areas can provide for a path for future development that minimize hydrologic impacts.
- Wetland creation/expansion this practice focuses on utilizing existing or suitable areas to create suitable vegetated areas typically influenced by groundwater that provide extended detention storage to treat significant water quality and reduce peak flows.
- Riparian buffer restoration this practice focuses on restoring the naturally vegetated areas that serve as the transition zone between terrestrial (land) and aquatic (water) habitats. If sufficiently structured, protected, and maintained, riparian buffers serve to mitigate the volume and intensity of stormwater runoff entering the adjacent waterbody, and can act to mitigate the discharge of pollutants to the waterway often associated with stormwater runoff.

The projects were ranked based on criteria associated with stormwater benefits (quantity and quality), constructability, cost and co-benefits. The rankings were based on the following criteria with total available points for each criterion in parentheses. **Appendix K** includes the detailed ranking matrix).

- Stormwater Benefits (total 50 out of 100 points)
  - Water Quantity Flood Reduction (40 points)
  - TSS reduction (5 points)
  - Nutrient: phosphorus and sediment reduction (5 points)
- Constructability (total 20 out of 100 points)
  - Land Acquisition/Public Ownership Potential (10 points)
  - Known constraints (5 points)
  - Permitting (5 points)
- Cost (total 20 out of 100 points)
  - Construction Cost not included in ranking as we are seeking a range of projects
  - Maintenance Cost (5 points)
  - Fundability (15 points)
- Co-Benefits (total 10 out of 100 points)
  - Energy and air quality impacts (2 points)
  - Habitat and biodiversity (2 points)
  - Community and aesthetic benefits (2 points)
  - Human health benefits (2 points)
  - Educational Opportunities/Visibility (2 points)

#### 6.2 Selection of Prioritized Alternatives

Based on the matrix rankings, the project advisory team selected six projects to progress to a more detailed analysis. The goal for selection of the six projects is to include a diverse collection of projects. The projects selected for more detailed analysis were not necessarily ranked based on the six highest overall scores. For example, the project advisory team determined that certain high-ranking projects have greater barriers to implementation.

The ranking matrix can be used as a template that can be repeated by the Town to progress additional projects as future funding becomes available.

The projects selected for further evaluation included options to address current conditions and potential Build-Out scenarios as follows:

#### Projects to Address Existing Flood & Water Quality Concerns:

- 1. Right-Sizing Critical Infrastructure culvert and drainage system modifications
  - Culvert outlet from existing pond/wetland complex in "98 Dennett Road Parcel"
  - East side of Martin Road
  - North end of Rt. 236
  - South end of Rt. 236

- I-95 drainage infrastructure (multiple locations)
- Intersection of Dennett and Old Dennett
- 2. Seep Collars along Martin Road Sewer Line
- 3. Upstream Detention/Wetland Expansion (above Martin Rd.)

Projects to Address Potential Build-Out Drainage Concerns:

4. Identification of potential conservation areas (requires easements or land acquisition)

- 5. Expanding stormwater storage at "98 Dennett" Parcel
- 6. Providing Low Impact Development considerations for future Build-Out scenarios
  - Synergy with updates to the Town's Stormwater Management Plan

The project locations selected to progress to more detailed analysis are provided on **Figure 6.1 (to be developed)**.

### 6.3 Evaluation of Alternatives {TO BE DEVELOPED FOLLOWING SELECTION OF PREFERRED ALTERNATIVES}

The six projects that progressed to supplemental analysis were incorporated into the hydrologic/hydraulic and pollutant load models to identify a quantifiable benefit of each alternative. Each alternative was modeled individually and combined to evaluate the cumulative benefit of multiple mitigation approaches. For the six prioritized alternatives, supplemental information will be provided including:

- Hydrologic/Hydraulic and Pollutant Load modeling analysis
- Cost estimates
- Non-Monetary Factors
  - Energy and Air Quality Impacts,
  - Habitat and Biodiversity,
  - Community and Aesthetic Benefits,
  - Human Health Benefits,
  - Educational Opportunities and Visibility,
  - Land Ownership, and
  - Regulatory Considerations and Permits Required.

Tables to be developed following Public Meeting 3 and final selection of alternatives.

Table 6-1: Water Quantity and Quality Benefits								
Selected Alternatives	Peak Flow Reduction (cfs)	Stormwater Volume Reduction (acre-feet)	Sediment Reduction (lbs.)	Phosphorus Reduction (lbs.)	Other			
PROJECT 1								
PROJECT 2								
PROJECT 3								
PROJECT 4								
PROJECT 5								
PROJECT 6								

Table 6-2: Implementation Cost Estimate										
Selected AlternativesFinal DesignPermittingLandTotal (w/20%ConstructionConstructionContingency										
PROJECT 1										
PROJECT 2										
PROJECT 3										
PROJECT 4										
PROJECT 5										
PROJECT 6										

\*Based on 2023 dollars

Table 6-3: Maintenance Considerations						
Selected Alternatives	Maintenance Needs	Avg. Annual Cost				
PROJECT 1						
PROJECT 2						
PROJECT 3						
PROJECT 4						
PROJECT 5						
PROJECT 6						

\*Based on 2023 dollars

Table 6-4: Non-Monetary Factors									
Energy and Air QualityCommunityHumanEducationalAir QualityHabitat & Biodiversity& AestheticHealthOpportunities/Selected AlternativesImpactsBiodiversityBenefitsBenefitsVisibility									
PROJECT 1									
PROJECT 2									
PROJECT 3									
PROJECT 4									
PROJECT 5									
PROJECT 6									

Table 6-5: Permitting Considerations								
Selected Alternatives Permit Name Permit Name Permit Name Permit Name								
PROJECT 1								
PROJECT 2								
PROJECT 3								
PROJECT 4								
PROJECT 5								
PROJECT 6								

### 6.4 Summary & Comparison of Alternatives

This section will be completed following Public Participation Meeting #3

### 6.5 Recommended Alternatives

This section will be completed following Public Participation Meeting #3

# 7.0 FUNDING OPPORTUNITIES {TO BE DEVELOPED FOLLOWING SELECTION OF PREFERRED ALTERNATIVES}

The table below will be populated to summarize programs offered through state and federal resources that may provide funding opportunities and assistance to support implementation of the projects.

Table 7-1: Potential Funding and Assistance Opportunities							
Selected Alternatives	Funding Source 1	Funding Source 2	Funding Source 3	Funding Source 4			
PROJECT 1							
PROJECT 2							
PROJECT 3							
PROJECT 4							
PROJECT 5							
PROJECT 6							

#### 8.0 PRIORITY AREA FINDINGS

Throughout the course of the study, priority areas were identified through field reconnaissance, evaluation of modeling results, and receipt of public input. The following is a summary of findings associated with these priority areas.

**8.1** Residential properties along Martin Road, primarily within close proximity to Rt-236 To be developed following Public Meeting 3.

#### 8.2 Herb Parson Pond

Streamworks conducted an assessment of groundwater conditions proximal to the Herb Parson Pond and determined that the pond surface elevation coincides with the groundwater elevation. There is minimal drainage area contributing to the pond, the pond setting is near a local topographic high point. The pond appears to be the result of historical gravel mining, and is denoted as a gravel pit on the USDA soil survey with a high infiltration rate. The findings of the Streamworks assessment is provided as **Appendix L.** 

This section to be expanded if further evaluation of the Martin Rd. sewer seep collars provides additional correlation to the Pond water level.

8.3 Central study area (potential future development areas)

To be developed following Public Meeting 3.

8.4 Residential properties along Dennett Road To be developed following Public Meeting 3.

### 8.5 Happy Avenue Access Road

The Happy Avenue access road extension has a series of small diameter culverts that drain surface water from east to west. Further evaluation of these culverts could be conducted to determine if they are adequately sized to convey flow under Happy Avenue. There is potential that these culverts could store water upgradient of Happy Avenue that could ultimately infiltrate and contribute to a localized perched groundwater table. This project was included on the retrofit matrix (see **Appendix K**), but was not pursued as one of the six projects for additional evaluation.

- 8.6 Residential properties along Old Dennett Road, located west of Dennett Road To be developed following Public Meeting 3.
- 8.7 Current Dennett Road development near I-95 on-ramp. To be developed following Public Meeting 3.

#### 9.0 HOMEOWNER FLOOD PROTECTION MEASURES

While the potential stormwater retrofit and improvement projects outlined above may assist with the reduction localized flooding concerns, additional homeowner flood protection measures should also be considered. The following homeowner flood protection recommendations were developed by FEMA (<u>https://www.fema.gov/sites/default/files/documents/fema\_protect-your-home-from-flooding-brochure\_2020.pdf</u>).

#### Exterior Homeowner Flood Protection Measures

- Maintain proper water runoff and drainage. Routinely clean and maintain gutters, downspouts, and splash pads. Ensure drainage ditches and storm drains are clear of debris and functioning properly.
- Improve lot grading. If necessary, consider building up any sunken areas around the foundation, digging small depressions to properly channel water, and grade properties to slopes away from structures.
- Reduce impervious surfaces. Retaining and creating natural green space around properties can help reduce stormwater runoff. Consider options such as rain gardens, vegetated swales, or pervious pavements, which allow more water to be absorbed by the ground.
- Install a rain barrel. Rain barrels are typically connected to gutter downspouts and collect the runoff from roofs. The stored water can be used for non-potable uses such as watering the lawn and gardens.
- Elevate utilities and service equipment. Raise and anchor air conditioning condensers, heat pumps, water meters and other service equipment onto pedestals or platforms that are at least 1 foot above the potential flood elevation.
- Anchor outdoor fuel tanks. Attach outdoor fuel tanks to a large concrete slab that weighs
  enough to resist the force of floodwaters, or install ground anchors that are connected across
  the top of the tank with metal straps. If located in an identified high-risk zone, fuel tanks should
  also be elevated to or above the regulatory flood elevation. If not feasible then all filling and
  ventilation tubes should be elevated so that floodwaters cannot enter the tank.

#### Interior Homeowner Flood Protection Measures

- Protect valuable possessions. Move important documents and valuables above the potential flood elevation and/or inside watertight containers.
- Seal foundations and basement walls. Close any foundation cracks with mortar and masonry caulk or hydraulic cement, which expands and fills gaps completely. Seal basement walls with

waterproofing compounds to avoid seepage. Make sure any floor drains are clear of obstructions.

- Install flood vents. Flood vents are small permanent openings that allow floodwater to flow freely through an enclosure such as a crawlspace or garage. Properly positioned and installed flood vents protect homes during floods by preventing water pressure buildup that can destroy walls and foundations. Flood openings may be required for lower enclosures of homes being built in high-risk flood zones, but they can also be installed in existing homes. Once installed, make sure your flood vents are kept free of debris and will allow the free-flow of floodwater.
- Install a sump pump. Sump pumps, which pump groundwater away from a structure, can prevent or minimize basement seepage and flooding. They draw in the groundwater from around a structure and direct it away from the structure through drainage pipes. Water powered or battery-backup systems are recommended in case of electrical power failure.
- Prevent sewer backups. Install drain plugs for all basement floor drains to prevent sewer backups. Another recommended option, regardless of the potential flood elevation, is to install sewer backflow valves for all pipes entering the building. These devices, which allow water to flow only one direction, prevent floodwater and wastewater from backing up into toilets, sinks, and other drains. A qualified, licensed plumber should install all measures.
- Use flood-resistant building materials. Examples include tile, vinyl, rubber, lime plaster, cement board, concrete, or pressure-treated and decay-resistant wood.
- Raise electrical system components. Increase the height of electric service panels (fuse and circuit breaker boxes) and all outlets, switches, and wiring to at least 1 foot above the potential flood elevation. A licensed electrician should make these modifications.
- Protect utilities and service equipment. Move the main parts of heating, ventilation, and air conditioning (HVAC) systems to a higher floor. Consider raising other major appliances. If relocation or elevation is not possible, service equipment can be protected using low floodwalls and shields.
- Anchor indoor fuel tanks. Anchor fuel tanks by attaching them to a large concrete slab that weighs enough to resist the force of floodwaters.
- Install a flood alert system. A variety of flood sensors and other early warning devices can alert the risk of imminent flooding so preventative or protective actions can be taken.

## 10.0 SUMMARY, RECOMMENDATIONS & NEXT STEPS {TO BE DEVELOPED FOLLOWING SELECTION OF PREFERRED ALTERNATIVES}

### 11.0 REFERENCES

- Dewitz, J. (2021). National Land Cover Database (NLCD) 2019 Products [Data set]. U.S. Geological Survey. https://doi.org/10.5066/P9KZCM54
- National Hydrography Dataset Plus V2 (NHD), 2017. Continental US Medium Resolution Stream Network. Horizon Systems Corporation, 2017.
- U.S. Department of Agriculture (USDA), 2017. Web Soil Survey. Available from: https://websoilsurvey.sc.egov.usda.gov/App/WebSoilSurvey.aspx
- U.S. Geological Survey, 20220622, USGS 1 Meter 19 x35y478 NH\_Coastal\_2019\_B19: U.S. Geological Survey. Available from: <u>USGS 1 Meter 19 x35y478 NH\_Coastal\_2019\_B19 - ScienceBase-Catalog</u>

#### Protect Your Home from Flooding. FEMA.

https://www.fema.gov/sites/default/files/documents/fema\_protect-your-home-from-flooding-brochure\_2020.pdf).

**FIGURES** 

APPENDICES

Appendix A

1984 Federal Emergency Management Agency (FEMA) Flood Rate Insurance Maps (FIRMs)





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	NLI IV M 500-Year Flood Roundary							
	100-Year Flood Boundary	ZONE B						
	Zone Designations*	ZONEAL						
		ZONEAS						
	100-Year Flood Boundary							
	500-Year Flood Boundary	ZONE B						
	Base Flood Elevation Line							
	With Elevation In Feet**							
	Base Flood Elevation in Feet Where Uniform Within Zone**	(EL 987)						
	Elevation Reference Mark	RM7×						
	Zone D Boundary							
	River Mile	●M1.5						
	**Referenced to the National Geodetic	e Vertical Datum of 1929						
	*EXPLANATION OF ZONE DESIGNATIONS							
	A Areas of 100-year flood;	base flood elevations and						
	flood hazard factors not de	etermined.						
	are between one (1) and the of inundation are shown.	hree (3) feet; average depths but no flood hazard factors						
$\mathbf{X}$	are determined.	w flooding where depths						
$\sum$	are between one (1) and elevations are shown, bu	three (3) feet; base flood						
	are determined.	hase flood elevations and						
	flood hazard factors deterr	nined.						
3	A99 Areas of 100-year flood protection system under alwations and flood box	to be protected by flood construction; base flood						
	elevations and flood haza B Areas between limits of t	he 100-year flood and 500-						
	year flood; or certain area ing with average depths less	s subject to 100-year flood- s than one (1) foot or where						
200	the contributing drainage mile; or areas protected by	area is less than one square / levees from the base flood.						
	(Medium shading) C Areas of minimal flooding.	(No shading)						
	D Areas of undetermined, b	out possible, flood hazards.						
-	V Areas of 100-year coastal action); base flood elevation	flood with velocity (wave ons and flood hazard factors						
	not determined. V1-V30 Areas of 100-year coastal	flood with velocity (wave						
	action); base flood elevation determined.	ons and flood hazard factors						
	manager (1999) and an							
	NOTES TO US	ER						
	Certain areas not in the special flood h. may be protected by flood control stru	azard areas (zones A and V) ctures.						
	trus map is for flood insurance and flo poses only; if does not necessarily sho	w all areas subject to flood-						
	ing in the community or all planimet flood hazard areas. The coastal flood	ric teatures outside special ding elevations shown may						
	differ significantly from those develop Service for hurricane evacuation planning	ed by the National Weather						
$\setminus  $	For adjoining man papels and plaining	elv printed Index To Min						
$\langle \rangle  $	Panels.	ety printed index to map						
Y	Coastal base flood elevations shown on	this map include the effects						
	of wave action.							
	Coastal base flood elevations apply onl	y landward of the shoreline						
	snown on this heap.							
	INITIAL IDENTIFI	CATION:						
	NOVEMBER 1,	1974						
	FLOOD HAZARD BOUNDAR	Y MAP REVISIONS:						
1945 1945	SEPTEMBER 24, OCTOBER 1, 1	983						
Standard								
	FLOOD INSURANCE RATE	MAP EFFECTIVE:						
	JULY 5, 198 FLOOD INSURANCE RATF							
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1								
	Reter to the FLOOD INSURANCE RA shown on this map to determine wh	TE MAPEFFECTIVE date en actuarial rates apply to						
	structures in the zones where elevation lished.	s or depths have been estab-						
	To determine if flood insurance is a	vailable in this community,						
	contact your insurance agent, or call th Program, at (800) 638-6620.	ne National Flood Insurance						
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	Federal Emergen	cy Management Agency						

Appendix B

**Current Town of Kittery Zoning** 



Appendix C

FB Environmental Wetland Reconnaissance



Notes: 1. This map represents a preliminary wetland assessment of the survey area; depicted wetland boundaries are approximate. 2. This map does not represent an official wetland delineation, nor does it depict wetlands that might be present within the survey area boundary 3. Areas not surveyed were inaccessible.

Appendix D

Public Survey and Interactive Mapper Summary

Do you experience localized flooding on or immediately adjacent to you	ur property?
Choice	Responses
Yes	9 56.25%
No	7 43.75%
Answered	16
Skipped	0



Is there a specific point in time (year) that you first started noticing flooding on your property? Response Huge rain storm on frozen ground Ever since the boat storage was filled in It floods all the time but starts with heavy spring rains right after the sewer project s[pring Always-I have wetlands No Spring I have a lot of wetlands and 2 vernal pools. every spring there is standing water. in past 2 drought years Answered Skipped

9

If you experience flooding at your property, how frequent is this flooding? Monthly, seasonally, annually, or once every several years? Response Seasonally Ongoing Constant unless there is a drought. I have been experiencing it since we received the recent rains and it continues. monthly (without snow) heavy rains and Spring until early summer Not applicable Seasonally Annually. Wet areas persist Feb through May **Answered Skipped** 

8

If seasonal flooding occurs, what months is it most prevalent?
Response
March. April
Most prevalent in spring, summer and fall.
spring
spring
April through June
N/A
March through November
Usually Feb through May, early June. This year it started earlier
Answered
Skipped

Project Survey: Kittery Watershed Study Please insert any dates you can recall (date, or year and month) of significant flood events that affected your property. Also include magnitude of flood impacts (e.g. 6 inches of water in basement on June 30, 2020).

basement on June 30, 2020).
Response
I have installed more sump pumps
Flooding occurs in the back of our property.
Since I have a fancy french drain system my sump pump which is below ground 12" goes off constantly - however if we loose power I will see an average of 3-4 inches in my basement. I have
not lost power recently but had flooding in Spring of 2017 and 2018.
rainstorms - normally 1-2 inches of water in basement
I get water in my basement with havy rains
Many many times over the year. It's been this way for years here.
My basement takes in some water with any heavy rains
Answered

Answered Skipped

Project Survey: Kittery Watershed Study If you have experienced flooding, have you incurred any out-of-pocket costs to fix damage? If so, how much? Response

Response No ability to fix drainage systems. The french drain cost \$18,000. when installed, since then I have installed a back up battery which was \$800. and a maintenance. Additionally, I have replaced 3 of the lolly columns that were rusted by the years of repeated flooding before I bought the house. Which was \$1200. and need to replace more. I also have to run a dehumidifier which has an increased cost of approx \$50. a month on my electric bill. The previous owners lost everything in the basement- which was completely finished and I am sure it was upwards 10's of thousands of dollars. 2 Sump pumps installed mold removal over 2000 dollars yes on going Missed work, still paying, Serv Pro, pump, dehumidifer, mops, shop vac, est \$2000 No, I have raised areas for storage in my cellar Yes. Foundation and drainage expenses

Yes. Foundation and drainage expenses

Answered

Skipped

Do you experience flooding impacts inside the basement of your home?ChoiceResponsesYes853.33%No746.67%Answered15

Skipped



If so, do you feel the flooding is related to surface water (stormwater) or groundwater	intrusion?
Choice	Responses
Surface water	4 40.00%
Groundwater	6 60.00%
Answered	10
Skipped	6



Where is	your he	ome loc	ated w	ithin the	e study	area?				
Choice							Re	espon	ses	
Dennett I	Rd							4 2	25.00%	
Martin Ro	b							85	50.00%	
Highway	236							0	0.00%	
Other An	swers							4 2	25.00%	
19 Adam	s Dr19	Adams	Dr, Kitt	ery Me						
Нарру А	ve off D	ennett	Rd							
Нарру А	venue									
31 Walke	er Ave									
			w	here is your h	ome located v	vithin the stud	y area?			
Other Answers										٦
										_  <b>_</b> !



Are there any stream, pond, or wetland resources located on or immediately adjacent to	your property?
Choice	Responses
Yes	10 62.50%
No	6 37.50%
Answered	16
Skipped	0

	Are there any stream, pond, or wetland resources located on or immediately adjacent to your property?										
							]				
NO											
							- Yes				
Yes		1		1							
	<u> </u>	2	4	с		<u></u>	]				
	U	2	4	ь	8 1	0	12				

**Project Survey: Kittery Watershed Study** What do you feel are the primary contributing factors to localized flooding that you have experienced?

Choice	Respo	onses
Increased development	10	76.92%
Wetland alteration	10	76.92%
Modifications to natural drainage paths	10	76.92%
Increasing storm intensity	3	23.08%
Drainage infrastructure maintenance	5	38.46%
Other Answers	1	7.69%
sewer project blasting		
Answered	13	
Skipped	3	

		What do you feel are	the primary contributing	factors to localized floodin	g that you have experienc	ed?	
Other Answers							
Drainage infrastructure maintenance							
Increasing storm intensity							wetland alteration
Modifications to natural drainage paths							Modifications to natural drainage paths
Wetland alteration							Increasing storm intensity
Increased development							
	0	2		6	0	10	Drainage infrastructure maintenance
	0	2	+	0	0	10	12

Project Survey: Kittery Watershed Study Identify any changes within the study area that you feel may have impacted flooding frequency. Response

#### Not in the study area.

following the sewer installation on Martin Road I have observed changes at the Herb Parsons Pond by the fisheries/wildlife place on Martin Road

Filling of wetlands.

There has been a lot of development in the area behind my property that was once a wetland. There is fill and new houses that have redirected the flow of the water. Poor, if any, oversight to local development in the area. un-approved or un-appropriate filled in wetland areas with no recourse.

Since mr Cullen built on land off right of way

#### No storm drains

My 92 Dennett Road parcel with the Dennett graveyard. There is a drain pipe and open culvert that bounds the south side of the cemetery alongside the stone wall. There is significant runoff from Dennett Road into this drain. The water table in the grave yard is high. The last grave dug in February 2018 filled with a foot of water and the sod over the grave sunk in 2020 and needed to be refilled. There is a "stream" about 50 feet away from the north side of the of the graveyard that runs east-west towards old Dennett Road that floods regularly in the spring and with heavy rain. The runoff from Dennett Road and Roseberry Lane has caused flooding to property on Old Dennett Road, especially 103 Old Dennett.

## No change. No flooding. Improper development

Drainage pipe to the south of Dennett graveyard impacted by runoff from Dennett Road and 76 Dennett development. Roseberry Lane development has changed water supply to old well on north side of property

Answered Skipped

Project Survey: Kittery Watershed Study Identify any other water quality concerns within the study area. Response

At the Herb Parsons Pond on Martin road I have observed drastic fluctuations and excessive green algae growth. I walk by with my dog EVERY DAY. I believe the water levels began big changes after the sewer project, the blasting etc. I am also concerned with the 236 disruptions in the wetlands near Martin/Stevenson.

Stagnant water . Herb Parson's pond has died since all the fill back in 2016 was brought in for Happy Ave Ext. Nobody from the town was watching. No up to date permits were filed. Shame on you, you need to

Its mix ASAP. Blasting for sever line lost wildlife habitat and appears to have made flooding worse. All runoff from newly developed land goes downhill to the east on the 236 side, creating marshy areas and the Piscataqua River on the west side Home and property damage creates polluted runoff into our creeks, residents properties, and has stopped the natural flow to a local pond. I am concerned about the level of the pond on Martin Rd. It is no longer thriving as it once was. It is VERY low as well, water that collects in area off 236 between Martin Road down to new pump station?

Answered Skipped

Your name (optional): Response Donald Gray Ellen Mitchell King Jodie Curtis Andrew Bedard Sarah Dennett Rod Welles Dennett Sarah **Answered Skipped**
Туре	Comment	Location	
Local Flooding	KLT wetland is partial in the floodplain. But has flooded from beaver activity. There is a former beaver dam remnants on Chickering Creek near the outlet of the wetland 12" CPP culvert under the existing is likely undersized. See drainage calculations for proposed project at 41 Bt 236	32 Route 236 Unit 1	1
Other	Per 1956 Rt 236 highway plansa 1'x2' stone culvert drained Chickering Creek under the abandoned railroad bed here. The stone culvert was abandoned when the current 36" RCP drain was installed as part of roadway construction.	41 Route 236	
Potentially Undersized Storm Drainage	15" CMP may be undersized. See drainage calculation from the downstream 41 Rt 236 project. Outlet is "hanging" about one foot above the water level creating a manmade barrier for aquatic species to pass.	124 Martin Rd	(P Burrmun
Local Flooding	Neighbors on Martin water displacement during the first fill/development of this wetland2009 for an extension of a boat yard. When more fill was added in 2015, residents began to lose trees in the remaining buffer area between them and the yard with one resident losing everything including furnace. The flooding worsened so a professional, permanent sump pump was installed. Trees continue to die and the remaining buffer barely exists on some of the properties. 1941 map of creek running to wetland	122 Martin Rd	Bouth E
Other	Drainage from this wetland is to the northeast, refer to Town GIS / LiDAR. Drainage across the Martin Road from this wetland is incorrectly shown on many maps since 1956.	91 Martin Rd	
Other	Culvert pipe 12 in black goes down side of road and empties into swamp. Not sure if pipe end is crushed, might be hard to find with all the leaves. They put timbers in for an excavator to go through power line section.	92 Martin Rd	
Local Flooding	Development on wetland at the end of Summer Ave. a couple of years ago. After 2 homes were built, the runoff now runs down Summer Lane, sheets across Martin into properties. I have very clear video, but am unable to attach. It is important to note that there are many inaccuracies with the GIS map. One of the most obvious is the lack of wetlands marked out	64 Martin Pd	
Other	Pond used to be a gravel pit, story goes the pond was dry at one point and steamshovel excavator hit a spring when digging and it flooded the pond over a weekend.	Club pond on Martin Rd	
	Herb Parsons Pond was created accidentally in the 40s when excavating was taking place for the Navy Yard. It remained a healthy and popular pond for 80 years until development began on the southerly side beginning with Roseberry, then Condo Way, then when a sewer line was installed in 2015 on Martin. The most notable change occurred around 2019. By 2020 it was clear that water was being redirected away from the pond through culverts that were installed for		
Potentially Undersized Culvert	sewer and water hookup for development.	Club pond on Martin Rd	
Local Flooding	Homes and properties flooded from development/displaced water from filling of wetlands and placement of culverts. Timeline attached. Basement flooding occurred 22 years ago when Happy Ave was	Нарру Аve	
Local Flooding	brought in to develop what is now called Condominium Way. This has redirected the water from an old pond into residents yards and basements.	Нарру Аve	
Local Flooding	All through this area continuing South on Dennett experienced flooding with at least one home with basement flooding with the development of Roseberry Ave. Needed to place large culverts, but the yard continues to flood along with other areas. in 2018 a new grave was dug. The water table was so high, there	118 Dennet Rd	
Local Flooding	was appox. 6" to 12" of water causing the coffin to float. In 2020, the gravesite sunk by approx the same amount and needed to be repaired.	next to 80 Old Dennet Rd	

#### Legend



**E** 

Project Area



#### TimeLine Happy Ave Flooding

1980's: 40+ years ago Mike and Brenda Crouse, family of Bob and Betty Kraft tried to buy land directly off the current ROW on Happy Ave but the water table was so high it would not perk.

1980's- The Stihman's owned 6 Happy Ave and had renovated the house- adding a finished recreational room with flooring, paneling and tiki bar it as a Rec room for many years.

**1990's** – in the mid to late 1990's water table changes started to occur when development occurred and in land around Happy Ave- with the development of homes and the clearing of trees. They worked hard to try and mitigate the situation by addressing the water coming into the foundation from the ground and outside by digging trenches and putting in pipes to redirect the water.

**2000's** Flooding became a regular occurrence during this time period and caused loss and damage to the basement of the house, forcing the owner to remove the finished walls and flooring property that had been in the space for over 50 years without incident.

2015 – A new owner purchased the property and after a significant water situation Jeffery put in an \$18,000. a French drain system with a buried sump pump was added around 2015/2016. This system pumps water out into the yard and fills my land with the expelled water.

2017 I purchased the home assuming that the elaborate water mitigations system would remove any water and I could restore the space in the basement to useful space. Unfortunately as we suffered power losses and sump pump burn outs, I quickly realized that I would be living with this issue all year long. I have replaced the pump twice because it has burned out and had to install a battery system for close to \$900.00 because when we lose power the pump would fail and I ended up with a flooded basement again. The first year I owned the house the basement flooded 4 times and I lost many of my belongings. There are many health hazards associated with having standing (or running in some cases) water rise up through the ground. I have worked hard to deal with this situation to reduce any more harm. Listening to the pump go off all night long has been disruptive to my sleep and affects my health beyond the physical water.

Present: The property behind my house that runs along the ROW has seen a change in wetlands according to the maps at the town website I have reviewed. There is definitely flooding from run off due to the culvert pipes that Bill Cullen created when he rebuilt the right of way to the property he built and sold in 2018 as well as his house. There seems to be 3 or 4 culverts that direct the water to run from the wetland on the opposite side of theROW (Maine F&G property) to my property. The ROW was built up to accommodate a sewer line and to support heavy equipment and put in Culverts/drain pipes that stretch on either side so the road would not flood.

Appendix E

National Oceanic and Atmospheric Administration (NOAA) Atlas 14 Rainfall Data



NOAA Atlas 14, Volume 10, Version 3 Location name: Kittery, Maine, USA\* Latitude: 43.1059°, Longitude: -70.7556° Elevation: 78 ft\*\* \* source: ESRI Maps \*\* source: USGS



#### POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

#### PF\_tabular | PF\_graphical | Maps\_&\_aerials

#### **PF tabular**

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) <sup>1</sup>										
Duration				Average	recurrence	interval (ye	ears)			
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	<b>0.307</b>	<b>0.370</b>	<b>0.473</b>	<b>0.558</b>	<b>0.675</b>	<b>0.763</b>	<b>0.856</b>	<b>0.960</b>	<b>1.11</b>	<b>1.23</b>
	(0.233-0.405)	(0.280-0.488)	(0.357-0.626)	(0.419-0.743)	(0.494-0.937)	(0.549-1.08)	(0.601-1.26)	(0.642-1.44)	(0.718-1.72)	(0.781-1.94)
10-min	<b>0.435</b>	<b>0.524</b>	<b>0.670</b>	<b>0.791</b>	<b>0.957</b>	<b>1.08</b>	<b>1.21</b>	<b>1.36</b>	<b>1.57</b>	<b>1.75</b>
	(0.330-0.574)	(0.397-0.692)	(0.506-0.887)	(0.594-1.05)	(0.700-1.33)	(0.778-1.53)	(0.851-1.78)	(0.909-2.04)	(1.02-2.43)	(1.11-2.75)
15-min	<b>0.512</b>	<b>0.617</b>	<b>0.788</b>	<b>0.930</b>	<b>1.12</b>	<b>1.27</b>	<b>1.43</b>	<b>1.60</b>	<b>1.85</b>	<b>2.06</b>
	(0.388-0.675)	(0.467-0.814)	(0.595-1.04)	(0.699-1.24)	(0.823-1.56)	(0.914-1.80)	(1.00-2.09)	(1.07-2.40)	(1.20-2.86)	(1.30-3.24)
30-min	<b>0.688</b>	<b>0.830</b>	<b>1.06</b>	<b>1.25</b>	<b>1.52</b>	<b>1.72</b>	<b>1.93</b>	<b>2.17</b>	<b>2.52</b>	<b>2.80</b>
	(0.522-0.908)	(0.629-1.10)	(0.802-1.41)	(0.942-1.67)	(1.11-2.11)	(1.24-2.44)	(1.35-2.83)	(1.45-3.24)	(1.62-3.89)	(1.78-4.42)
60-min	<b>0.865</b>	<b>1.04</b>	<b>1.34</b>	<b>1.58</b>	<b>1.91</b>	<b>2.16</b>	<b>2.42</b>	<b>2.73</b>	<b>3.18</b>	<b>3.55</b>
	(0.656-1.14)	(0.790-1.38)	(1.01-1.77)	(1.19-2.10)	(1.40-2.66)	(1.56-3.07)	(1.71-3.57)	(1.82-4.09)	(2.05-4.92)	(2.25-5.60)
2-hr	<b>1.16</b>	<b>1.40</b>	<b>1.80</b>	<b>2.14</b>	<b>2.60</b>	<b>2.94</b>	<b>3.31</b>	<b>3.75</b>	<b>4.42</b>	<b>5.00</b>
	(0.880-1.51)	(1.07-1.84)	(1.37-2.38)	(1.62-2.83)	(1.92-3.60)	(2.13-4.17)	(2.35-4.88)	(2.51-5.60)	(2.86-6.82)	(3.17-7.85)
3-hr	<b>1.36</b>	<b>1.66</b>	<b>2.14</b>	<b>2.54</b>	<b>3.09</b>	<b>3.50</b>	<b>3.95</b>	<b>4.49</b>	<b>5.31</b>	<b>6.02</b>
	(1.04-1.78)	(1.26-2.16)	(1.63-2.81)	(1.92-3.35)	(2.29-4.28)	(2.55-4.96)	(2.82-5.82)	(3.01-6.68)	(3.45-8.18)	(3.83-9.44)
6-hr	<b>1.77</b>	<b>2.17</b>	<b>2.82</b>	<b>3.36</b>	<b>4.10</b>	<b>4.65</b>	<b>5.24</b>	<b>5.97</b>	<b>7.09</b>	<b>8.05</b>
	(1.36-2.30)	(1.66-2.82)	(2.16-3.67)	(2.56-4.40)	(3.04-5.64)	(3.40-6.55)	(3.76-7.71)	(4.02-8.85)	(4.61-10.9)	(5.14-12.6)
12-hr	<b>2.23</b>	<b>2.75</b>	<b>3.60</b>	<b>4.31</b>	<b>5.29</b>	<b>6.01</b>	<b>6.79</b>	<b>7.74</b>	<b>9.20</b>	<b>10.5</b>
	(1.72-2.88)	(2.12-3.56)	(2.77-4.67)	(3.30-5.62)	(3.95-7.24)	(4.41-8.43)	(4.89-9.93)	(5.23-11.4)	(6.01-14.1)	(6.70-16.3)
24-hr	<b>2.63</b>	<b>3.31</b>	<b>4.41</b>	<b>5.32</b>	<b>6.58</b>	<b>7.50</b>	<b>8.52</b>	<b>9.81</b>	<b>11.9</b>	<b>13.6</b>
	(2.04-3.38)	(2.56-4.25)	(3.41-5.68)	(4.10-6.90)	(4.95-9.00)	(5.56-10.5)	(6.20-12.5)	(6.65-14.4)	(7.76-18.1)	(8.76-21.2)
2-day	<b>2.93</b>	<b>3.77</b>	<b>5.15</b>	<b>6.28</b>	<b>7.85</b>	<b>8.99</b>	<b>10.3</b>	<b>12.0</b>	<b>14.9</b>	<b>17.5</b>
	(2.29-3.74)	(2.94-4.82)	(4.00-6.59)	(4.86-8.10)	(5.96-10.8)	(6.73-12.7)	(7.59-15.2)	(8.16-17.6)	(9.77-22.6)	(11.3-27.1)
3-day	<b>3.19</b>	<b>4.10</b>	<b>5.58</b>	<b>6.81</b>	<b>8.51</b>	<b>9.73</b>	<b>11.1</b>	<b>13.0</b>	<b>16.3</b>	<b>19.2</b>
	(2.50-4.05)	(3.21-5.21)	(4.36-7.12)	(5.29-8.75)	(6.48-11.6)	(7.32-13.7)	(8.27-16.5)	(8.88-19.1)	(10.7-24.7)	(12.4-29.7)
4-day	<b>3.44</b>	<b>4.38</b>	<b>5.92</b>	<b>7.20</b>	<b>8.96</b>	<b>10.2</b>	<b>11.7</b>	<b>13.7</b>	<b>17.1</b>	<b>20.2</b>
	(2.70-4.36)	(3.44-5.56)	(4.63-7.54)	(5.60-9.22)	(6.84-12.2)	(7.71-14.4)	(8.70-17.3)	(9.33-20.0)	(11.2-25.9)	(13.0-31.1)
7-day	<b>4.16</b>	<b>5.16</b>	<b>6.78</b>	<b>8.13</b>	<b>9.98</b>	<b>11.3</b>	<b>12.8</b>	<b>14.9</b>	<b>18.5</b>	<b>21.6</b>
	(3.28-5.25)	(4.06-6.51)	(5.32-8.59)	(6.35-10.4)	(7.64-13.5)	(8.55-15.8)	(9.57-18.9)	(10.2-21.8)	(12.2-27.8)	(14.0-33.3)
10-day	<b>4.85</b>	<b>5.88</b>	<b>7.56</b>	<b>8.95</b>	<b>10.9</b>	<b>12.3</b>	<b>13.8</b>	<b>15.9</b>	<b>19.4</b>	<b>22.5</b>
	(3.84-6.10)	(4.65-7.40)	(5.95-9.55)	(7.01-11.4)	(8.33-14.6)	(9.26-17.0)	(10.3-20.1)	(10.9-23.1)	(12.8-29.2)	(14.6-34.5)
20-day	<b>6.88</b>	<b>8.01</b>	<b>9.85</b>	<b>11.4</b>	<b>13.5</b>	<b>15.1</b>	<b>16.7</b>	<b>18.8</b>	<b>21.8</b>	<b>24_4</b>
	(5.47-8.59)	(6.36-10.0)	(7.80-12.4)	(8.96-14.4)	(10.3-17.9)	(11.3-20.4)	(12.3-23.7)	(13.0-27.1)	(14.5-32.6)	(15.9-37.3)
30-day	<b>8.55</b>	<b>9.76</b>	<b>11.7</b>	<b>13.4</b>	<b>15.6</b>	<b>17.3</b>	<b>19.1</b>	<b>21.0</b>	<b>23.8</b>	<b>26.0</b>
	(6.82-10.6)	(7.78-12.2)	(9.32-14.7)	(10.6-16.8)	(12.0-20.5)	(13.0-23.3)	(13.9-26.7)	(14.6-30.2)	(15.9-35.5)	(16.9-39.7)
45-day	<b>10.6</b>	<b>11.9</b>	<b>14.1</b>	<b>15.8</b>	<b>18.2</b>	<b>20.1</b>	<b>22.0</b>	<b>23.9</b>	<b>26.4</b>	<b>28.2</b>
	(8.50-13.2)	(9.53-14.8)	(11.2-17.5)	(12.5-19.8)	(14.0-23.8)	(15.1-26.8)	(15.9-30.3)	(16.6-34.2)	(17.6-39.2)	(18.4-43.0)
60-day	<b>12_4</b>	<b>13.7</b>	<b>16.0</b>	<b>17.8</b>	<b>20.4</b>	<b>22.4</b>	<b>24.4</b>	<b>26.2</b>	<b>28.6</b>	<b>30.3</b>
	(9.92-15.3)	(11.0-17.0)	(12.8-19.9)	(14.2-22.3)	(15.7-26.5)	(16.8-29.7)	(17.6-33.3)	(18.3-37.5)	(19.2-42.5)	(19.8-46.1)

<sup>1</sup> Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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## **PF graphical**

30 25 Precipitation depth (in) 20 15 10 5 0 30-min 60-min Juration 7-day 10-day 45-day 60-day 5-min 10-min 15-min 2-hr 3-hr 24-hr 2-day 3-day 4-day 20-day 30-day 30 25 Precipitation depth (in) 20 15 10 5 0 5 10 25 50 100 200 500 1000 1 2 Average recurrence interval (years)



Dura	ation
5-min	- 2-day
- 10-min	— 3-day
- 15-min	— 4-day
- 30-min	- 7-day
- 60-min	— 10-day
- 2-hr	- 20-day
— 3-hr	— 30-day
— 6-hr	— 45-day
- 12-hr	- 60-day
24-hr	

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Maps & aerials

Small scale terrain

Appendix F

Existing Conditions (No Build) HydroCAD Summary Report



# **Project Notes**

Copied 10 events from ME-DennettRoad 24-hr S1 storm

	Event#	Event	Storm Type		Mode	Duration	B/B	Depth	AMC
_		Name				(nours)		(inches)	
	1	1-yr	ME-DennettRoad 24-hr S1	1-yr	Default	24.00	1	2.63	2
	2	2-yr	ME-DennettRoad 24-hr S1	2-yr	Default	24.00	1	3.31	2
	3	5-yr	ME-DennettRoad 24-hr S1	5-yr	Default	24.00	1	4.41	2
	4	10-yr	ME-DennettRoad 24-hr S1	10-yr	Default	24.00	1	5.32	2
	5	25-yr	ME-DennettRoad 24-hr S1	25-yr	Default	24.00	1	6.58	2
	6	50-yr	ME-DennettRoad 24-hr S1	50-yr	Default	24.00	1	7.50	2
	7	100-yr	ME-DennettRoad 24-hr S1	100-yr	Default	24.00	1	8.52	2
	8	Extreme: 1-yr	ME-DennettRoad 24-hr S1	1-yr	Default	24.00	1	2.63	2
	9	Extreme: 10-yr	ME-DennettRoad 24-hr S1	10-yr	Default	24.00	1	5.32	2

#### Rainfall Events Listing (selected events)

## Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
0.423	89	*BARREN, D (89,92,95) (1-2)
0.723	89	*COMMERCIAL, A (89) (2-2)
9.929	95	*COMMERCIAL, D (95) (1-2, 1-3)
6.495	85	*RESIDENTIAL, D (85, 90, 95) (1-2, 1-3)
33.767	30	*WOODS, A (30, 60, 95) (2-1, 2-2, 4-1)
20.728	77	*WOODS, D (77, 86, 95) (1-3, 2-1, 2-2, 4-1)
1.065	77	*WOODS, D (77, 86,95) (1-2)
8.453	54	1/2 acre lots, 25% imp, HSG A (2-1, 2-2, 3-1, 3-2, 4-1)
0.193	80	1/2 acre lots, 25% imp, HSG C (1-1)
12.899	85	1/2 acre lots, 25% imp, HSG D (1-1, 1-2, 2-1, 3-1, 3-2)
1.776	77	1/8 acre lots, 65% imp, HSG A (1-1, 3-1, 3-2, 4-1)
0.089	90	1/8 acre lots, 65% imp, HSG C (1-1)
3.453	92	1/8 acre lots, 65% imp, HSG D (1-1, 1-2, 2-1, 3-1, 3-2)
7.040	39	>75% Grass cover, Good, HSG A (2-1, 2-2, 3-1, 3-2, 4-1)
0.916	74	>75% Grass cover, Good, HSG C (1-1)
16.248	80	>75% Grass cover, Good, HSG D (1-1, 1-2, 2-1, 2-2, 3-1, 3-2)
0.006	65	Brush, Good, HSG C (1-1)
0.291	89	COMMERCIAL, A (89) (2-4, 5-1)
23.114	95	COMMERCIAL, D (95) (1-4, 2-3, 2-4, 4-2, 5-1, 5-2, 6-1)
3.708	85	RESIDENTIAL, D (85, 90, 95) (1-4)
4.367	30	WOODS, A (30, 60, 95) (2-4, 4-2, 5-1)
3.785	77	WOODS, D (77, 81, 85) (1-4)
32.923	77	WOODS, D (77, 86, 95) (1-4, 2-3, 2-4, 4-2, 5-1, 5-2, 6-1)
1.790	98	Water Surface, HSG A (3-1-P)
28.687	30	Woods, Good, HSG A (1-1, 2-1, 2-2, 3-1, 3-2, 4-1)
2.174	70	Woods, Good, HSG C (1-1)
98.866	77	Woods, Good, HSG D (1-1, 1-2, 1-3, 2-1, 2-2, 3-1, 3-2, 4-1)
323.908	69	TOTAL AREA

# Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
47.746	HSG A	1-1, 2-1, 2-2, 3-1, 3-1-P, 3-2, 4-1
0.000	HSG B	
3.378	HSG C	1-1
131.466	HSG D	1-1, 1-2, 1-3, 2-1, 2-2, 3-1, 3-2, 4-1
141.318	Other	1-2, 1-3, 1-4, 2-1, 2-2, 2-3, 2-4, 4-1, 4-2, 5-1, 5-2, 6-1
323.908		TOTAL AREA

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Ground Covers (all nodes)

HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
(acres)	(acres)	(acres)	(acres)	(acres)	(acres)	Cover	Numbers
0.000	0.000	0.000	0.000	0.423	0.423	*BARREN, D (89,92,95)	1-2
0.000	0.000	0.000	0.000	0.723	0.723	*COMMERCIAL, A (89)	2-2
0.000	0.000	0.000	0.000	9.929	9.929	*COMMERCIAL, D (95)	1-2, 1-3
0.000	0.000	0.000	0.000	6.495	6.495	*RESIDENTIAL, D (85, 90, 95)	1-2, 1-3
0.000	0.000	0.000	0.000	33.767	33.767	*WOODS, A (30, 60, 95)	2-1, 2-2,
							4-1
0.000	0.000	0.000	0.000	20.728	20.728	*WOODS, D (77, 86, 95)	1-3, 2-1,
							2-2, 4-1
0.000	0.000	0.000	0.000	1.065	1.065	*WOODS, D (77, 86,95)	1-2
8.453	0.000	0.193	12.899	0.000	21.545	1/2 acre lots, 25% imp	1-1, 1-2,
							2-1, 2-2,
							3-1, 3-2,
							4-1
1.776	0.000	0.089	3.453	0.000	5.318	1/8 acre lots, 65% imp	1-1, 1-2,
							2-1, 3-1,
							3-2, 4-1
7.040	0.000	0.916	16.248	0.000	24.204	>75% Grass cover, Good	1-1, 1-2,
							2-1, 2-2,
							3-1, 3-2,
							4-1
0.000	0.000	0.006	0.000	0.000	0.006	Brush, Good	1-1
0.000	0.000	0.000	0.000	0.291	0.291	COMMERCIAL, A (89)	2-4, 5-1
0.000	0.000	0.000	0.000	23.114	23.114	COMMERCIAL, D (95)	1-4, 2-3,
							2-4, 4-2,
							5-1, 5-2,
							6-1
0.000	0.000	0.000	0.000	3.708	3.708	RESIDENTIAL, D (85, 90, 95)	1-4
0.000	0.000	0.000	0.000	4.367	4.367	WOODS, A (30, 60, 95)	2-4, 4-2,
							5-1
0.000	0.000	0.000	0.000	3.785	3.785	WOODS, D (77, 81, 85)	1-4
0.000	0.000	0.000	0.000	32.923	32.923	WOODS, D (77, 86, 95)	1-4, 2-3,
							2-4, 4-2,
							5-1, 5-2,
. =							6-1
1.790	0.000	0.000	0.000	0.000	1.790	Water Surface	3-1-P
28.687	0.000	2.174	98.866	0.000	129.727	Woods, Good	1-1, 1-2,
							1-3, 2-1,
							2-2, 3-1,
47 7 40	0 000	0.070	404 400	444 040	000 000		3-2, 4-1
47.746	0.000	3.378	131.466	141.318	323.908	IUIAL AREA	

## Kittery\_NoBuild (ID 2853676)

16

17

18

19

20

CB-4-1

CB-4-2

CB-5-1

CB-5-2

CB-6-1

54.00

54.00

56.00

40.00

49.25

52.00

52.00

55.00

34.00

46.31

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#### Line# Node In-Invert Width Diam/Height Inside-Fill Out-Invert Length Slope n Number (feet) (feet) (feet) (ft/ft) (inches) (inches) (inches) 1 1-4 0.00 0.00 225.0 0.0180 0.025 0.0 12.0 0.0 2 3-1 0.00 0.00 10.0 0.0500 0.013 0.0 6.0 0.0 3 3-2 0.00 0.00 68.0 0.0290 0.025 0.0 12.0 0.0 4 1-P 37.91 37.91 23.0 0.0000 0.012 0.0 12.0 0.0 2-P 0.0560 5 35.58 32.78 50.0 0.025 0.0 18.0 0.0 6 CB-1-1 47.05 46.31 50.0 0.0148 0.025 0.0 14.0 0.0 7 CB-1-3 95.0 0.0019 0.012 0.0 36.98 36.80 0.0 36.0 8 CB-1-4 0.025 0.0 36.00 33.00 84.0 0.0357 0.0 24.0 9 CB-2-3 30.00 28.50 150.0 0.0100 0.012 0.0 36.0 0.0 CB-2-4 75.0 0.0 10 30.00 30.00 0.0000 0.012 0.0 24.0 11 CB-3-1 86.11 86.46 20.0 -0.0175 0.012 0.0 0.0 6.0 CB-3-1 0.0175 0.012 0.0 12 86.67 86.32 20.0 0.0 12.0 13 CB-3-1 86.64 86.51 20.0 0.0065 0.012 0.0 6.0 0.0 14 CB-3-1 86.97 86.75 20.0 0.0110 0.012 0.0 14.0 0.0 15 CB-3-2 54.00 52.00 100.0 0.0200 0.012 0.0 24.0 0.0

100.0

100.0

70.0

95.0

217.0

0.0200

0.0200

0.0143

0.0276

0.0309

0.012

0.012

0.012

0.012

0.012

0.0

0.0

0.0

0.0

0.0

24.0

24.0

36.0

32.0

18.0

0.0

0.0

0.0

0.0

0.0

## Pipe Listing (all nodes)

Kittery\_NoBuild (ID 2853676)ME-DennettRoad 24-hr S1 1-yrRainfall=2.63"Prepared by Barton & Loguidice, DPCPrinted 8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 8

Subcatchment 1-1: DA-1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth=0.90" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=18.87 cfs 3.059 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth=1.02" Flow Length=485' Tc=27.5 min CN=WQ Runoff=13.31 cfs 1.914 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth=1.45" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=22.23 cfs 2.442 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth=1.39" Flow Length=875' Tc=18.9 min CN=WQ Runoff=18.50 cfs 2.139 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth=0.80" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=17.96 cfs 4.502 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth=0.26" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=4.74 cfs 1.127 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth=1.03" Flow Length=850' Tc=25.6 min CN=WQ Runoff=4.55 cfs 0.639 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth=1.12" Flow Length=745' Tc=17.7 min CN=WQ Runoff=7.03 cfs 0.803 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth=0.57" Flow Length=688' Tc=33.2 min CN=WQ Runoff=2.40 cfs 0.399 af
Subcatchment 3-1-P: DA-3-	I-PRunoff Area=1.790 ac100.00% ImperviousRunoff Depth=2.40"Tc=0.0 minCN=98Runoff=5.87 cfs0.358 af
Subcatchment 3-2: DA-3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth=0.31" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=5.12 cfs 0.673 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth=0.24" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=1.68 cfs 0.319 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth=0.69" Flow Length=955' Tc=37.6 min CN=WQ Runoff=3.29 cfs 0.578 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth=0.90" Flow Length=625' Tc=33.8 min CN=WQ Runoff=4.74 cfs 0.778 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth=1.84" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=23.98 cfs 2.023 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth=1.97" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.13 cfs 0.017 af

Kittery_NoBuild (ID 28	53676)	ME-DennettRoad 24-h	r S1 1-yr Rainfall <mark>=</mark> 2.63"
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Reach R1-1: R1-1	Avg. Flow l	Depth=0.95' Max Vel=3.32 fps	Inflow=18.87 cfs 3.059 af
	n=0.035 L=800.0' S=0	).0117 '/' Capacity=90.60 cfs	Outflow=18.51 cfs 3.059 af
Reach R1-2: R1-2	Avg. Flow	/ Depth=0.09' Max Vel=1.75 fp	os Inflow=0.52 cfs 0.810 af
	n=0.035 L=20.0' S=0	).0465 '/' Capacity=180.91 cfs	Outflow=0.52 cfs 0.810 af
Reach R1-3: R-1-3	Avg. Flow l	Depth=1.51' Max Vel=1.73 fps	s Inflow=31.63 cfs 6.311 af
	n=0.040 L=225.0' S=0	).0022 '/' Capacity=56.74 cfs	Outflow=31.45 cfs 6.307 af
Reach R1-4: R1-4	Avg. Flow l	Depth=0.79' Max Vel=4.33 fps	Inflow=18.50 cfs 2.139 af
	n=0.035 L=290.0' S=0.0	0241 '/' Capacity=130.35 cfs	Outflow=18.41 cfs 2.139 af
Reach R1-5: R1-5			Inflow=31.45 cfs 6.307 af Outflow=31.45 cfs 6.307 af
Reach R2-1: R2-1	Avg. Flow	/ Depth=0.42' Max Vel=1.78 fp	os Inflow=3.18 cfs 4.846 af
	n=0.035 L=460.0' S=	=0.0082 '/' Capacity=76.05 cfs	Outflow=3.18 cfs 4.836 af
Reach R2-2: R2-2	Avg. Flow	/ Depth=0.61' Max Vel=3.60 fp	os Inflow=7.03 cfs 0.803 af
	n=0.030 L=175.0' S=	=0.0171 '/' Capacity=82.75 cfs	Outflow=7.02 cfs 0.803 af
Reach R2-3: R2-3	Avg. Flow	/ Depth=0.59' Max Vel=2.83 fp	os Inflow=5.34 cfs 5.475 af
	n=0.030 L=410.0' S=	=0.0110 '/' Capacity=66.21 cfs	Outflow=5.29 cfs 5.469 af
Reach R3-1: R3-1	Avg. Flow	/ Depth=0.31' Max Vel=2.71 fp	os Inflow=2.40 cfs 0.399 af
	n=0.030 L=1,596.0' S=	=0.0203 '/' Capacity=89.94 cfs	Outflow=2.18 cfs 0.399 af
Reach R3-P: R3-P	Avg. Flow	/ Depth=0.00' Max Vel=0.00 fp	os Inflow=0.00 cfs 0.000 af
	n=0.040 L=276.0' S	S=0.0056 '/' Capacity=3.77 cfs	Outflow=0.00 cfs 0.000 af
Reach R4-1: R4-1	Avg. Flow	/ Depth=0.19' Max Vel=3.67 fp	os Inflow=1.68 cfs 0.319 af
	n=0.030 L=95.0' S=0	).0632 '/' Capacity=158.84 cfs	Outflow=1.68 cfs 0.319 af
Reach R4-2: R4-2	Avg. Flow	/ Depth=0.27' Max Vel=4.75 fp	os Inflow=3.29 cfs 0.578 af
	n=0.030 L=140.0' S=0	).0714 '/' Capacity=168.92 cfs	Outflow=3.29 cfs 0.578 af
Reach R5-1: R5-1	Avg. Flow	/ Depth=0.39' Max Vel=2.89 fp	os Inflow=4.74 cfs 0.778 af
	n=0.035 L=640.0' S=0	).0234 '/' Capacity=325.19 cfs	Outflow=4.67 cfs 0.778 af
Reach R5-2: R5-2	Avg. Flow l	Depth=1.13' Max Vel=3.14 fps	Inflow=25.25 cfs 2.801 af
	n=0.035 L=700.0' S=0	).0086 '/' Capacity=77.67 cfs	Outflow=22.75 cfs 2.801 af
Reach R6-1: R6-1	Avg. Flow	/ Depth=0.06' Max Vel=1.01 fp	os Inflow=0.13 cfs 0.017 af
	n=0.030 L=360.0' S=	=0.0203 '/' Capacity=90.06 cfs	Outflow=0.12 cfs 0.017 af
Pond 1-P:	Peak	Elev=38.44' Storage=1.490 at	f Inflow=13.31 cfs 1.914 af
	12.0" Round Culvert	n=0.012 L=23.0' S=0.0000 '/'	Outflow=0.52 cfs 0.810 af
Pond 2-P:	Peak	Elev=36.55' Storage=2.849 af	f Inflow=22.61 cfs 5.629 af Outflow=3.18 cfs 4.846 af

Kittery_NoBuild (ID 2853676 Prepared by Barton & Loguidic	b)ME-DennettRoad 24-hr S1 1-yr Rainfae, DPCPrinted 8	l <b>l=2.63"</b> 3/7/2023
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Pond 3-P:	Peak Elev=86.25' Storage=0.358 af Inflow=5.87 cfs Outflow=0.00 cfs	0.358 af 0.000 af
Pond CB-1-1: L1-1	Peak Elev=79.53' Inflow=18.87 cfs 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=18.87 cfs	3.059 af 3.059 af
Pond CB-1-3: L1-3	Peak Elev=39.83' Inflow=31.63 cfs 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=31.63 cfs	6.311 af 6.311 af
Pond CB-1-4: L1-4	Peak Elev=39.40' Inflow=18.50 cfs 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=18.50 cfs	2.139 af 2.139 af
Pond CB-2-3: L2-2	Peak Elev=30.86' Inflow=5.34 cfs 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=5.34 cfs	5.475 af 5.475 af
Pond CB-2-4: L2-3	Peak Elev=31.64' Inflow=7.03 cfs 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=7.03 cfs	0.803 af 0.803 af
Pond CB-3-1: L3-1	Peak Elev=87.28' Inflow=2.40 cfs Outflow=2.40 cfs	0.399 af 0.399 af
Pond CB-3-2: L3-2	Peak Elev=55.08' Inflow=6.14 cfs 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=6.14 cfs	1.073 af 1.073 af
Pond CB-4-1: L4-1	Peak Elev=54.53' Inflow=1.68 cfs 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=1.68 cfs	0.319 af 0.319 af
Pond CB-4-2: L4-2	Peak Elev=54.76' Inflow=3.29 cfs 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=3.29 cfs	0.578 af 0.578 af
Pond CB-5-1: L5-1	Peak Elev=56.81' Inflow=4.74 cfs 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=4.74 cfs	0.778 af 0.778 af
Pond CB-5-2: L5-2	Peak Elev=42.22' Inflow=25.25 cfs 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=25.25 cfs	2.801 af 2.801 af
Pond CB-6-1: L6-1	Peak Elev=49.40' Inflow=0.13 cfs 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.13 cfs	0.017 af 0.017 af
Link L1-5: L1-5	Inflow=31.45 cfs Primary=31.45 cfs	6.307 af 6.307 af
Link L1-6: L1-6	Inflow=49.32 cfs Primary=49.32 cfs	8.446 af 8.446 af
Link L2-1: L2-1	Inflow=3.18 cfs Primary=3.18 cfs	4.846 af 4.846 af
Link L2-4: L2-4	Inflow=5.29 cfs Primary=5.29 cfs	5.469 af 5.469 af

Kittery_NoBuild (ID 2853676)	ME-DennettRoad 24-hr S1 1-yr Rainfall=2.63			
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Link L2-5: L2-5	Inflow=7.02 cfs 0.803 af			
	Primary=7.02 cfs 0.803 af			
1 ink   2 6   2 6	Inflow-11 55 cfs 6 272 of			
LIIK L2-0. L2-0	Primary-11.55 cfs 6.272 af			
	1 hindry=11.00 013 0.272 di			
Link L4-3: L4-3	Inflow=1.68 cfs 0.319 af			
	Primary=1.68 cfs 0.319 af			
Link L4-4: L4-4	Inflow=3.29 cfs 0.578 af			
	Primary=3.29 cfs 0.578 af			
LINK L4-5: L4-5	Inflow=4.94 cfs 0.897 af			
	Filinaly=4.94 Cis 0.097 al			
Link   5-3:   5-3	Inflow=22.75 cfs 2.801 af			
	Primary=22.75 cfs 2.801 af			
	,			
Link L6-2: L6-2	Inflow=0.12 cfs 0.017 af			
	Primary=0.12 cfs 0.017 af			
Total Runoff Area = 323.908 ac Runoff V	olume = 21.771 af Average Runoff Depth = 0.81"			
96.72% Perv	ous = 313.275  ac 3.28% impervious = 10.633 ac			

Kittery\_NoBuild (ID 2853676)ME-DennettRoad 24-hr S1 2-yrRainfall=3.31"Prepared by Barton & Loguidice, DPCPrinted8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 12

Subcatchment 1-1: DA-1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth=1.37" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=28.60 cfs 4.677 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth=1.53" Flow Length=485' Tc=27.5 min CN=WQ Runoff=19.60 cfs 2.864 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth=2.02" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=29.78 cfs 3.403 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth=1.95" Flow Length=875' Tc=18.9 min CN=WQ Runoff=25.03 cfs 3.003 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth=1.22" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=27.33 cfs 6.908 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth=0.40" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=7.36 cfs 1.752 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth=1.53" Flow Length=850' Tc=25.6 min CN=WQ Runoff=6.66 cfs 0.949 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth=1.63" Flow Length=745' Tc=17.7 min CN=WQ Runoff=10.02 cfs 1.170 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth=0.89" Flow Length=688' Tc=33.2 min CN=WQ Runoff=3.74 cfs 0.629 af
Subcatchment 3-1-P: DA-3-	I-P Runoff Area=1.790 ac 100.00% Impervious Runoff Depth>3.08" Tc=0.0 min CN=98 Runoff=7.12 cfs 0.459 af
Subcatchment 3-2: DA-3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth=0.51" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=7.99 cfs 1.099 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth=0.39" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=2.65 cfs 0.507 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth=1.04" Flow Length=955' Tc=37.6 min CN=WQ Runoff=4.91 cfs 0.872 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth=1.33" Flow Length=625' Tc=33.8 min CN=WQ Runoff=6.89 cfs 1.150 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth=2.47" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=30.49 cfs 2.709 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth=2.61" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.16 cfs 0.023 af

Kittery_NoBuild (ID 28	53676) ME-DennettRoad 24-hr S1 2-yr Rainfall=3.31"
Prepared by Barton & Lo	Printed 8/7/2023
HydroCAD® 10.20-2g s/n 05	255 © 2022 HydroCAD Software Solutions LLC Page 13
Reach R1-1: R1-1	Avg. Flow Depth=1.17' Max Vel=3.72 fps Inflow=28.60 cfs 4.677 af n=0.035 L=800.0' S=0.0117 '/' Capacity=90.60 cfs Outflow=28.18 cfs 4.677 af
Reach R1-2: R1-2	Avg. Flow Depth=0.12' Max Vel=2.12 fps Inflow=0.90 cfs 1.416 af n=0.035 L=20.0' S=0.0465 '/' Capacity=180.91 cfs Outflow=0.90 cfs 1.416 af
Reach R1-3: R-1-3	Avg. Flow Depth=1.80' Max Vel=1.91 fps Inflow=45.68 cfs 9.496 af n=0.040 L=225.0' S=0.0022 '/' Capacity=56.74 cfs Outflow=45.49 cfs 9.490 af
Reach R1-4: R1-4	Avg. Flow Depth=0.92' Max Vel=4.70 fps Inflow=25.03 cfs 3.003 af n=0.035 L=290.0' S=0.0241 '/' Capacity=130.35 cfs Outflow=24.92 cfs 3.003 af
Reach R1-5: R1-5	Inflow=45.49 cfs 9.490 af Outflow=45.49 cfs 9.490 af
Reach R2-1: R2-1	Avg. Flow Depth=0.53' Max Vel=2.02 fps Inflow=4.87 cfs 7.303 af n=0.035 L=460.0' S=0.0082 '/' Capacity=76.05 cfs Outflow=4.87 cfs 7.291 af
Reach R2-2: R2-2	Avg. Flow Depth=0.73' Max Vel=3.97 fps Inflow=10.02 cfs 1.170 af n=0.030 L=175.0' S=0.0171 '/' Capacity=82.75 cfs Outflow=10.01 cfs 1.170 af
Reach R2-3: R2-3	Avg. Flow Depth=0.71' Max Vel=3.13 fps Inflow=7.63 cfs 8.240 af n=0.030 L=410.0' S=0.0110 '/' Capacity=66.21 cfs Outflow=7.58 cfs 8.232 af
Reach R3-1: R3-1	Avg. Flow Depth=0.40' Max Vel=3.12 fps Inflow=3.74 cfs 0.629 af n=0.030 L=1,596.0' S=0.0203 '/' Capacity=89.94 cfs Outflow=3.48 cfs 0.629 af
Reach R3-P: R3-P	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.040 L=276.0' S=0.0056 '/' Capacity=3.77 cfs Outflow=0.00 cfs 0.000 af
Reach R4-1: R4-1	Avg. Flow Depth=0.25' Max Vel=4.26 fps Inflow=2.65 cfs 0.507 af n=0.030 L=95.0' S=0.0632 '/' Capacity=158.84 cfs Outflow=2.65 cfs 0.507 af
Reach R4-2: R4-2	Avg. Flow Depth=0.34' Max Vel=5.38 fps Inflow=4.91 cfs 0.872 af n=0.030 L=140.0' S=0.0714 '/' Capacity=168.92 cfs Outflow=4.91 cfs 0.872 af
Reach R5-1: R5-1	Avg. Flow Depth=0.48' Max Vel=3.23 fps Inflow=6.89 cfs 1.150 af n=0.035 L=640.0' S=0.0234 '/' Capacity=325.19 cfs Outflow=6.81 cfs 1.150 af
Reach R5-2: R5-2	Avg. Flow Depth=1.29' Max Vel=3.36 fps Inflow=32.56 cfs 3.859 af n=0.035 L=700.0' S=0.0086 '/' Capacity=77.67 cfs Outflow=29.67 cfs 3.859 af
Reach R6-1: R6-1	Avg. Flow Depth=0.07' Max Vel=1.10 fps Inflow=0.16 cfs 0.023 af n=0.030 L=360.0' S=0.0203 '/' Capacity=90.06 cfs Outflow=0.15 cfs 0.023 af
Pond 1-P:	Peak Elev=38.61' Storage=2.087 af Inflow=19.60 cfs 2.864 af 12.0" Round Culvert n=0.012 L=23.0' S=0.0000 '/' Outflow=0.90 cfs 1.416 af
Pond 2-P:	Peak Elev=36.86' Storage=4.469 af Inflow=34.51 cfs 8.660 af Outflow=4.87 cfs 7.303 af

<b>Kittery_NoBuild (ID 285367</b> Prepared by Barton & Loguidic	6) ME-DennettRoad 24-hr S1 2-yr Rainfall=3.31" ce, DPC Printed 8/7/2023	
HydroCAD® 10.20-2g S/1105255 @	2022 HydroCAD Software Solutions LLC Page 14	
Pond 3-P:	Peak Elev=86.32' Storage=0.459 af Inflow=7.12 cfs 0.459 af Outflow=0.00 cfs 0.000 af	
Pond CB-1-1: L1-1	Peak Elev=121.15' Inflow=28.60 cfs 4.677 af 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=28.60 cfs 4.677 af	
Pond CB-1-3: L1-3	Peak Elev=40.80' Inflow=45.68 cfs 9.496 af 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=45.68 cfs 9.496 af	
Pond CB-1-4: L1-4	Peak Elev=41.39' Inflow=25.03 cfs 3.003 af 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=25.03 cfs 3.003 af	
Pond CB-2-3: L2-2	Peak Elev=31.05' Inflow=7.63 cfs 8.240 af 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=7.63 cfs 8.240 af	
Pond CB-2-4: L2-3	Peak Elev=32.02' Inflow=10.02 cfs 1.170 af 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=10.02 cfs 1.170 af	
Pond CB-3-1: L3-1	Peak Elev=87.48' Inflow=3.74 cfs 0.629 af Outflow=3.74 cfs 0.629 af	
Pond CB-3-2: L3-2	Peak Elev=55.44' Inflow=9.84 cfs 1.728 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=9.84 cfs 1.728 af	
Pond CB-4-1: L4-1	Peak Elev=54.68' Inflow=2.65 cfs 0.507 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=2.65 cfs 0.507 af	
Pond CB-4-2: L4-2	Peak Elev=54.95' Inflow=4.91 cfs 0.872 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=4.91 cfs 0.872 af	
Pond CB-5-1: L5-1	Peak Elev=56.99' Inflow=6.89 cfs 1.150 af 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=6.89 cfs 1.150 af	
Pond CB-5-2: L5-2	Peak Elev=42.80' Inflow=32.56 cfs 3.859 af 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=32.56 cfs 3.859 af	
Pond CB-6-1: L6-1	Peak Elev=49.42' Inflow=0.16 cfs 0.023 af 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.16 cfs 0.023 af	
Link L1-5: L1-5	Inflow=45.49 cfs 9.490 af Primary=45.49 cfs 9.490 af	
Link L1-6: L1-6	Inflow=69.47 cfs 12.494 af Primary=69.47 cfs 12.494 af	
Link L2-1: L2-1	Inflow=4.87 cfs 7.303 af Primary=4.87 cfs 7.303 af	
Link L2-4: L2-4	Inflow=7.58 cfs 8.232 af Primary=7.58 cfs 8.232 af	

Kittery_NoBuild (ID 2853676)	ME-DennettRoad	d 24-hr S1 2-yr Rainfall=3.31"
Prepared by Barton & Loguidice, DPC		Printed 8/7/2023
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Link L2-5: L2-5		Inflow=10.01 cfs 1.170 af
		Primary=10.01 cfs 1.170 af
Link L2-6: L2-6		Inflow=16.53 cfs 9.402 af
		Primary=16.53 cfs 9.402 af
Link L4-3: L4-3		Inflow=2.65 cfs 0.507 af
		Primary=2.65 cfs 0.507 af
Link L4-4: L4-4		Inflow=4.91 cfs 0.872 af
		Primary=4.91 cfs 0.872 af
Link L4-5: L4-5		Inflow=7.54 cfs 1.379 af
		Primary=7.54 cfs 1.379 af
Link L5-3: L5-3		Inflow=29.67 cfs 3.859 af
		Primary=29.67 cfs 3.859 af
Link L6-2: L6-2		Inflow=0.15 cfs 0.023 af
		Primary=0.15 cfs 0.023 af
Total Runoff Area = 323 908 ac	Runoff Volume = 32,175 af	Average Runoff Depth = 1.19"

Total Runoff Area = 323.908 acRunoff Volume = 32.175 afAverage Runoff Depth = 1.19"96.72% Pervious = 313.275 ac3.28% Impervious = 10.633 ac

Kittery\_NoBuild (ID 2853676)ME-DennettRoad 24-hr S1 5-yrRainfall=4.41"Prepared by Barton & Loguidice, DPCPrinted 8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 16

Subcatchment 1-1: DA-1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth=2.22" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=45.59 cfs 7.569 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth=2.42" Flow Length=485' Tc=27.5 min CN=WQ Runoff=30.45 cfs 4.544 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth=2.99" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=42.44 cfs 5.039 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth=2.91" Flow Length=875' Tc=18.9 min CN=WQ Runoff=36.03 cfs 4.483 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth=1.99" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=43.79 cfs 11.228 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth=0.66" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=11.99 cfs 2.882 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth=2.41" Flow Length=850' Tc=25.6 min CN=WQ Runoff=10.32 cfs 1.499 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth=2.53" Flow Length=745' Tc=17.7 min CN=WQ Runoff=15.17 cfs 1.813 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth=1.51" Flow Length=688' Tc=33.2 min CN=WQ Runoff=6.21 cfs 1.061 af
Subcatchment 3-1-P: DA-3-	I-P Runoff Area=1.790 ac 100.00% Impervious Runoff Depth>4.17" Tc=0.0 min CN=98 Runoff=9.15 cfs 0.622 af
Subcatchment 3-2: DA-3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth=0.91" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=14.21 cfs 1.940 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth=0.65" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=4.45 cfs 0.855 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth=1.67" Flow Length=955' Tc=37.6 min CN=WQ Runoff=7.75 cfs 1.396 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth=2.09" Flow Length=625' Tc=33.8 min CN=WQ Runoff=10.62 cfs 1.808 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth=3.51" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=41.14 cfs 3.847 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth=3.67" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.21 cfs 0.032 af

Kittery_NoBuild (ID 2853	676) ME-DennettRoad 24-hr S1 5-yr Rainfall=4.41"
Prepared by Barton & Logui	dice, DPC Printed 8/7/2023
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Reach R1-1: R1-1	Avg. Flow Depth=1.46' Max Vel=4.21 fps Inflow=45.59 cfs 7.569 af n=0.035 L=800.0' S=0.0117 '/' Capacity=90.60 cfs Outflow=45.09 cfs 7.569 af
Reach R1-2: R1-2	Avg. Flow Depth=0.18' Max Vel=2.61 fps Inflow=1.63 cfs 2.603 af n=0.035 L=20.0' S=0.0465 '/' Capacity=180.91 cfs Outflow=1.63 cfs 2.603 af
Reach R1-3: R-1-3	Avg. Flow Depth=2.22' Max Vel=2.13 fps Inflow=70.22 cfs 15.212 af n=0.040 L=225.0' S=0.0022 '/' Capacity=56.74 cfs Outflow=70.01 cfs 15.204 af
Reach R1-4: R1-4	Avg. Flow Depth=1.10' Max Vel=5.18 fps Inflow=36.03 cfs 4.483 af n=0.035 L=290.0' S=0.0241 '/' Capacity=130.35 cfs Outflow=35.90 cfs 4.483 af
Reach R1-5: R1-5	Inflow=70.01 cfs 15.204 af Outflow=70.01 cfs 15.204 af
Reach R2-1: R2-1	Avg. Flow Depth=0.64' Max Vel=2.26 fps Inflow=7.18 cfs 11.555 af n=0.035 L=460.0' S=0.0082 '/' Capacity=76.05 cfs Outflow=7.18 cfs 11.538 af
Reach R2-2: R2-2	Avg. Flow Depth=0.90' Max Vel=4.44 fps Inflow=15.17 cfs 1.813 af n=0.030 L=175.0' S=0.0171 '/' Capacity=82.75 cfs Outflow=15.14 cfs 1.813 af
Reach R2-3: R2-3	Avg. Flow Depth=0.89' Max Vel=3.53 fps Inflow=11.88 cfs 13.036 af n=0.030 L=410.0' S=0.0110 '/' Capacity=66.21 cfs Outflow=11.82 cfs 13.025 af
Reach R3-1: R3-1	Avg. Flow Depth=0.53' Max Vel=3.64 fps Inflow=6.21 cfs 1.061 af n=0.030 L=1,596.0' S=0.0203 '/' Capacity=89.94 cfs Outflow=5.87 cfs 1.061 af
Reach R3-P: R3-P	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.040 L=276.0' S=0.0056 '/' Capacity=3.77 cfs Outflow=0.00 cfs 0.000 af
Reach R4-1: R4-1	Avg. Flow Depth=0.33' Max Vel=5.00 fps Inflow=4.45 cfs 0.855 af n=0.030 L=95.0' S=0.0632 '/' Capacity=158.84 cfs Outflow=4.45 cfs 0.855 af
Reach R4-2: R4-2	Avg. Flow Depth=0.44' Max Vel=6.17 fps Inflow=7.75 cfs 1.396 af n=0.030 L=140.0' S=0.0714 '/' Capacity=168.92 cfs Outflow=7.75 cfs 1.396 af
Reach R5-1: R5-1	Avg. Flow Depth=0.60' Max Vel=3.66 fps Inflow=10.62 cfs 1.808 af n=0.035 L=640.0' S=0.0234 '/' Capacity=325.19 cfs Outflow=10.53 cfs 1.808 af
Reach R5-2: R5-2	Avg. Flow Depth=1.50' Max Vel=3.66 fps Inflow=44.67 cfs 5.655 af n=0.035 L=700.0' S=0.0086 '/' Capacity=77.67 cfs Outflow=41.23 cfs 5.655 af
Reach R6-1: R6-1	Avg. Flow Depth=0.08' Max Vel=1.22 fps Inflow=0.21 cfs 0.032 af n=0.030 L=360.0' S=0.0203 '/' Capacity=90.06 cfs Outflow=0.21 cfs 0.032 af
Pond 1-P:	Peak Elev=38.91' Storage=3.081 af Inflow=30.45 cfs 4.544 af 12.0" Round Culvert n=0.012 L=23.0' S=0.0000 '/' Outflow=1.63 cfs 2.603 af
Pond 2-P:	Peak Elev=37.47' Storage=7.674 af Inflow=55.64 cfs 14.110 af Outflow=7.18 cfs 11.555 af

Kittery_NoBuild (ID Prepared by Barton &	2853676) ME-DennettRoad 24-hr S1 5-yr Rainfall=4.41" Loguidice, DPC Printed 8/7/2023
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Pond 3-P:	Peak Elev=86.43' Storage=0.622 af Inflow=9.15 cfs 0.622 af Outflow=0.00 cfs 0.000 af
Pond CB-1-1: L1-1	Peak Elev=234.94' Inflow=45.59 cfs 7.569 af 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=45.59 cfs 7.569 af
Pond CB-1-3: L1-3	Peak Elev=42.54' Inflow=70.22 cfs 15.212 af 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=70.22 cfs 15.212 af
Pond CB-1-4: L1-4	Peak Elev=46.79' Inflow=36.03 cfs 4.483 af 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=36.03 cfs 4.483 af
Pond CB-2-3: L2-2	Peak Elev=31.33' Inflow=11.88 cfs 13.036 af 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=11.88 cfs 13.036 af
Pond CB-2-4: L2-3	Peak Elev=32.83' Inflow=15.17 cfs 1.813 af 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=15.17 cfs 1.813 af
Pond CB-3-1: L3-1	Peak Elev=87.86' Inflow=6.21 cfs 1.061 af Outflow=6.21 cfs 1.061 af
Pond CB-3-2: L3-2	Peak Elev=56.35' Inflow=17.59 cfs 3.001 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=17.59 cfs 3.001 af
Pond CB-4-1: L4-1	Peak Elev=54.90' Inflow=4.45 cfs 0.855 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=4.45 cfs 0.855 af
Pond CB-4-2: L4-2	Peak Elev=55.24' Inflow=7.75 cfs 1.396 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=7.75 cfs 1.396 af
Pond CB-5-1: L5-1	Peak Elev=57.25' Inflow=10.62 cfs 1.808 af 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=10.62 cfs 1.808 af
Pond CB-5-2: L5-2	Peak Elev=44.09' Inflow=44.67 cfs 5.655 af 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=44.67 cfs 5.655 af
Pond CB-6-1: L6-1	Peak Elev=49.45' Inflow=0.21 cfs 0.032 af 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.21 cfs 0.032 af
Link L1-5: L1-5	Inflow=70.01 cfs 15.204 af Primary=70.01 cfs 15.204 af
Link L1-6: L1-6	Inflow=104.12 cfs 19.688 af Primary=104.12 cfs 19.688 af
Link L2-1: L2-1	Inflow=7.18 cfs 11.555 af Primary=7.18 cfs 11.555 af
Link L2-4: L2-4	Inflow=11.82 cfs 13.025 af Primary=11.82 cfs 13.025 af

Kittery_NoBuild (ID 2853676)	ME-DennettRoad 24-hr S1	5-yr Rainfall=4.41"
Prepared by Barton & Loguidice, DPC		Printed 8/7/2023
HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD	Software Solutions LLC	Page 19
Link L2-5: L2-5	Infl	ow=15.14 cfs 1.813 af
	Prim	ary=15.14 cfs 1.813 af
Link L2-6: L2-6	Inflo	w=25.39 cfs 14.838 af
	Prima	ry=25.39 cfs 14.838 af
Link L4-3: L4-3	Ir	flow=4.45 cfs 0.855 af
	Prir	nary=4.45 cfs 0.855 af
Link L4-4: L4-4	Ir	flow=7.75 cfs 1.396 af
	Prir	nary=7.75 cfs 1.396 af
Link L4-5: L4-5	Infl	ow=12.18 cfs 2.252 af
	Prim	ary=12.18 cfs 2.252 af
Link L5-3: L5-3	Infl	ow=41.23 cfs 5.655 af
	Prim	ary=41.23 cfs 5.655 af
Link L6-2: L6-2	Ir	nflow=0.21 cfs 0.032 af
	Prir	nary=0.21 cfs 0.032 af
Total Runoff Area - 323 908 ac	Runoff Volume - 50 620 af Average R	Runoff Denth - 1 88"

Total Runoff Area = 323.908 acRunoff Volume = 50.620 afAverage Runoff Depth = 1.88"96.72% Pervious = 313.275 ac3.28% Impervious = 10.633 ac

Kittery\_NoBuild (ID 2853676)ME-DennettRoad 24-hr S1 10-yrRainfall=5.32"Prepared by Barton & Loguidice, DPCPrinted 8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 20

Subcatchment 1-1: DA-1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth=2.97" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=59.93 cfs 10.129 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth=3.21" Flow Length=485' Tc=27.5 min CN=WQ Runoff=39.53 cfs 6.018 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth=3.82" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=52.87 cfs 6.442 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth=3.74" Flow Length=875' Tc=18.9 min CN=WQ Runoff=45.11 cfs 5.756 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth=2.67" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=57.79 cfs 15.075 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth=0.90" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=15.92 cfs 3.944 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth=3.19" Flow Length=850' Tc=25.6 min CN=WQ Runoff=13.40 cfs 1.982 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth=3.31" Flow Length=745' Tc=17.7 min CN=WQ Runoff=19.48 cfs 2.376 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth=2.07" Flow Length=688' Tc=33.2 min CN=WQ Runoff=8.38 cfs 1.462 af
Subcatchment 3-1-P: DA-3-	I-P Runoff Area=1.790 ac 100.00% Impervious Runoff Depth>5.07"   Tc=0.0 min CN=98 Runoff=10.82 cfs 0.757 af
Subcatchment 3-2: DA-3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth=1.29" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=20.07 cfs 2.759 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth=0.91" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=6.02 cfs 1.187 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth=2.23" Flow Length=955' Tc=37.6 min CN=WQ Runoff=10.15 cfs 1.864 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth=2.76" Flow Length=625' Tc=33.8 min CN=WQ Runoff=13.75 cfs 2.388 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth=4.38" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=49.83 cfs 4.804 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth=4.56" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.26 cfs 0.040 af

Kittery_NoBuild (ID 28536	76) ME-DennettRoad 24-hr S1 10-yr Rainfall=5.32"
Prepared by Barton & Loguid	ice, DPC Printed 8/7/2023
HydrocAD® 10.20-2g S/1103235	© 2022 Hydrocad Software Solutions ELC Page 21
Reach R1-1: R1-1	Avg. Flow Depth=1.65' Max Vel=4.52 fps Inflow=59.93 cfs 10.129 af n=0.035 L=800.0' S=0.0117 '/' Capacity=90.60 cfs Outflow=59.37 cfs 10.129 af
Reach R1-2: R1-2	Avg. Flow Depth=0.21' Max Vel=2.90 fps Inflow=2.23 cfs 3.658 af n=0.035 L=20.0' S=0.0465 '/' Capacity=180.91 cfs Outflow=2.23 cfs 3.658 af
Reach R1-3: R-1-3	Avg. Flow Depth=2.57' Max Vel=2.25 fps Inflow=90.97 cfs 20.230 af n=0.040 L=225.0' S=0.0022 '/' Capacity=56.74 cfs Outflow=90.72 cfs 20.221 af
Reach R1-4: R1-4	Avg. Flow Depth=1.23' Max Vel=5.50 fps Inflow=45.11 cfs 5.756 af n=0.035 L=290.0' S=0.0241 '/' Capacity=130.35 cfs Outflow=44.96 cfs 5.756 af
Reach R1-5: R1-5	Inflow=90.72 cfs 20.221 af Outflow=90.72 cfs 20.221 af
Reach R2-1: R2-1	Avg. Flow Depth=0.77' Max Vel=2.48 fps Inflow=10.06 cfs 15.098 af n=0.035 L=460.0' S=0.0082 '/' Capacity=76.05 cfs Outflow=10.06 cfs 15.075 af
Reach R2-2: R2-2	Avg. Flow Depth=1.02' Max Vel=4.75 fps Inflow=19.48 cfs 2.376 af n=0.030 L=175.0' S=0.0171 '/' Capacity=82.75 cfs Outflow=19.45 cfs 2.376 af
Reach R2-3: R2-3	Avg. Flow Depth=1.02' Max Vel=3.80 fps Inflow=15.72 cfs 17.057 af n=0.030 L=410.0' S=0.0110 '/' Capacity=66.21 cfs Outflow=15.65 cfs 17.042 af
Reach R3-1: R3-1	Avg. Flow Depth=0.62' Max Vel=3.96 fps Inflow=8.38 cfs 1.462 af n=0.030 L=1,596.0' S=0.0203 '/' Capacity=89.94 cfs Outflow=7.98 cfs 1.462 af
Reach R3-P: R3-P	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.040 L=276.0' S=0.0056 '/' Capacity=3.77 cfs Outflow=0.00 cfs 0.000 af
Reach R4-1: R4-1	Avg. Flow Depth=0.39' Max Vel=5.48 fps Inflow=6.02 cfs 1.187 af n=0.030 L=95.0' S=0.0632 '/' Capacity=158.84 cfs Outflow=6.02 cfs 1.187 af
Reach R4-2: R4-2	Avg. Flow Depth=0.51' Max Vel=6.67 fps Inflow=10.15 cfs 1.864 af n=0.030 L=140.0' S=0.0714 '/' Capacity=168.92 cfs Outflow=10.15 cfs 1.864 af
Reach R5-1: R5-1	Avg. Flow Depth=0.69' Max Vel=3.94 fps Inflow=13.75 cfs 2.388 af n=0.035 L=640.0' S=0.0234 '/' Capacity=325.19 cfs Outflow=13.66 cfs 2.388 af
Reach R5-2: R5-2	Avg. Flow Depth=1.65' Max Vel=3.87 fps Inflow=54.63 cfs 7.192 af n=0.035 L=700.0' S=0.0086 '/' Capacity=77.67 cfs Outflow=50.77 cfs 7.192 af
Reach R6-1: R6-1	Avg. Flow Depth=0.09' Max Vel=1.31 fps Inflow=0.26 cfs 0.040 af n=0.030 L=360.0' S=0.0203 '/' Capacity=90.06 cfs Outflow=0.25 cfs 0.040 af
Pond 1-P:	Peak Elev=39.17' Storage=3.987 af Inflow=39.53 cfs 6.018 af 12.0" Round Culvert n=0.012 L=23.0' S=0.0000 '/' Outflow=2.23 cfs 3.658 af
Pond 2-P:	Peak Elev=38.02' Storage=10.594 af Inflow=73.53 cfs 19.019 af Outflow=10.06 cfs 15.098 af

Kittery_NoBuild (ID 285367 Prepared by Barton & Loguidie	(6)ME-DennettRoad 24-hr S1 10-yr Rainfall=5.32"ce, DPCPrinted 8/7/202300000 Ukdas 0AD Settures Salutions U.S.December 200
HydroCAD® 10.20-2g s/n 05255 @	2022 HydroCAD Software Solutions LLC Page 22
Pond 3-P:	Peak Elev=86.52' Storage=0.757 af Inflow=10.82 cfs 0.757 af Outflow=0.00 cfs 0.000 af
Pond CB-1-1: L1-1	Peak Elev=371.42' Inflow=59.93 cfs 10.129 af 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=59.93 cfs 10.129 af
Pond CB-1-3: L1-3	Peak Elev=44.40' Inflow=90.97 cfs 20.230 af 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=90.97 cfs 20.230 af
Pond CB-1-4: L1-4	Peak Elev=53.48' Inflow=45.11 cfs 5.756 af 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=45.11 cfs 5.756 af
Pond CB-2-3: L2-2	Peak Elev=31.56' Inflow=15.72 cfs 17.057 af 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=15.72 cfs 17.057 af
Pond CB-2-4: L2-3	Peak Elev=33.37' Inflow=19.48 cfs 2.376 af 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=19.48 cfs 2.376 af
Pond CB-3-1: L3-1	Peak Elev=88.29' Inflow=8.38 cfs 1.462 af Outflow=8.38 cfs 1.462 af
Pond CB-3-2: L3-2	Peak Elev=57.68' Inflow=24.79 cfs 4.221 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=24.79 cfs 4.221 af
Pond CB-4-1: L4-1	Peak Elev=55.07' Inflow=6.02 cfs 1.187 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=6.02 cfs 1.187 af
Pond CB-4-2: L4-2	Peak Elev=55.46' Inflow=10.15 cfs 1.864 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=10.15 cfs 1.864 af
Pond CB-5-1: L5-1	Peak Elev=57.44' Inflow=13.75 cfs 2.388 af 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=13.75 cfs 2.388 af
Pond CB-5-2: L5-2	Peak Elev=45.46' Inflow=54.63 cfs 7.192 af 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=54.63 cfs 7.192 af
Pond CB-6-1: L6-1	Peak Elev=49.47' Inflow=0.26 cfs 0.040 af 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.26 cfs 0.040 af
Link L1-5: L1-5	Inflow=90.72 cfs 20.221 af Primary=90.72 cfs 20.221 af
Link L1-6: L1-6	Inflow=133.07 cfs 25.977 af Primary=133.07 cfs 25.977 af
Link L2-1: L2-1	Inflow=10.06 cfs 15.098 af Primary=10.06 cfs 15.098 af
Link L2-4: L2-4	Inflow=15.65 cfs 17.042 af Primary=15.65 cfs 17.042 af

Kittery_NoBuild (ID 2853676)	ME-DennettRoad 24-hr S1 10-yr Rainfall=5.32"
Prepared by Barton & Loguidice, DPC	Printed 8/7/2023
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Link L2-5: L2-5	Inflow=19.45 cfs 2.376 af Primary=19.45 cfs 2.376 af
l ink   2-6   2-6	Inflow=33 10 cfs 19 418 af
	Primary=33.10 cfs 19.418 af
Link L4-3: L4-3	Inflow=6.02 cfs 1.187 af Primary=6.02 cfs 1.187 af
Link L4-4: L4-4	Inflow=10.15 cfs 1.864 af Primary=10.15 cfs 1.864 af
Link L4-5: L4-5	Inflow=16.15 cfs 3.052 af Primary=16.15 cfs 3.052 af
Link L5-3: L5-3	Inflow=50.77 cfs 7.192 af Primary=50.77 cfs 7.192 af
Link L6-2: L6-2	Inflow=0.25 cfs 0.040 af Primary=0.25 cfs 0.040 af
Total Runoff Area = 323.908 ac Runoff 96.72% Per	Volume = 66.985 af Average Runoff Depth = 2.48"   vious = 313.275 ac 3.28% Impervious = 10.633 ac

Kittery\_NoBuild (ID 2853676)ME-DennettRoad 24-hr S1 25-yrRainfall=6.58"Prepared by Barton & Loguidice, DPCPrinted8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 24

Subcatchment 1-1: DA-1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth=4.06" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=79.98 cfs 13.839 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth=4.34" Flow Length=485' Tc=27.5 min CN=WQ Runoff=52.17 cfs 8.139 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth=5.00" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=67.26 cfs 8.430 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth=4.91" Flow Length=875' Tc=18.9 min CN=WQ Runoff=57.66 cfs 7.563 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth=3.66" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=77.50 cfs 20.712 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth=1.32" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=21.47 cfs 5.796 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth=4.31" Flow Length=850' Tc=25.6 min CN=WQ Runoff=17.71 cfs 2.679 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth=4.44" Flow Length=745' Tc=17.7 min CN=WQ Runoff=25.50 cfs 3.184 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth=2.93" Flow Length=688' Tc=33.2 min CN=WQ Runoff=11.56 cfs 2.067 af
Subcatchment 3-1-P: DA-3-	I-P Runoff Area=1.790 ac 100.00% Impervious Runoff Depth>6.32" Tc=0.0 min CN=98 Runoff=13.11 cfs 0.943 af
Subcatchment 3-2: DA-3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth=1.90" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=28.92 cfs 4.082 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth=1.35" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=8.29 cfs 1.763 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth=3.07" Flow Length=955' Tc=37.6 min CN=WQ Runoff=13.51 cfs 2.564 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth=3.75" Flow Length=625' Tc=33.8 min CN=WQ Runoff=18.13 cfs 3.236 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth=5.60" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=61.74 cfs 6.144 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth=5.80" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.31 cfs 0.051 af

Kittery_NoBuild (ID 28	ME-DennettRoad 24-hr S1 25-yr Rainfall=6.58"
Prepared by Barton & Lo	pguidice, DPC Printed 8/7/2023
HydroCAD® 10.20-2g S/n 05	2255 © 2022 HydroCAD Software Solutions LLC Page 25
Reach R1-1: R1-1	Avg. Flow Depth=1.88' Max Vel=4.86 fps Inflow=79.98 cfs 13.839 af n=0.035 L=800.0' S=0.0117 '/' Capacity=90.60 cfs Outflow=79.33 cfs 13.839 af
Reach R1-2: R1-2	Avg. Flow Depth=0.26' Max Vel=3.25 fps Inflow=3.19 cfs 5.171 af n=0.035 L=20.0' S=0.0465 '/' Capacity=180.91 cfs Outflow=3.19 cfs 5.171 af
Reach R1-3: R-1-3	Avg. Flow Depth=3.05' Max Vel=2.35 fps Inflow=120.11 cfs 27.440 af n=0.040 L=225.0' S=0.0022 '/' Capacity=56.74 cfs Outflow=119.78 cfs 27.429 af
Reach R1-4: R1-4	Avg. Flow Depth=1.38' Max Vel=5.86 fps Inflow=57.66 cfs 7.563 af n=0.035 L=290.0' S=0.0241 '/' Capacity=130.35 cfs Outflow=57.48 cfs 7.563 af
Reach R1-5: R1-5	Inflow=119.78 cfs 27.429 af Outflow=119.78 cfs 27.429 af
Reach R2-1: R2-1	Avg. Flow Depth=1.44' Max Vel=3.50 fps Inflow=36.82 cfs 22.164 af n=0.035 L=460.0' S=0.0082 '/' Capacity=76.05 cfs Outflow=36.79 cfs 22.139 af
Reach R2-2: R2-2	Avg. Flow Depth=1.16' Max Vel=5.09 fps Inflow=25.50 cfs 3.184 af n=0.030 L=175.0' S=0.0171 '/' Capacity=82.75 cfs Outflow=25.45 cfs 3.184 af
Reach R2-3: R2-3	Avg. Flow Depth=1.57' Max Vel=4.82 fps Inflow=39.05 cfs 24.818 af n=0.030 L=410.0' S=0.0110 '/' Capacity=66.21 cfs Outflow=39.04 cfs 24.802 af
Reach R3-1: R3-1	Avg. Flow Depth=0.74' Max Vel=4.34 fps Inflow=11.56 cfs 2.067 af n=0.030 L=1,596.0' S=0.0203 '/' Capacity=89.94 cfs Outflow=11.09 cfs 2.067 af
Reach R3-P: R3-P	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.040 L=276.0' S=0.0056 '/' Capacity=3.77 cfs Outflow=0.00 cfs 0.000 af
Reach R4-1: R4-1	Avg. Flow Depth=0.47' Max Vel=6.02 fps Inflow=8.29 cfs 1.763 af n=0.030 L=95.0' S=0.0632 '/' Capacity=158.84 cfs Outflow=8.29 cfs 1.763 af
Reach R4-2: R4-2	Avg. Flow Depth=0.59' Max Vel=7.23 fps Inflow=13.51 cfs 2.564 af n=0.030 L=140.0' S=0.0714 '/' Capacity=168.92 cfs Outflow=13.51 cfs 2.564 af
Reach R5-1: R5-1	Avg. Flow Depth=0.79' Max Vel=4.26 fps Inflow=18.13 cfs 3.236 af n=0.035 L=640.0' S=0.0234 '/' Capacity=325.19 cfs Outflow=18.03 cfs 3.236 af
Reach R5-2: R5-2	Avg. Flow Depth=1.83' Max Vel=4.10 fps Inflow=68.34 cfs 9.380 af n=0.035 L=700.0' S=0.0086 '/' Capacity=77.67 cfs Outflow=64.06 cfs 9.380 af
Reach R6-1: R6-1	Avg. Flow Depth=0.10' Max Vel=1.41 fps Inflow=0.31 cfs 0.051 af n=0.030 L=360.0' S=0.0203 '/' Capacity=90.06 cfs Outflow=0.31 cfs 0.051 af
Pond 1-P:	Peak Elev=39.56' Storage=5.294 af Inflow=52.17 cfs 8.139 af 12.0" Round Culvert n=0.012 L=23.0' S=0.0000 '/' Outflow=3.19 cfs 5.171 af
Pond 2-P:	Peak Elev=38.17' Storage=11.654 af Inflow=98.72 cfs 26.508 af Outflow=36.82 cfs 22.164 af

Kittery_NoBuild (ID 28536 Prepared by Barton & Loguid	ME-DennettRoad 24-hr S1 25-yr Rainfall=6.58" dice, DPC Printed 8/7/2023
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Pond 3-P:	Peak Elev=86.65' Storage=0.943 af Inflow=13.11 cfs 0.943 af Outflow=0.00 cfs 0.000 af
Pond CB-1-1: L1-1	Peak Elev=624.27' Inflow=79.98 cfs 13.839 af 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=79.98 cfs 13.839 af
Pond CB-1-3: L1-3	Peak Elev=47.84' Inflow=120.11 cfs 27.440 af 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=120.11 cfs 27.440 af
Pond CB-1-4: L1-4	Peak Elev=65.19' Inflow=57.66 cfs 7.563 af 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=57.66 cfs 7.563 af
Pond CB-2-3: L2-2	Peak Elev=32.80' Inflow=39.05 cfs 24.818 af 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=39.05 cfs 24.818 af
Pond CB-2-4: L2-3	Peak Elev=34.35' Inflow=25.50 cfs 3.184 af 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=25.50 cfs 3.184 af
Pond CB-3-1: L3-1	Peak Elev=89.17' Inflow=11.56 cfs 2.067 af Outflow=11.56 cfs 2.067 af
Pond CB-3-2: L3-2	Peak Elev=60.56' Inflow=35.65 cfs 6.149 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=35.65 cfs 6.149 af
Pond CB-4-1: L4-1	Peak Elev=55.29' Inflow=8.29 cfs 1.763 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=8.29 cfs 1.763 af
Pond CB-4-2: L4-2	Peak Elev=55.79' Inflow=13.51 cfs 2.564 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=13.51 cfs 2.564 af
Pond CB-5-1: L5-1	Peak Elev=57.69' Inflow=18.13 cfs 3.236 af 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=18.13 cfs 3.236 af
Pond CB-5-2: L5-2	Peak Elev=47.79' Inflow=68.34 cfs 9.380 af 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=68.34 cfs 9.380 af
Pond CB-6-1: L6-1	Peak Elev=49.49' Inflow=0.31 cfs 0.051 af 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.31 cfs 0.051 af
Link L1-5: L1-5	Inflow=119.78 cfs 27.429 af Primary=119.78 cfs 27.429 af
Link L1-6: L1-6	Inflow=173.53 cfs 34.991 af Primary=173.53 cfs 34.991 af
Link L2-1: L2-1	Inflow=36.82 cfs 22.164 af Primary=36.82 cfs 22.164 af
Link L2-4: L2-4	Inflow=39.04 cfs 24.802 af Primary=39.04 cfs 24.802 af

Kittery_NoBuild (ID 2853676)	ME-DennettRoad 24-hr S1 25-yr Rainfall=6.58"
Prepared by Barton & Loguidice, DPC	Printed 8/7/2023
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Link L2-5: L2-5	Inflow=25.45 cfs 3.184 af
	Primary=25.45 cfs 3.184 af
Link 1.2.6, 1.2.6	Inflow-14.20 efc. 27.096 of
LINK L2-0: L2-0	Primary-44.20 cfs 27.960 al
	Fillinary=44.20 Cr3 27.900 ar
Link L4-3: L4-3	Inflow=8.29 cfs 1.763 af
	Primary=8.29 cfs 1.763 af
Link L4-4: L4-4	Inflow=13.51 cfs 2.564 af
	Primary=13.51 cfs 2.564 af
Link L4-5: L4-5	Inflow=21.// cfs 4.32/ af
l ink   5-3:   5-3	Inflow=64.06 cfs. 9.380 af
	Primary=64.06 cfs 9.380 af
	,
Link L6-2: L6-2	Inflow=0.31 cfs 0.051 af
	Primary=0.31 cfs 0.051 af
Total Runoff Area = 323.908 ac Runoff	Volume = 91.191 af Average Runoff Depth = 3.38"
96.72% Per	$v_{10}$ in $s = 313.275 \text{ ac} = 3.28\% \text{ impervious} = 10.633 \text{ ac}$

Kittery\_NoBuild (ID 2853676)ME-DennettRoad 24-hr S1 50-yrRainfall=7.50"Prepared by Barton & Loguidice, DPCPrinted 8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 28

Subcatchment 1-1: DA-1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth=4.88" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=95.05 cfs 16.633 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth=5.19" Flow Length=485' Tc=27.5 min CN=WQ Runoff=61.64 cfs 9.727 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth>5.88" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=78.04 cfs 9.903 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth=5.78" Flow Length=875' Tc=18.9 min CN=WQ Runoff=67.05 cfs 8.904 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth=4.42" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=92.39 cfs 24.994 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth=1.69" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=25.70 cfs 7.393 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth=5.16" Flow Length=850' Tc=25.6 min CN=WQ Runoff=20.95 cfs 3.202 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth=5.28" Flow Length=745' Tc=17.7 min CN=WQ Runoff=30.01 cfs 3.789 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth=3.60" Flow Length=688' Tc=33.2 min CN=WQ Runoff=14.07 cfs 2.537 af
Subcatchment 3-1-P: DA-3-	<b>1-P</b> Runoff Area=1.790 ac100.00% ImperviousRunoff Depth>7.24"Tc=0.0 minCN=98Runoff=14.83 cfs1.079 af
Subcatchment 3-2: DA-3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth=2.41" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=36.12 cfs 5.162 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth=1.73" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=10.03 cfs 2.256 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth=3.72" Flow Length=955' Tc=37.6 min CN=WQ Runoff=16.04 cfs 3.105 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth=4.49" Flow Length=625' Tc=33.8 min CN=WQ Runoff=21.43 cfs 3.880 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth>6.50" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=70.66 cfs 7.129 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth=6.70" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.36 cfs 0.059 af

Kittery_NoBuild (ID 28	ME-DennettRoad 24-hr S1 50-yr Rainfall=7.50"
Prepared by Barton & Lo	pguidice, DPC Printed 8/7/2023
Tyulucade 10.20-29 5/1100	2255 © 2022 HydroCAD Sortware Solutions LLC Page 29
Reach R1-1: R1-1	Avg. Flow Depth=2.04' Max Vel=5.08 fps Inflow=95.05 cfs 16.633 af n=0.035 L=800.0' S=0.0117 '/' Capacity=90.60 cfs Outflow=94.33 cfs 16.633 af
Reach R1-2: R1-2	Avg. Flow Depth=0.28' Max Vel=3.38 fps Inflow=3.61 cfs 6.193 af n=0.035 L=20.0' S=0.0465 '/' Capacity=180.91 cfs Outflow=3.61 cfs 6.192 af
Reach R1-3: R-1-3	Avg. Flow Depth=3.41' Max Vel=2.40 fps Inflow=142.10 cfs 32.729 af n=0.040 L=225.0' S=0.0022 '/' Capacity=56.74 cfs Outflow=141.71 cfs 32.716 af
Reach R1-4: R1-4	Avg. Flow Depth=1.48' Max Vel=6.10 fps Inflow=67.05 cfs 8.904 af n=0.035 L=290.0' S=0.0241 '/' Capacity=130.35 cfs Outflow=66.86 cfs 8.904 af
Reach R1-5: R1-5	Inflow=141.71 cfs 32.716 af Outflow=141.71 cfs 32.716 af
Reach R2-1: R2-1	Avg. Flow Depth=1.85' Max Vel=4.04 fps Inflow=64.00 cfs 27.943 af n=0.035 L=460.0' S=0.0082 '/' Capacity=76.05 cfs Outflow=63.94 cfs 27.918 af
Reach R2-2: R2-2	Avg. Flow Depth=1.25' Max Vel=5.31 fps Inflow=30.01 cfs 3.789 af n=0.030 L=175.0' S=0.0171 '/' Capacity=82.75 cfs Outflow=29.96 cfs 3.789 af
Reach R2-3: R2-3	Avg. Flow Depth=2.01' Max Vel=5.54 fps Inflow=67.13 cfs 31.120 af n=0.030 L=410.0' S=0.0110 '/' Capacity=66.21 cfs Outflow=67.10 cfs 31.104 af
Reach R3-1: R3-1	Avg. Flow Depth=0.81' Max Vel=4.58 fps Inflow=14.07 cfs 2.537 af n=0.030 L=1,596.0' S=0.0203 '/' Capacity=89.94 cfs Outflow=13.55 cfs 2.537 af
Reach R3-P: R3-P	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.040 L=276.0' S=0.0056 '/' Capacity=3.77 cfs Outflow=0.00 cfs 0.000 af
Reach R4-1: R4-1	Avg. Flow Depth=0.52' Max Vel=6.36 fps Inflow=10.03 cfs 2.256 af n=0.030 L=95.0' S=0.0632 '/' Capacity=158.84 cfs Outflow=10.03 cfs 2.256 af
Reach R4-2: R4-2	Avg. Flow Depth=0.64' Max Vel=7.58 fps Inflow=16.04 cfs 3.105 af n=0.030 L=140.0' S=0.0714 '/' Capacity=168.92 cfs Outflow=16.04 cfs 3.105 af
Reach R5-1: R5-1	Avg. Flow Depth=0.86' Max Vel=4.46 fps Inflow=21.43 cfs 3.880 af n=0.035 L=640.0' S=0.0234 '/' Capacity=325.19 cfs Outflow=21.31 cfs 3.880 af
Reach R5-2: R5-2	Avg. Flow Depth=1.96' Max Vel=4.26 fps Inflow=78.63 cfs 11.009 af n=0.035 L=700.0' S=0.0086 '/' Capacity=77.67 cfs Outflow=74.00 cfs 11.009 af
Reach R6-1: R6-1	Avg. Flow Depth=0.11' Max Vel=1.48 fps Inflow=0.36 cfs 0.059 af n=0.030 L=360.0' S=0.0203 '/' Capacity=90.06 cfs Outflow=0.35 cfs 0.059 af
Pond 1-P:	Peak Elev=39.87' Storage=6.370 af Inflow=61.64 cfs 9.727 af 12.0" Round Culvert n=0.012 L=23.0' S=0.0000 '/' Outflow=3.61 cfs 6.193 af
Pond 2-P:	Peak Elev=38.26' Storage=12.351 af Inflow=117.82 cfs 32.388 af Outflow=64.00 cfs 27.943 af

Kittery_NoBuild (ID 28536 Prepared by Barton & Loguid	76) ME-DennettRoad 24-hr S1 50-yr Rainfall=7.50" ice, DPC Printed 8/7/2023
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Pond 3-P:	Peak Elev=86.74' Storage=1.079 af Inflow=14.83 cfs 1.079 af Outflow=0.00 cfs 0.000 af
Pond CB-1-1: L1-1	Peak Elev=861.96' Inflow=95.05 cfs 16.633 af 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=95.05 cfs 16.633 af
Pond CB-1-3: L1-3	Peak Elev=51.41' Inflow=142.10 cfs 32.729 af 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=142.10 cfs 32.729 af
Pond CB-1-4: L1-4	Peak Elev=75.83' Inflow=67.05 cfs 8.904 af 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=67.05 cfs 8.904 af
Pond CB-2-3: L2-2	Peak Elev=35.39' Inflow=67.13 cfs 31.120 af 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=67.13 cfs 31.120 af
Pond CB-2-4: L2-3	Peak Elev=35.25' Inflow=30.01 cfs 3.789 af 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=30.01 cfs 3.789 af
Pond CB-3-1: L3-1	Peak Elev=90.06' Inflow=14.07 cfs 2.537 af Outflow=14.07 cfs 2.537 af
Pond CB-3-2: L3-2	Peak Elev=63.63' Inflow=44.45 cfs 7.698 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=44.45 cfs 7.698 af
Pond CB-4-1: L4-1	Peak Elev=55.45' Inflow=10.03 cfs 2.256 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=10.03 cfs 2.256 af
Pond CB-4-2: L4-2	Peak Elev=56.12' Inflow=16.04 cfs 3.105 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=16.04 cfs 3.105 af
Pond CB-5-1: L5-1	Peak Elev=57.86' Inflow=21.43 cfs 3.880 af 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=21.43 cfs 3.880 af
Pond CB-5-2: L5-2	Peak Elev=49.88' Inflow=78.63 cfs 11.009 af 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=78.63 cfs 11.009 af
Pond CB-6-1: L6-1	Peak Elev=49.51' Inflow=0.36 cfs 0.059 af 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.36 cfs 0.059 af
Link L1-5: L1-5	Inflow=141.71 cfs 32.716 af Primary=141.71 cfs 32.716 af
Link L1-6: L1-6	Inflow=204.00 cfs 41.620 af Primary=204.00 cfs 41.620 af
Link L2-1: L2-1	Inflow=64.00 cfs 27.943 af Primary=64.00 cfs 27.943 af
Link L2-4: L2-4	Inflow=67.10 cfs 31.104 af Primary=67.10 cfs 31.104 af

Kittery_NoBuild (ID 2853676)	ME-DennettRoad 24-hr S1 50-yr Rainfall=7.50'	
Prepared by Barton & Loguidice, DPC		Printed 8/7/2023
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Link L2-5: L2-5		Inflow=29.96 cfs 3.789 af
		Primary=29.96 cfs 3.789 af
Link L2-6: L2-6		Inflow=70.58 cfs 34.893 af
		Primary=70.58 cfs 34.893 af
Link L4-3: L4-3		Inflow=10.03 cfs 2.256 af
		Primary=10.03 cfs 2.256 af
Link L4-4: L4-4		Inflow=16.04 cfs 3.105 af
		Primary=16.04 cfs 3.105 af
Link L4-5: L4-5		Inflow=26.03 cfs 5.361 af
		Primary=26.03 cfs 5.361 af
Link L5-3: L5-3		Inflow=74.00 cfs 11.009 af
		Primary=74.00 cfs 11.009 af
Link L6-2: L6-2		Inflow=0.35 cfs 0.059 af
		Primary=0.35 cfs 0.059 af
Total Runoff Area - 323 908 ac	Runoff Volume - 109 753 af	Average Runoff Depth – $4.07$ "

Total Runoff Area = 323.908 acRunoff Volume = 109.753 afAverage Runoff Depth = 4.07"96.72% Pervious = 313.275 ac3.28% Impervious = 10.633 ac
Kittery\_NoBuild (ID 2853676)ME-DennettRoad 24-hr S1 100-yrRainfall=8.52"Prepared by Barton & Loguidice, DPCPrinted 8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 32

Time span=1.00-36.00 hrs, dt=0.02 hrs, 1751 points Runoff by SCS TR-20 method, UH=SCS, Weighted-Q Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1-1: DA-1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth=5.80" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=111.40 cfs 19.791 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth=6.14" Flow Length=485' Tc=27.5 min CN=WQ Runoff=71.87 cfs 11.516 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth>6.86" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=89.60 cfs 11.552 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth>6.76" Flow Length=875' Tc=18.9 min CN=WQ Runoff=77.14 cfs 10.406 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth=5.28" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=108.81 cfs 29.866 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth=2.14" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=31.10 cfs 9.371 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth>6.10" Flow Length=850' Tc=25.6 min CN=WQ Runoff=24.46 cfs 3.791 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth>6.23" Flow Length=745' Tc=17.7 min CN=WQ Runoff=34.88 cfs 4.470 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth=4.37" Flow Length=688' Tc=33.2 min CN=WQ Runoff=16.91 cfs 3.080 af
Subcatchment 3-1-P: DA-3-	I-P Runoff Area=1.790 ac 100.00% Impervious Runoff Depth>8.25"   Tc=0.0 min CN=98 Runoff=16.65 cfs 1.230 af
Subcatchment 3-2: DA-3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth=3.01" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=44.42 cfs 6.451 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth=2.19" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=12.13 cfs 2.865 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth=4.47" Flow Length=955' Tc=37.6 min CN=WQ Runoff=18.84 cfs 3.729 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth=5.34" Flow Length=625' Tc=33.8 min CN=WQ Runoff=25.02 cfs 4.612 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth>7.50" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=80.16 cfs 8.226 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth>7.71" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.41 cfs 0.067 af

Kittery_NoBuild (ID 285	<b>3676)</b> <i>ME-DennettRoad</i> 24-hr S1 100-yr Rainfall=8.52"
Prepared by Barton & Log	uidice, DPC Printed 8/7/2023
TydioCAD® 10.20-29 3/11032	55 © 2022 Hydrochd Soltware Soldiions LLC Fage 55
Reach R1-1: R1-1	Avg. Flow Depth=2.20' Max Vel=5.27 fps Inflow=111.40 cfs 19.791 af n=0.035 L=800.0' S=0.0117 '/' Capacity=90.60 cfs Outflow=110.57 cfs 19.791 af
Reach R1-2: R1-2	Avg. Flow Depth=0.55' Max Vel=4.91 fps Inflow=12.76 cfs 7.669 af n=0.035 L=20.0' S=0.0465 '/' Capacity=180.91 cfs Outflow=12.48 cfs 7.668 af
Reach R1-3: R-1-3	Avg. Flow Depth=3.81' Max Vel=2.44 fps Inflow=165.97 cfs 39.012 af n=0.040 L=225.0' S=0.0022 '/' Capacity=56.74 cfs Outflow=165.48 cfs 38.998 af
Reach R1-4: R1-4	Avg. Flow Depth=1.57' Max Vel=6.32 fps Inflow=77.14 cfs 10.406 af n=0.035 L=290.0' S=0.0241 '/' Capacity=130.35 cfs Outflow=76.92 cfs 10.406 af
Reach R1-5: R1-5	Inflow=165.48 cfs 38.998 af Outflow=165.48 cfs 38.998 af
Reach R2-1: R2-1	Avg. Flow Depth=2.26' Max Vel=4.48 fps Inflow=97.78 cfs 34.722 af n=0.035 L=460.0' S=0.0082 '/' Capacity=76.05 cfs Outflow=97.67 cfs 34.697 af
Reach R2-2: R2-2	Avg. Flow Depth=1.34' Max Vel=5.52 fps Inflow=34.88 cfs 4.470 af n=0.030 L=175.0' S=0.0171 '/' Capacity=82.75 cfs Outflow=34.82 cfs 4.470 af
Reach R2-3: R2-3	Avg. Flow Depth=2.49' Max Vel=6.05 fps Inflow=101.98 cfs 38.488 af n=0.030 L=410.0' S=0.0110 '/' Capacity=66.21 cfs Outflow=101.93 cfs 38.471 af
Reach R3-1: R3-1	Avg. Flow Depth=0.89' Max Vel=4.82 fps Inflow=16.91 cfs 3.080 af n=0.030 L=1,596.0' S=0.0203 '/' Capacity=89.94 cfs Outflow=16.34 cfs 3.080 af
Reach R3-P: R3-P	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.040 L=276.0' S=0.0056 '/' Capacity=3.77 cfs Outflow=0.00 cfs 0.000 af
Reach R4-1: R4-1	Avg. Flow Depth=0.57' Max Vel=6.71 fps Inflow=12.13 cfs 2.865 af n=0.030 L=95.0' S=0.0632 '/' Capacity=158.84 cfs Outflow=12.13 cfs 2.865 af
Reach R4-2: R4-2	Avg. Flow Depth=0.70' Max Vel=7.93 fps Inflow=18.84 cfs 3.729 af n=0.030 L=140.0' S=0.0714 '/' Capacity=168.92 cfs Outflow=18.84 cfs 3.729 af
Reach R5-1: R5-1	Avg. Flow Depth=0.93' Max Vel=4.65 fps Inflow=25.02 cfs 4.612 af n=0.035 L=640.0' S=0.0234 '/' Capacity=325.19 cfs Outflow=24.89 cfs 4.612 af
Reach R5-2: R5-2	Avg. Flow Depth=2.08' Max Vel=4.40 fps Inflow=89.65 cfs 12.838 af n=0.035 L=700.0' S=0.0086 '/' Capacity=77.67 cfs Outflow=84.64 cfs 12.838 af
Reach R6-1: R6-1	Avg. Flow Depth=0.12' Max Vel=1.55 fps Inflow=0.41 cfs 0.067 af n=0.030 L=360.0' S=0.0203 '/' Capacity=90.06 cfs Outflow=0.40 cfs 0.067 af
Pond 1-P:	Peak Elev=56.67' Storage=6.806 af Inflow=71.87 cfs 11.516 af 12.0" Round Culvert n=0.012 L=23.0' S=0.0000 '/' Outflow=12.76 cfs 7.669 af
Pond 2-P:	Peak Elev=38.36' Storage=13.065 af Inflow=139.67 cfs 39.238 af Outflow=97.78 cfs 34.722 af

Kittery_NoBuild (	D 2853676) ME-DennettRoad 24-hr S1 100-yr Rainfall=8.52"
Prepared by Barton	& Loguidice, DPC Printed 8/7/2023
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Pond 3-P:	Peak Elev=86.85' Storage=1.230 af Inflow=16.65 cfs 1.230 af Outflow=0.00 cfs 0.000 af
Pond CB-1-1: L1-1	Peak Elev=1,165.95' Inflow=111.40 cfs 19.791 af 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=111.40 cfs 19.791 af
Pond CB-1-3: L1-3	Peak Elev=55.89' Inflow=165.97 cfs 39.012 af 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=165.97 cfs 39.012 af
Pond CB-1-4: L1-4	Peak Elev=89.03' Inflow=77.14 cfs 10.406 af 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=77.14 cfs 10.406 af
Pond CB-2-3: L2-2	Peak Elev=40.48' Inflow=101.98 cfs 38.488 af 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=101.98 cfs 38.488 af
Pond CB-2-4: L2-3	Peak Elev=36.65' Inflow=34.88 cfs 4.470 af 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=34.88 cfs 4.470 af
Pond CB-3-1: L3-1	Peak Elev=91.28' Inflow=16.91 cfs 3.080 af Outflow=16.91 cfs 3.080 af
Pond CB-3-2: L3-2	Peak Elev=68.03' Inflow=54.60 cfs 9.531 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=54.60 cfs 9.531 af
Pond CB-4-1: L4-1	Peak Elev=55.65' Inflow=12.13 cfs 2.865 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=12.13 cfs 2.865 af
Pond CB-4-2: L4-2	Peak Elev=56.55' Inflow=18.84 cfs 3.729 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=18.84 cfs 3.729 af
Pond CB-5-1: L5-1	Peak Elev=58.05' Inflow=25.02 cfs 4.612 af 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=25.02 cfs 4.612 af
Pond CB-5-2: L5-2	Peak Elev=52.44' Inflow=89.65 cfs 12.838 af 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=89.65 cfs 12.838 af
Pond CB-6-1: L6-1	Peak Elev=49.53' Inflow=0.41 cfs 0.067 af 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.41 cfs 0.067 af
Link L1-5: L1-5	Inflow=165.48 cfs 38.998 af Primary=165.48 cfs 38.998 af
Link L1-6: L1-6	Inflow=237.01 cfs 49.404 af Primary=237.01 cfs 49.404 af
Link L2-1: L2-1	Inflow=97.78 cfs 34.722 af Primary=97.78 cfs 34.722 af
Link L2-4: L2-4	Inflow=101.93 cfs 38.471 af Primary=101.93 cfs 38.471 af

Kittery_NoBuild (ID 2853676)	ME-DennettRoad 24-hr S1 100-yr Rainfall=8.52"	
Prepared by Barton & Loguidice, DPC	Printed 8/7/2023	
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Link L2-5: L2-5	Inflow=34.82 cfs 4.470 af	
	Primary=34.82 cfs 4.470 af	
Link L2-6: L2-6	Inflow=106.49 cfs 42.941 af	
	Primary=106.49 cfs 42.941 af	
Link L4-3: L4-3	Inflow=12.13 cfs 2.865 af	
	Primary=12.13 cfs 2.865 af	
Link L4-4: L4-4	Inflow=18.84 cfs 3.729 af	
	Primary=18.84 cfs 3.729 af	
Link L4-5: L4-5	Inflow=30.89 cfs 6.594 af	
	Primary=30.89 cfs 6.594 af	
Link L5-3: L5-3	Inflow=84.64 cfs 12.838 af	
	Primary=84.64 cfs 12.838 af	
Link L6-2: L6-2	Inflow=0.40 cfs 0.067 af	
	Primary=0.40 cfs 0.067 af	
Total Runoff Area = 323,908 ac Runo	off Volume = 131.025 af Average Runoff Depth = 4.85"	

Total Runoff Area = 323.908 acRunoff Volume = 131.025 afAverage Runoff Depth = 4.85"96.72% Pervious = 313.275 ac3.28% Impervious = 10.633 ac

Kittery\_NoBuild (ID 2853676)ME-DennettRoad 24-hr S1 1-yrExtreme: 1-yr Rainfall=2.63"Prepared by Barton & Loguidice, DPCPrinted 8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 36

Time span=1.00-36.00 hrs, dt=0.02 hrs, 1751 points Runoff by SCS TR-20 method, UH=SCS, Weighted-Q Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1-1: DA-1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth=0.90" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=18.87 cfs 3.059 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth=1.02" Flow Length=485' Tc=27.5 min CN=WQ Runoff=13.31 cfs 1.914 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth=1.45" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=22.23 cfs 2.442 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth=1.39" Flow Length=875' Tc=18.9 min CN=WQ Runoff=18.50 cfs 2.139 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth=0.80" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=17.96 cfs 4.502 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth=0.26" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=4.74 cfs 1.127 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth=1.03" Flow Length=850' Tc=25.6 min CN=WQ Runoff=4.55 cfs 0.639 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth=1.12" Flow Length=745' Tc=17.7 min CN=WQ Runoff=7.03 cfs 0.803 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth=0.57" Flow Length=688' Tc=33.2 min CN=WQ Runoff=2.40 cfs 0.399 af
Subcatchment 3-1-P: DA-3-7	I-P Runoff Area=1.790 ac 100.00% Impervious Runoff Depth=2.40" Tc=0.0 min CN=98 Runoff=5.87 cfs 0.358 af
Subcatchment 3-2: DA-3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth=0.31" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=5.12 cfs 0.673 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth=0.24" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=1.68 cfs 0.319 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth=0.69" Flow Length=955' Tc=37.6 min CN=WQ Runoff=3.29 cfs 0.578 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth=0.90" Flow Length=625' Tc=33.8 min CN=WQ Runoff=4.74 cfs 0.778 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth=1.84" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=23.98 cfs 2.023 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth=1.97" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.13 cfs 0.017 af

Kittery_NoBuild (ID 2 Prepared by Barton & L	853676) ME-DennettRoad 24-hr S1 1-yr Extreme: 1-yr Rainfall=2.63" oguidice, DPC Printed 8/7/2023
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Reach R1-1: R1-1	Avg. Flow Depth=0.95' Max Vel=3.32 fps Inflow=18.87 cfs 3.059 af n=0.035 L=800.0' S=0.0117 '/' Capacity=90.60 cfs Outflow=18.51 cfs 3.059 af
Reach R1-2: R1-2	Avg. Flow Depth=0.09' Max Vel=1.75 fps Inflow=0.52 cfs 0.810 af n=0.035 L=20.0' S=0.0465 '/' Capacity=180.91 cfs Outflow=0.52 cfs 0.810 af
Reach R1-3: R-1-3	Avg. Flow Depth=1.51' Max Vel=1.73 fps Inflow=31.63 cfs 6.311 af n=0.040 L=225.0' S=0.0022 '/' Capacity=56.74 cfs Outflow=31.45 cfs 6.307 af
Reach R1-4: R1-4	Avg. Flow Depth=0.79' Max Vel=4.33 fps Inflow=18.50 cfs 2.139 af n=0.035 L=290.0' S=0.0241 '/' Capacity=130.35 cfs Outflow=18.41 cfs 2.139 af
Reach R1-5: R1-5	Inflow=31.45 cfs 6.307 af Outflow=31.45 cfs 6.307 af
Reach R2-1: R2-1	Avg. Flow Depth=0.42' Max Vel=1.78 fps Inflow=3.18 cfs 4.846 af n=0.035 L=460.0' S=0.0082 '/' Capacity=76.05 cfs Outflow=3.18 cfs 4.836 af
Reach R2-2: R2-2	Avg. Flow Depth=0.61' Max Vel=3.60 fps Inflow=7.03 cfs 0.803 af n=0.030 L=175.0' S=0.0171 '/' Capacity=82.75 cfs Outflow=7.02 cfs 0.803 af
Reach R2-3: R2-3	Avg. Flow Depth=0.59' Max Vel=2.83 fps Inflow=5.34 cfs 5.475 af n=0.030 L=410.0' S=0.0110 '/' Capacity=66.21 cfs Outflow=5.29 cfs 5.469 af
Reach R3-1: R3-1	Avg. Flow Depth=0.31' Max Vel=2.71 fps Inflow=2.40 cfs 0.399 af n=0.030 L=1,596.0' S=0.0203 '/' Capacity=89.94 cfs Outflow=2.18 cfs 0.399 af
Reach R3-P: R3-P	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.040 L=276.0' S=0.0056 '/' Capacity=3.77 cfs Outflow=0.00 cfs 0.000 af
Reach R4-1: R4-1	Avg. Flow Depth=0.19' Max Vel=3.67 fps Inflow=1.68 cfs 0.319 af n=0.030 L=95.0' S=0.0632 '/' Capacity=158.84 cfs Outflow=1.68 cfs 0.319 af
Reach R4-2: R4-2	Avg. Flow Depth=0.27' Max Vel=4.75 fps Inflow=3.29 cfs 0.578 af n=0.030 L=140.0' S=0.0714 '/' Capacity=168.92 cfs Outflow=3.29 cfs 0.578 af
Reach R5-1: R5-1	Avg. Flow Depth=0.39' Max Vel=2.89 fps Inflow=4.74 cfs 0.778 af n=0.035 L=640.0' S=0.0234 '/' Capacity=325.19 cfs Outflow=4.67 cfs 0.778 af
Reach R5-2: R5-2	Avg. Flow Depth=1.13' Max Vel=3.14 fps Inflow=25.25 cfs 2.801 af n=0.035 L=700.0' S=0.0086 '/' Capacity=77.67 cfs Outflow=22.75 cfs 2.801 af
Reach R6-1: R6-1	Avg. Flow Depth=0.06' Max Vel=1.01 fps Inflow=0.13 cfs 0.017 af n=0.030 L=360.0' S=0.0203 '/' Capacity=90.06 cfs Outflow=0.12 cfs 0.017 af
Pond 1-P:	Peak Elev=38.44' Storage=1.490 af Inflow=13.31 cfs 1.914 af 12.0" Round Culvert n=0.012 L=23.0' S=0.0000 '/' Outflow=0.52 cfs 0.810 af
Pond 2-P:	Peak Elev=36.55' Storage=2.849 af Inflow=22.61 cfs 5.629 af Outflow=3.18 cfs 4.846 af

Kittery_NoBuild (ID 285367 Prepared by Barton & Loguidi	<b>76)</b> <i>ME-DennettRoad</i> 24-hr S1 1-yr <i>Extreme:</i> 1-yr <i>Ra</i> ce, DPC Printe	infall=2.63" ed 8/7/2023
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Pond 3-P:	Peak Elev=86.25' Storage=0.358 af Inflow=5.87 Outflow=0.00	cfs 0.358 af cfs 0.000 af
Pond CB-1-1: L1-1	Peak Elev=79.53' Inflow=18.87 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=18.87	cfs 3.059 af cfs 3.059 af
Pond CB-1-3: L1-3	Peak Elev=39.83' Inflow=31.63 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=31.63	cfs 6.311 af cfs 6.311 af
Pond CB-1-4: L1-4	Peak Elev=39.40' Inflow=18.50 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=18.50	cfs 2.139 af cfs 2.139 af
Pond CB-2-3: L2-2	Peak Elev=30.86' Inflow=5.34 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=5.34	cfs 5.475 af cfs 5.475 af
Pond CB-2-4: L2-3	Peak Elev=31.64' Inflow=7.03 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=7.03	cfs 0.803 af cfs 0.803 af
Pond CB-3-1: L3-1	Peak Elev=87.28' Inflow=2.40 Outflow=2.40	cfs 0.399 af cfs 0.399 af
Pond CB-3-2: L3-2	Peak Elev=55.08' Inflow=6.14 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=6.14	cfs 1.073 af cfs 1.073 af
Pond CB-4-1: L4-1	Peak Elev=54.53' Inflow=1.68 24.0" Round Culvert n=0.012 L=100.0' S=0.0200'/' Outflow=1.68	cfs 0.319 af cfs 0.319 af
Pond CB-4-2: L4-2	Peak Elev=54.76' Inflow=3.29 24.0" Round Culvert n=0.012 L=100.0' S=0.0200'/' Outflow=3.29	cfs 0.578 af cfs 0.578 af
Pond CB-5-1: L5-1	Peak Elev=56.81' Inflow=4.74 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=4.74	cfs 0.778 af cfs 0.778 af
Pond CB-5-2: L5-2	Peak Elev=42.22' Inflow=25.25 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=25.25	cfs 2.801 af cfs 2.801 af
Pond CB-6-1: L6-1	Peak Elev=49.40' Inflow=0.13 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.13	cfs 0.017 af cfs 0.017 af
Link L1-5: L1-5	Inflow=31.45 Primary=31.45	cfs 6.307 af cfs 6.307 af
Link L1-6: L1-6	Inflow=49.32 Primary=49.32	cfs 8.446 af cfs 8.446 af
Link L2-1: L2-1	Inflow=3.18 Primary=3.18	cfs 4.846 af cfs 4.846 af
Link L2-4: L2-4	Inflow=5.29 Primary=5.29	cfs 5.469 af cfs 5.469 af

Kittery_NoBuild (ID 2853676) Prepared by Barton & Loguidice, DPC	ME-DennettRoad 24-hr S1 1-yr	Extreme: 1-yr Rainfall=2.63" Printed 8/7/2023
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Link L2-5: L2-5		Inflow=7.02 cfs 0.803 af Primary=7.02 cfs 0.803 af
Link L2-6: L2-6		Inflow=11.55 cfs 6.272 af Primary=11.55 cfs 6.272 af
Link L4-3: L4-3		Inflow=1.68 cfs 0.319 af Primary=1.68 cfs 0.319 af
Link L4-4: L4-4		Inflow=3.29 cfs 0.578 af Primary=3.29 cfs 0.578 af
Link L4-5: L4-5		Inflow=4.94 cfs 0.897 af Primary=4.94 cfs 0.897 af
Link L5-3: L5-3		Inflow=22.75 cfs 2.801 af Primary=22.75 cfs 2.801 af
Link L6-2: L6-2		Inflow=0.12 cfs 0.017 af Primary=0.12 cfs 0.017 af
Total Runoff Area - 323 0	18 ac Runoff Volume - 21 771 af	Average Runoff Depth - 0.81"

323.908 ac Runoff Volume = 21.771 af Average Runoff Depth = 0.81" 96.72% Pervious = 313.275 ac 3.28% Impervious = 10.633 ac TOLAI RUNOIT Area

Kittery\_NoBuild (ID 2853676)ME-DennettRoad 24-hr S1 10-yrExtreme: 10-yr Rainfall=5.32"Prepared by Barton & Loguidice, DPCPrinted 8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 40

Time span=1.00-36.00 hrs, dt=0.02 hrs, 1751 points Runoff by SCS TR-20 method, UH=SCS, Weighted-Q Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1-1: DA-1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth=2.97" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=59.93 cfs 10.129 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth=3.21" Flow Length=485' Tc=27.5 min CN=WQ Runoff=39.53 cfs 6.018 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth=3.82" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=52.87 cfs 6.442 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth=3.74" Flow Length=875' Tc=18.9 min CN=WQ Runoff=45.11 cfs 5.756 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth=2.67" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=57.79 cfs 15.075 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth=0.90" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=15.92 cfs 3.944 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth=3.19" Flow Length=850' Tc=25.6 min CN=WQ Runoff=13.40 cfs 1.982 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth=3.31" Flow Length=745' Tc=17.7 min CN=WQ Runoff=19.48 cfs 2.376 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth=2.07" Flow Length=688' Tc=33.2 min CN=WQ Runoff=8.38 cfs 1.462 af
Subcatchment 3-1-P: DA-3-	I-P Runoff Area=1.790 ac 100.00% Impervious Runoff Depth>5.07"   Tc=0.0 min CN=98 Runoff=10.82 cfs 0.757 af
Subcatchment 3-2: DA-3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth=1.29" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=20.07 cfs 2.759 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth=0.91" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=6.02 cfs 1.187 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth=2.23" Flow Length=955' Tc=37.6 min CN=WQ Runoff=10.15 cfs 1.864 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth=2.76" Flow Length=625' Tc=33.8 min CN=WQ Runoff=13.75 cfs 2.388 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth=4.38" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=49.83 cfs 4.804 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth=4.56" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.26 cfs 0.040 af

Kittery_NoBuild (ID 28536	76) ME-DennettRoad 24-hr S1 10-yr Extreme: 10-yr Rainfall=5.32"
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Reach R1-1: R1-1	Avg. Flow Depth=1.65' Max Vel=4.52 fps Inflow=59.93 cfs 10.129 af n=0.035 L=800.0' S=0.0117 '/' Capacity=90.60 cfs Outflow=59.37 cfs 10.129 af
Reach R1-2: R1-2	Avg. Flow Depth=0.21' Max Vel=2.90 fps Inflow=2.23 cfs 3.658 af n=0.035 L=20.0' S=0.0465 '/' Capacity=180.91 cfs Outflow=2.23 cfs 3.658 af
Reach R1-3: R-1-3	Avg. Flow Depth=2.57' Max Vel=2.25 fps Inflow=90.97 cfs 20.230 af n=0.040 L=225.0' S=0.0022 '/' Capacity=56.74 cfs Outflow=90.72 cfs 20.221 af
Reach R1-4: R1-4	Avg. Flow Depth=1.23' Max Vel=5.50 fps Inflow=45.11 cfs 5.756 af n=0.035 L=290.0' S=0.0241 '/' Capacity=130.35 cfs Outflow=44.96 cfs 5.756 af
Reach R1-5: R1-5	Inflow=90.72 cfs 20.221 af Outflow=90.72 cfs 20.221 af
Reach R2-1: R2-1	Avg. Flow Depth=0.77' Max Vel=2.48 fps Inflow=10.06 cfs 15.098 af n=0.035 L=460.0' S=0.0082 '/' Capacity=76.05 cfs Outflow=10.06 cfs 15.075 af
Reach R2-2: R2-2	Avg. Flow Depth=1.02' Max Vel=4.75 fps Inflow=19.48 cfs 2.376 af n=0.030 L=175.0' S=0.0171 '/' Capacity=82.75 cfs Outflow=19.45 cfs 2.376 af
Reach R2-3: R2-3	Avg. Flow Depth=1.02' Max Vel=3.80 fps Inflow=15.72 cfs 17.057 af n=0.030 L=410.0' S=0.0110 '/' Capacity=66.21 cfs Outflow=15.65 cfs 17.042 af
Reach R3-1: R3-1	Avg. Flow Depth=0.62' Max Vel=3.96 fps Inflow=8.38 cfs 1.462 af n=0.030 L=1,596.0' S=0.0203 '/' Capacity=89.94 cfs Outflow=7.98 cfs 1.462 af
Reach R3-P: R3-P	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.040 L=276.0' S=0.0056 '/' Capacity=3.77 cfs Outflow=0.00 cfs 0.000 af
Reach R4-1: R4-1	Avg. Flow Depth=0.39' Max Vel=5.48 fps Inflow=6.02 cfs 1.187 af n=0.030 L=95.0' S=0.0632 '/' Capacity=158.84 cfs Outflow=6.02 cfs 1.187 af
Reach R4-2: R4-2	Avg. Flow Depth=0.51' Max Vel=6.67 fps Inflow=10.15 cfs 1.864 af n=0.030 L=140.0' S=0.0714 '/' Capacity=168.92 cfs Outflow=10.15 cfs 1.864 af
Reach R5-1: R5-1	Avg. Flow Depth=0.69' Max Vel=3.94 fps Inflow=13.75 cfs 2.388 af n=0.035 L=640.0' S=0.0234 '/' Capacity=325.19 cfs Outflow=13.66 cfs 2.388 af
Reach R5-2: R5-2	Avg. Flow Depth=1.65' Max Vel=3.87 fps Inflow=54.63 cfs 7.192 af n=0.035 L=700.0' S=0.0086 '/' Capacity=77.67 cfs Outflow=50.77 cfs 7.192 af
Reach R6-1: R6-1	Avg. Flow Depth=0.09' Max Vel=1.31 fps Inflow=0.26 cfs 0.040 af n=0.030 L=360.0' S=0.0203 '/' Capacity=90.06 cfs Outflow=0.25 cfs 0.040 af
Pond 1-P:	Peak Elev=39.17' Storage=3.987 af Inflow=39.53 cfs 6.018 af 12.0" Round Culvert n=0.012 L=23.0' S=0.0000 '/' Outflow=2.23 cfs 3.658 af
Pond 2-P:	Peak Elev=38.02' Storage=10.594 af Inflow=73.53 cfs 19.019 af Outflow=10.06 cfs 15.098 af

Kittery_NoBuild ( Prepared by Barton	<b>D 2853676)</b> ME-DennettRoad 24-hr S1 10-yr Extreme: 10-yr Rainfall=5.32" & Loguidice, DPC Printed 8/7/2023
HydroCAD® 10.20-2g	s/n 05255 © 2022 HydroCAD Software Solutions LLC Page 42
Pond 3-P:	Peak Elev=86.52' Storage=0.757 af Inflow=10.82 cfs 0.757 af Outflow=0.00 cfs 0.000 af
Pond CB-1-1: L1-1	Peak Elev=371.42' Inflow=59.93 cfs 10.129 af 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=59.93 cfs 10.129 af
Pond CB-1-3: L1-3	Peak Elev=44.40' Inflow=90.97 cfs 20.230 af 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=90.97 cfs 20.230 af
Pond CB-1-4: L1-4	Peak Elev=53.48' Inflow=45.11 cfs 5.756 af 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=45.11 cfs 5.756 af
Pond CB-2-3: L2-2	Peak Elev=31.56' Inflow=15.72 cfs 17.057 af 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=15.72 cfs 17.057 af
Pond CB-2-4: L2-3	Peak Elev=33.37' Inflow=19.48 cfs 2.376 af 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=19.48 cfs 2.376 af
Pond CB-3-1: L3-1	Peak Elev=88.29' Inflow=8.38 cfs 1.462 af Outflow=8.38 cfs 1.462 af
Pond CB-3-2: L3-2	Peak Elev=57.68' Inflow=24.79 cfs 4.221 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=24.79 cfs 4.221 af
Pond CB-4-1: L4-1	Peak Elev=55.07' Inflow=6.02 cfs 1.187 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=6.02 cfs 1.187 af
Pond CB-4-2: L4-2	Peak Elev=55.46' Inflow=10.15 cfs 1.864 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=10.15 cfs 1.864 af
Pond CB-5-1: L5-1	Peak Elev=57.44' Inflow=13.75 cfs 2.388 af 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=13.75 cfs 2.388 af
Pond CB-5-2: L5-2	Peak Elev=45.46' Inflow=54.63 cfs 7.192 af 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=54.63 cfs 7.192 af
Pond CB-6-1: L6-1	Peak Elev=49.47' Inflow=0.26 cfs 0.040 af 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.26 cfs 0.040 af
Link L1-5: L1-5	Inflow=90.72 cfs 20.221 af Primary=90.72 cfs 20.221 af
Link L1-6: L1-6	Inflow=133.07 cfs 25.977 af Primary=133.07 cfs 25.977 af
Link L2-1: L2-1	Inflow=10.06 cfs 15.098 af Primary=10.06 cfs 15.098 af
Link L2-4: L2-4	Inflow=15.65 cfs 17.042 af Primary=15.65 cfs 17.042 af

Kittery_NoBuild (ID 2853676) <i>ME-DennettRoad 24-hr S1 10-yr</i> Prepared by Barton & Loguidice, DPC	Extreme: 10-yr Rainfall=5.32" Printed 8/7/2023
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Link L2-5: L2-5	Inflow=19.45 cfs 2.376 af Primary=19 45 cfs 2.376 af
Link L2-6: L2-6	Inflow=33.10 cfs 19.418 af Primary=33.10 cfs 19.418 af
Link L4-3: L4-3	Inflow=6.02 cfs 1.187 af Primary=6.02 cfs 1.187 af
Link L4-4: L4-4	Inflow=10.15 cfs 1.864 af Primary=10.15 cfs 1.864 af
Link L4-5: L4-5	Inflow=16.15 cfs 3.052 af Primary=16.15 cfs 3.052 af
Link L5-3: L5-3	Inflow=50.77 cfs 7.192 af Primary=50.77 cfs 7.192 af
Link L6-2: L6-2	Inflow=0.25 cfs 0.040 af Primary=0.25 cfs 0.040 af
Total Duneff Area 202,000 as Duneff Valume 66,005 of	Average Duneff Denth 2 49"

Total Runoff Area = 323.908 acRunoff Volume = 66.985 afAverage Runoff Depth = 2.48"96.72% Pervious = 313.275 ac3.28% Impervious = 10.633 ac

Appendix G

Model My Watershed Existing Conditions Nutrient Load Summary Reports

Drainage Area 1 (DA-1)						
Sources	Sediment	Total Nitrogen	Total Phosphorus			
Total Loads (lb)	2,771.60	141.3	5.7			
Loading Rates (lb/ac)	29.13	1.49	0.06			
Sources	Sediment (lb)	Total Nitrogen (Ib)	Total Phosphorus (lb)			
Hay/Pasture	0	0	0			
Cropland	0	0	0			
Wooded Areas	28.2	9.1	0.5			
Wetlands	0	13.2	0.7			
Open Land	9.7	0.7	0			
Barren Areas	1	0.8	0			
Low-Density Mixed	153.40	3.9	0.4			
Medium-Density Mixed	712.80	14.9	1.5			
High-Density Mixed	250.40	5.2	0.5			
Low-Density Open Space	180.9	4.6	0.5			
Farm Animals	0	0	0			
Stream Bank Erosion	1,435.20	2.2	0			
Subsurface Flow	0	57.6	1.5			
Point Sources	0	0	0			
Septic Systems	0	29.1	0			

Drainage Area 4 (DA-4)								
Sources Sediment Total Nitrogen Total Phosphore								
Total Loads (lb)	110.4	42.7	1.4					
Loading Rates (lb/ac)	3.92	1.51	0.05					
Sources	Sediment (lb)	Total Nitrogen (Ib)	Total Phosphorus (Ib)					
Hay/Pasture	0	0	0					
Cropland	0	0	0					
Wooded Areas	0.9	1.5	0.1					
Wetlands	0	0.4	0					
Open Land	0	0	0					
Barren Areas	0	0.7	0					
Low-Density Mixed	57	1.7	0.2					
Medium-Density Mixed	21.4	0.4	0					
High-Density Mixed	13.1	0.5	0.1					
Low-Density Open Space	11.4	0.3	0					
Farm Animals	0	0	0					
Stream Bank Erosion	6.6	0	0					
Subsurface Flow	0	37.2	1					
Point Sources	0	0	0					
Septic Systems	0	0	0					

Drainage Area 2 (DA-2)					
Sources	Sediment	Total Nitrogen	Total Phosphorus		
Total Loads (lb)	2,921.60	221.3	8		
Loading Rates (lb/ac)	19.87	1.51	0.05		
Sources	Sediment (lb)	Total Nitrogen (Ib)	Total Phosphorus (Ib)		
Hay/Pasture	0	0	0		
Cropland	0	0	0		
Wooded Areas	56.6	16.2	0.9		
Wetlands	13.2	9.7	0.5		
Open Land	9.5	0.4	0		
Barren Areas	0	0.2	0		
Low-Density Mixed	162.5	4.2	0.5		
Medium-Density Mixed	511.60	10.7	1.1		
High-Density Mixed	182.70	3.8	0.4		
Low-Density Open Space	166.6	4.3	0.5		
Farm Animals	0	0	0		
Stream Bank Erosion	1,818.80	2.2	0		
Subsurface Flow	0	158.7	4.2		
Point Sources	0	0	0		
Septic Systems	0	10.9	0		

Drainage Area 5 (DA-5)						
Sources	Total Nitrogen	Total Phosphorus				
Total Loads (lb)	213.8	41.6	1.3			
Loading Rates (lb/ac)	9.11	1.77	0.06			
Sources	Sediment (lb)	Total Nitrogen (Ib)	Total Phosphorus (lb)			
Hay/Pasture	0	0	0			
Cropland	0	0	0			
Wooded Areas	8.9	2.7	0.2			
Wetlands	0	0.3	0			
Open Land	28.9	1.1	0.1			
Barren Areas	0	0.7	0			
Low-Density Mixed	15	0.5	0			
Medium-Density Mixed	80.1	2	0.2			
High-Density Mixed	68.6	1.7	0.2			
Low-Density Open Space	3.4	0.1	0			
Farm Animals	0	0	0			
Stream Bank Erosion	8.8	0	0			
Subsurface Flow	0	25.4	0.7			
Point Sources	0	0	0			
Septic Systems	0	7.3	0			

Sources	
Total Loads Loading Rate	(lb) es (lb/ac)
	Sources

Hay/Pasture Cropland Wooded Areas Wetlands Open Land Barren Areas Low-Density Mixed Medium-Density Mixed High-Density Mixed Low-Density Open Space Farm Animals Stream Bank Erosion Subsurface Flow Point Sources Septic Systems

Sources	
Total Loads (lb)	
Loading Rates (I	b/ac)
	Sources
Hay/Pasture	
Cropland	
Wooded Areas	
Wetlands	
Open Land	
Barren Areas	
Low-Density Mi	xed
Medium-Densit	y Mixed
High-Density M	ixed
Low-Density Op	en Space
Farm Animals	
Stream Bank Er	osion
Subsurface Flow	<i>ı</i>
Point Sources	
Septic Systems	

Drainage Area 3	(DA-3)	
Sediment	Total Nitrogen	Total Phosphorus
459.5 12.74	62.8 1.74	2.2 0.06
Sediment (lb)	Total Nitrogen (lb)	Total Phosphorus (lb)
0	0	0
0	0	0
0	0.6	0
0.1	1.1	0.1
0	0	0
0	0	0
110.5	2.9	0.3
214	3.6	0.4
0	0	0
110.5	2.9	0.3
0	0	0
24.3	0	0
0	44.4	1.2
0	0	0
0	7.3	0

Drai	inage Area 6 (I	DA-6)	
	Sediment	Total Nitrogen	Total Phosphorus
	19.4	1.1	0.1
	26.21	1.43	0.09
	Sediment (lb)	Total Nitrogen (Ib)	Total Phosphorus (Ib)
	0	0	0
	0	0	0
	0	0	0
	0	0	0
	0	0	0
	0	0	0
	15	0.5	0
	0	0	0
	0	0	0
	0	0	0
	0	0	0
	4.4	0	0
	0	0.6	0
	0	0	0
	0	0	0

Appendix H

50% Build-Out HydroCAD Summary Report



# **Project Notes**

Copied 10 events from ME-DennettRoad 24-hr S1 storm

Event#	Event	Storm Type		Mode	Duration	B/B	Depth	AMC
	Name				(hours)		(inches)	
1	1-yr	ME-DennettRoad 24-hr S1	1-yr	Default	24.00	1	2.63	2
2	2-yr	ME-DennettRoad 24-hr S1	2-yr	Default	24.00	1	3.31	2
3	5-yr	ME-DennettRoad 24-hr S1	5-yr	Default	24.00	1	4.41	2
4	10-yr	ME-DennettRoad 24-hr S1	10-yr	Default	24.00	1	5.32	2
5	25-yr	ME-DennettRoad 24-hr S1	25-yr	Default	24.00	1	6.58	2
6	50-yr	ME-DennettRoad 24-hr S1	50-yr	Default	24.00	1	7.50	2
7	100-yr	ME-DennettRoad 24-hr S1	100-yr	Default	24.00	1	8.52	2
8	Extreme: 1-yr	ME-DennettRoad 24-hr S1	10-yr	Default	24.00	1	3.02	2
9	Extreme: 10-yr	ME-DennettRoad 24-hr S1	10-yr	Default	24.00	1	5.32	2

## Rainfall Events Listing (selected events)

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### Area Listing (all nodes)

CN	Description
	(subcatchment-numbers)
92	*BARREN, D (89, 92, 95) (1-2)
89	*COMMERCIAL, A (89) (2-2)
95	*COMMERCIAL, D (95) (1-2, 1-3)
90	*RESIDENTIAL, D (85, 90, 95) (1-2, 1-3)
60	*WOODS, A (30, 60, 95) (2-1, 2-2, 4-1)
86	*WOODS, D (77, 86, 95) (1-2, 1-3, 2-1, 2-2, 4-1)
54	1/2 acre lots, 25% imp, HSG A (2-1, 2-2, 3-1, 3-2, 4-1)
80	1/2 acre lots, 25% imp, HSG C (1-1)
85	1/2 acre lots, 25% imp, HSG D (1-1, 1-2, 2-1, 3-1, 3-2)
77	1/8 acre lots, 65% imp, HSG A (1-1, 3-1, 3-2, 4-1)
90	1/8 acre lots, 65% imp, HSG C (1-1)
92	1/8 acre lots, 65% imp, HSG D (1-1, 1-2, 2-1, 3-1, 3-2)
39	>75% Grass cover, Good, HSG A (2-1, 2-2, 3-1, 3-2, 4-1)
74	>75% Grass cover, Good, HSG C (1-1)
80	>75% Grass cover, Good, HSG D (1-1, 1-2, 2-1, 2-2, 3-1, 3-2)
65	Brush, Good, HSG C (1-1)
89	COMMERCIAL, A (89) (2-4, 5-1)
95	COMMERCIAL, D (95) (1-4, 2-3, 2-4, 4-2, 5-1, 5-2, 6-1)
90	RESIDENTIAL, D (85, 90, 95) (1-4)
60	WOODS, A (30, 60, 95) (2-4, 4-2, 5-1)
81	WOODS, D (77, 81, 85) (1-4)
86	WOODS, D (77, 86, 95) (1-4, 2-3, 2-4, 4-2, 5-1, 5-2, 6-1)
98	Water Surface, HSG A (3-1-P)
30	Woods, Good, HSG A (1-1, 2-1, 2-2, 3-1, 3-2, 4-1)
70	Woods, Good, HSG C (1-1)
77	Woods, Good, HSG D (1-1, 1-2, 1-3, 2-1, 2-2, 3-1, 3-2, 4-1)
74	TOTAL AREA
	CN 92 89 95 90 60 86 54 80 85 77 90 92 39 74 80 65 89 95 90 60 81 86 98 30 70 77 <b>74</b>

# Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
47.746	HSG A	1-1, 2-1, 2-2, 3-1, 3-1-P, 3-2, 4-1
0.000	HSG B	
3.378	HSG C	1-1
131.466	HSG D	1-1, 1-2, 1-3, 2-1, 2-2, 3-1, 3-2, 4-1
141.318	Other	1-2, 1-3, 1-4, 2-1, 2-2, 2-3, 2-4, 4-1, 4-2, 5-1, 5-2, 6-1
323.908		TOTAL AREA

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Ground Covers (all nodes)

HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
(acres)	(acres)	(acres)	(acres)	(acres)	(acres)	Cover	Numbers
 0.000	0.000	0.000	0.000	0.423	0.423	*BARREN, D (89, 92, 95)	1-2
0.000	0.000	0.000	0.000	0.723	0.723	*COMMERCIAL, A (89)	2-2
0.000	0.000	0.000	0.000	9.929	9.929	*COMMERCIAL, D (95)	1-2, 1-3
0.000	0.000	0.000	0.000	6.495	6.495	*RESIDENTIAL, D (85, 90, 95)	1-2, 1-3
0.000	0.000	0.000	0.000	33.767	33.767	*WOODS, A (30, 60, 95)	2-1, 2-2,
							4-1
0.000	0.000	0.000	0.000	21.793	21.793	*WOODS, D (77, 86, 95)	1-2, 1-3,
							2-1, 2-2,
							4-1
8.453	0.000	0.193	12.899	0.000	21.545	1/2 acre lots, 25% imp	1-1, 1-2,
							2-1, 2-2,
							3-1, 3-2,
							4-1
1.776	0.000	0.089	3.453	0.000	5.318	1/8 acre lots, 65% imp	1-1, 1-2,
							2-1, 3-1,
							3-2, 4-1
7.040	0.000	0.916	16.248	0.000	24.204	>75% Grass cover, Good	1-1, 1-2,
							2-1, 2-2,
							3-1, 3-2,
							4-1
0.000	0.000	0.006	0.000	0.000	0.006	Brush, Good	1-1
0.000	0.000	0.000	0.000	0.291	0.291	COMMERCIAL, A (89)	2-4, 5-1
0.000	0.000	0.000	0.000	23.114	23.114	COMMERCIAL, D (95)	1-4, 2-3,
							2-4, 4-2,
							5-1, 5-2,
							6-1
0.000	0.000	0.000	0.000	3.708	3.708	RESIDENTIAL, D (85, 90, 95)	1-4
0.000	0.000	0.000	0.000	4.367	4.367	WOODS, A (30, 60, 95)	2-4, 4-2,
							5-1
0.000	0.000	0.000	0.000	3.785	3.785	WOODS, D (77, 81, 85)	1-4
0.000	0.000	0.000	0.000	32.923	32.923	WOODS, D (77, 86, 95)	1-4, 2-3,
							2-4, 4-2,
							5-1, 5-2,
							6-1
1.790	0.000	0.000	0.000	0.000	1.790	Water Surface	3-1-P
28.687	0.000	2.174	98.866	0.000	129.727	Woods, Good	1-1, 1-2,
							1-3, 2-1,
							2-2, 3-1,
			101 100				3-2, 4-1
47.746	0.000	3.378	131.466	141.318	323.908	IOTAL AREA	

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Line#	Node	In-Invert	Out-Invert	Length	Slope	n	Width	Diam/Height	Inside-Fill
	Number	(feet)	(feet)	(feet)	(ft/ft)		(inches)	(inches)	(inches)
1	1-4	0.00	0.00	225.0	0.0180	0.025	0.0	12.0	0.0
2	3-1	0.00	0.00	10.0	0.0500	0.013	0.0	6.0	0.0
3	3-2	0.00	0.00	68.0	0.0290	0.025	0.0	12.0	0.0
4	1-P	37.91	37.91	23.0	0.0000	0.012	0.0	12.0	0.0
5	2-P	35.58	32.78	50.0	0.0560	0.025	0.0	18.0	0.0
6	CB-1-1	47.05	46.31	50.0	0.0148	0.025	0.0	14.0	0.0
7	CB-1-3	36.98	36.80	95.0	0.0019	0.012	0.0	36.0	0.0
8	CB-1-4	36.00	33.00	84.0	0.0357	0.025	0.0	24.0	0.0
9	CB-2-3	30.00	28.50	150.0	0.0100	0.012	0.0	36.0	0.0
10	CB-2-4	30.00	30.00	75.0	0.0000	0.012	0.0	24.0	0.0
11	CB-3-1	86.11	86.46	20.0	-0.0175	0.012	0.0	6.0	0.0
12	CB-3-1	86.67	86.32	20.0	0.0175	0.012	0.0	12.0	0.0
13	CB-3-1	86.64	86.51	20.0	0.0065	0.012	0.0	6.0	0.0
14	CB-3-1	86.97	86.75	20.0	0.0110	0.012	0.0	14.0	0.0
15	CB-3-2	54.00	52.00	100.0	0.0200	0.012	0.0	24.0	0.0
16	CB-4-1	54.00	52.00	100.0	0.0200	0.012	0.0	24.0	0.0
17	CB-4-2	54.00	52.00	100.0	0.0200	0.012	0.0	24.0	0.0
18	CB-5-1	56.00	55.00	70.0	0.0143	0.012	0.0	36.0	0.0
19	CB-5-2	40.00	34.00	217.0	0.0276	0.012	0.0	32.0	0.0
20	CB-6-1	49.25	46.31	95.0	0.0309	0.012	0.0	18.0	0.0

### Pipe Listing (all nodes)

Kittery\_50%Build (ID 2853680)ME-DennettRoad 24-hr S1 1-yrRainfall=2.63"Prepared by Barton & Loguidice, DPCPrinted 8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 8

Time span=5.00-36.00 hrs, dt=0.02 hrs, 1551 points Runoff by SCS TR-20 method, UH=SCS, Weighted-Q Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1-1: DA-1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth>0.90" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=18.87 cfs 3.059 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth>1.09" Flow Length=485' Tc=27.5 min CN=WQ Runoff=14.39 cfs 2.052 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth>1.61" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=25.23 cfs 2.720 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth>1.62" Flow Length=875' Tc=18.9 min CN=WQ Runoff=22.05 cfs 2.488 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth=0.83" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=18.85 cfs 4.694 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth=0.46" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=7.38 cfs 2.028 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth>1.47" Flow Length=850' Tc=25.6 min CN=WQ Runoff=6.88 cfs 0.911 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth>1.51" Flow Length=745' Tc=17.7 min CN=WQ Runoff=9.99 cfs 1.084 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth=0.57" Flow Length=688' Tc=33.2 min CN=WQ Runoff=2.40 cfs 0.399 af
Subcatchment 3-1-P: DA-3-	I-PRunoff Area=1.790 ac100.00% ImperviousRunoff Depth>2.31"Tc=0.0 minCN=98Runoff=5.87 cfs0.345 af
Subcatchment 3-2: DA-3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth>0.31" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=5.12 cfs 0.673 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth=0.42" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=2.73 cfs 0.554 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth>1.09" Flow Length=955' Tc=37.6 min CN=WQ Runoff=5.27 cfs 0.909 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth>1.29" Flow Length=625' Tc=33.8 min CN=WQ Runoff=7.06 cfs 1.118 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth>1.93" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=25.53 cfs 2.117 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth>2.00" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.13 cfs 0.018 af

Kittery_50%Build	(ID 2853680)	ME-De	nnettRoad 24-hr	S1 1-yr Rainfa	nll=2.63"
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Reach R1-1: R1-1	ہ n=0.035 L=8	Avg. Flow Depth=0.95' 00.0' S=0.0117 '/' Ca	Max Vel=3.32 fps apacity=90.60 cfs 0	Inflow=18.87 cfs Dutflow=18.51 cfs	3.059 af 3.059 af
Reach R1-2: R1-2	n=0.035 L=2	Avg. Flow Depth=0.10 20.0' S=0.0465 '/' Ca	)' Max Vel=1.81 fps apacity=180.91 cfs	s Inflow=0.57 cfs Outflow=0.57 cfs	0.896 af 0.896 af
Reach R1-3: R-1-3	ہ n=0.040 L=22	Avg. Flow Depth=1.57' 25.0' S=0.0022 '/' Ca	Max Vel=1.77 fps apacity=56.74 cfs (	Inflow=34.31 cfs Dutflow=34.06 cfs	6.675 af 6.671 af
Reach R1-4: R1-4	ہ n=0.035 L=290	Avg. Flow Depth=0.86' 0.0' S=0.0241 '/' Cap	Max Vel=4.54 fps bacity=130.35 cfs	Inflow=22.05 cfs Dutflow=21.95 cfs	2.488 af 2.488 af
Reach R1-5: R1-5			c	Inflow=34.06 cfs Dutflow=34.06 cfs	6.671 af 6.671 af
Reach R2-1: R2-1	n=0.035 L=₄	Avg. Flow Depth=0.46 460.0' S=0.0082 '/' C	5' Max Vel=1.87 fps Capacity=76.05 cfs	s Inflow=3.75 cfs Outflow=3.75 cfs	5.717 af 5.706 af
Reach R2-2: R2-2	n=0.030 L=-	Avg. Flow Depth=0.73 175.0' S=0.0171 '/' C	s' Max Vel=3.97 fps Capacity=82.75 cfs	s Inflow=9.99 cfs Outflow=9.97 cfs	1.084 af 1.084 af
Reach R2-3: R2-3	n=0.030 L=₄	Avg. Flow Depth=0.71 410.0' S=0.0110 '/' C	' Max Vel=3.14 fps Capacity=66.21 cfs	s Inflow=7.71 cfs Outflow=7.66 cfs	6.618 af 6.610 af
Reach R3-1: R3-1	n=0.030 L=1,5	Avg. Flow Depth=0.31 596.0' S=0.0203 '/' C	' Max Vel=2.71 fps Capacity=89.94 cfs	s Inflow=2.40 cfs Outflow=2.18 cfs	0.399 af 0.399 af
Reach R3-P: R3-P	n=0.040 L=	Avg. Flow Depth=0.00 =276.0' S=0.0056 '/'	" Max Vel=0.00 fps Capacity=3.77 cfs	s Inflow=0.00 cfs Outflow=0.00 cfs	0.000 af 0.000 af
Reach R4-1: R4-1	n=0.030 L=	Avg. Flow Depth=0.25 95.0' S=0.0632 '/' Ca	5' Max Vel=4.30 fps apacity=158.84 cfs	s Inflow=2.73 cfs Outflow=2.73 cfs	0.554 af 0.554 af
Reach R4-2: R4-2	n=0.030 L=14	Avg. Flow Depth=0.35 40.0' S=0.0714 '/' Ca	5' Max Vel=5.50 fps apacity=168.92 cfs	s Inflow=5.27 cfs Outflow=5.27 cfs	0.909 af 0.909 af
Reach R5-1: R5-1	n=0.035 L=64	Avg. Flow Depth=0.48 40.0' S=0.0234 '/' Ca	s' Max Vel=3.26 fps apacity=325.19 cfs	s Inflow=7.06 cfs Outflow=6.99 cfs	1.118 af 1.118 af
Reach R5-2: R5-2	/ n=0.035 L=70	Avg. Flow Depth=1.19' 00.0' S=0.0086 '/' Ca	Max Vel=3.22 fps apacity=77.67 cfs(	Inflow=27.76 cfs Dutflow=25.21 cfs	3.236 af 3.236 af
Reach R6-1: R6-1	n=0.030 L=3	Avg. Flow Depth=0.06 360.0' S=0.0203 '/' C	5' Max Vel=1.02 fps Capacity=90.06 cfs	s Inflow=0.13 cfs Outflow=0.12 cfs	0.018 af 0.018 af
Pond 1-P:	12.0" Rour	Peak Elev=38.46' nd Culvert n=0.012 L=	Storage=1.577 af :23.0' S=0.0000 '/'	Inflow=14.39 cfs Outflow=0.57 cfs	2.052 af 0.896 af
Pond 2-P:		Peak Elev=36.65'	Storage=3.391 af	Inflow=26.12 cfs Outflow=3.75 cfs	6.722 af 5.717 af

Kittery_50%Build (ID 2853680 Prepared by Barton & Loguidice.	) ME-DennettRoad 24-hr S1 1-yr Rainf DPC Printed	all=2.63" 8/7/2023
HydroCAD® 10.20-2g s/n 05255 © 20	022 HydroCAD Software Solutions LLC	Page 10
Pond 3-P:	Peak Elev=86.24' Storage=0.345 af Inflow=5.87 cfs Outflow=0.00 cfs	s 0.345 af s 0.000 af
Pond CB-1-1: L1-1	Peak Elev=79.53' Inflow=18.87 cfs 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=18.87 cfs	3.059 af 3.059 af
Pond CB-1-3: L1-3	Peak Elev=39.99' Inflow=34.31 cfs 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=34.31 cfs	6.675 af 6.675 af
Pond CB-1-4: L1-4	Peak Elev=40.41' Inflow=22.05 cfs 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=22.05 cfs	<ul><li>2.488 af</li><li>2.488 af</li></ul>
Pond CB-2-3: L2-2	Peak Elev=31.05' Inflow=7.71 cfs 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=7.71 cfs	s 6.618 af s 6.618 af
Pond CB-2-4: L2-3	Peak Elev=32.02' Inflow=9.99 cfs 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=9.99 cfs	s 1.084 af s 1.084 af
Pond CB-3-1: L3-1	Peak Elev=87.28' Inflow=2.40 cfs Outflow=2.40 cfs	s 0.399 af s 0.399 af
Pond CB-3-2: L3-2	Peak Elev=55.08' Inflow=6.14 cfs 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=6.14 cfs	s 1.073 af s 1.073 af
Pond CB-4-1: L4-1	Peak Elev=54.69' Inflow=2.73 cfs 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=2.73 cfs	s 0.554 af s 0.554 af
Pond CB-4-2: L4-2	Peak Elev=54.99' Inflow=5.27 cfs 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=5.27 cfs	s 0.909 af s 0.909 af
Pond CB-5-1: L5-1	Peak Elev=57.00' Inflow=7.06 cfs 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=7.06 cfs	s 1.118 af s 1.118 af
<b>Pond CB-5-2: L5-2</b>	Peak Elev=42.39' Inflow=27.76 cfs 2.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=27.76 cfs	s 3.236 af s 3.236 af
Pond CB-6-1: L6-1	Peak Elev=49.41' Inflow=0.13 cfs 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.13 cfs	s 0.018 af s 0.018 af
Link L1-5: L1-5	Inflow=34.06 cfs Primary=34.06 cfs	s 6.671 af s 6.671 af
Link L1-6: L1-6	Inflow=55.61 cfs Primary=55.61 cfs	s 9.159 af s 9.159 af
Link L2-1: L2-1	Inflow=3.75 cfs Primary=3.75 cfs	s 5.717 af s 5.717 af
Link L2-4: L2-4	Inflow=7.66 cfs Primary=7.66 cfs	6.610 af 6.610 af

Kittery_50%Build (ID 2853680)	ME-DennettRoad 24-hr S1 1-yr Rainfall=2.63
Prepared by Barton & Loguidice, DPC	Printed 8/7/2023
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Link L2-5: L2-5	Inflow=9.97 cfs 1.084 a
	Primary=9.97 cts 1.084 a
Link L2-6: L2-6	Inflow=16.60 cfs 7.694 a
	Primary=16.60 cfs 7.694 a
l ink   4-3 ·   4-3	Inflow=2 73 cfs_0 554 a
	Primary=2.73 cfs 0.554 a
	Inflow 5.27 efc. 0.000 c
LINK L4-4: L4-4	Innow=5.27 cts 0.909 a
	Phinary=5.27 crs 0.909 a
Link L4-5: L4-5	Inflow=7.97 cfs 1.462 a
	Primary=7.97 cfs 1.462 a
Link L5-3: L5-3	Inflow=25.21 cfs_3.236 a
	Primary=25.21 cfs 3.236 a
l ink   6-2.   6-2	Inflow-0.12 cfs. 0.018 a
LIIR LV-2. LV-2	Primary-0.12 cls 0.010 a
	1 hindry=0.12 CIS 0.010 a
Total Runoff Area = 323.908 ac	Runoff Volume = 25.169 af Average Runoff Depth = 0.93

96.72% Pervious = 313.275 ac 3.28% Impervious = 10.633 ac

Kittery\_50%Build (ID 2853680)ME-DennettRoad 24-hr S1 2-yrRainfall=3.31"Prepared by Barton & Loguidice, DPCPrinted8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 12

### Time span=5.00-36.00 hrs, dt=0.02 hrs, 1551 points Runoff by SCS TR-20 method, UH=SCS, Weighted-Q Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1-1: DA-1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth>1.37" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=28.60 cfs 4.676 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth>1.61" Flow Length=485' Tc=27.5 min CN=WQ Runoff=20.75 cfs 3.023 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth>2.21" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=33.03 cfs 3.718 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth>2.21" Flow Length=875' Tc=18.9 min CN=WQ Runoff=28.92 cfs 3.409 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth>1.26" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=28.32 cfs 7.141 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth=0.76" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=12.24 cfs 3.313 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth>2.06" Flow Length=850' Tc=25.6 min CN=WQ Runoff=9.26 cfs 1.278 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth>2.10" Flow Length=745' Tc=17.7 min CN=WQ Runoff=13.33 cfs 1.508 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth>0.89" Flow Length=688' Tc=33.2 min CN=WQ Runoff=3.74 cfs 0.629 af
Subcatchment 3-1-P: DA-3-	<b>1-P</b> Runoff Area=1.790 ac100.00% ImperviousRunoff Depth>2.94"Tc=0.0 minCN=98Runoff=7.12 cfs0.438 af
Subcatchment 3-2: DA-3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth>0.51" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=7.99 cfs 1.099 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth=0.67" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=4.27 cfs 0.874 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth>1.58" Flow Length=955' Tc=37.6 min CN=WQ Runoff=7.38 cfs 1.318 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth>1.84" Flow Length=625' Tc=33.8 min CN=WQ Runoff=9.63 cfs 1.586 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth>2.56" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=32.21 cfs 2.805 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth>2.64" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.16 cfs 0.023 af

Kittery_50%Build	(ID 2853680)		ME-D	ennettRoad 24	4-hr S1 2-yr	Rainfa	<i>II</i> <b>=</b> 3.31"
Prepared by Barton	& Loguidice, DPC		<b>.</b>		Pri	nted 8	8/7/2023
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Reach R1-1: R1-1	n=0.035 l	Avg. Flov ==800.0' S	w Depth=1.17 =0.0117 '/' 0	7' Max Vel=3.72 Capacity=90.60 c	2 fps Inflow=28 fs Outflow=28	.60 cfs .18 cfs	4.676 af 4.676 af
Reach R1-2: R1-2	n=0.035	Avg. Fle L=20.0' S	ow Depth=0.1 =0.0465 '/' 0	13' Max Vel=2.1 Capacity=180.91	8 fps Inflow=0 cfs Outflow=0	.96 cfs .96 cfs	1.527 af 1.527 af
Reach R1-3: R-1-3	n=0.040 l	Avg. Flov ==225.0' S	w Depth=1.85 =0.0022 '/'	5' Max Vel=1.94 Capacity=56.74 c	fps Inflow=48 fs Outflow=48	.47 cfs .23 cfs	9.921 af 9.915 af
Reach R1-4: R1-4	n=0.035 L=	Avg. Flo =290.0' S=	w Depth=0.99 0.0241 '/' Ca	9' Max Vel=4.89 apacity=130.35 c	) fps Inflow=28 fs Outflow=28	.92 cfs .81 cfs	3.409 af 3.409 af
Reach R1-5: R1-5					Inflow=48 Outflow=48	.23 cfs .23 cfs	9.915 af 9.915 af
Reach R2-1: R2-1	n=0.035	Avg. Fl L=460.0'	ow Depth=0.5 S=0.0082 '/'	57' Max Vel=2.1 Capacity=76.05	1 fps Inflow=5 cfs Outflow=5	.68 cfs .68 cfs	8.735 af 8.721 af
Reach R2-2: R2-2	n=0.030 l	Avg. Flov =175.0' S	w Depth=0.84 =0.0171 '/' (	l' Max Vel=4.29 Capacity=82.75 c	fps Inflow=13 fs Outflow=13	.33 cfs .31 cfs	1.508 af 1.508 af
Reach R2-3: R2-3	n=0.030 l	Avg. Flov ==410.0' S	w Depth=0.83 =0.0110 '/' 0	8' Max Vel=3.40 Capacity=66.21 c	) fps Inflow=10 fs Outflow=10	.34 cfs .28 cfs	9.999 af 9.990 af
Reach R3-1: R3-1	n=0.030 L	Avg. Fl =1,596.0'	ow Depth=0.4 S=0.0203 '/'	40' Max Vel=3.1 Capacity=89.94	2 fps Inflow=3 cfs Outflow=3	.74 cfs .48 cfs	0.629 af 0.629 af
Reach R3-P: R3-P	n=0.040	Avg. Fl L=276.0'	ow Depth=0.0 S=0.0056 '/'	00' Max Vel=0.0 Capacity=3.77	0 fps Inflow=0 cfs Outflow=0	.00 cfs .00 cfs	0.000 af 0.000 af
Reach R4-1: R4-1	n=0.030	Avg. Fl L=95.0' S	ow Depth=0.3 =0.0632 '/' 0	33' Max Vel=4.9 Capacity=158.84	94 fps Inflow=4 cfs Outflow=4	.27 cfs .27 cfs	0.874 af 0.874 af
Reach R4-2: R4-2	n=0.030 L	Avg. Fl _=140.0' S	ow Depth=0.4 =0.0714 '/' 0	13' Max Vel=6.0 Capacity=168.92	8 fps Inflow=7 cfs Outflow=7	.38 cfs .38 cfs	1.318 af 1.318 af
Reach R5-1: R5-1	n=0.035 l	Avg. Fl =640.0' S	ow Depth=0.5 =0.0234 '/' 0	57' Max Vel=3.5 Capacity=325.19	6 fps Inflow=9 cfs Outflow=9	.63 cfs .54 cfs	1.586 af 1.586 af
Reach R5-2: R5-2	n=0.035 L	Avg. Flov ==700.0' S	w Depth=1.34 =0.0086 '/'	4' Max Vel=3.45 Capacity=77.67 c	fps Inflow=35 fs Outflow=32	.42 cfs .49 cfs	4.390 af 4.390 af
Reach R6-1: R6-1	n=0.030	Avg. Fl L=360.0'	ow Depth=0.0 S=0.0203 '/'	)7' Max Vel=1.1 Capacity=90.06	1 fps Inflow=0 cfs Outflow=0	.16 cfs .16 cfs	0.023 af 0.023 af
Pond 1-P:	12.0" R	Pe ound Culve	ak Elev=38.6 ert_n=0.012_L	4' Storage=2.17 .=23.0' S=0.000	8 af Inflow=20 0 '/' Outflow=0	.75 cfs .96 cfs	3.023 af 1.527 af
Pond 2-P:		Pea	k Elev=37.04	Storage=5.400	af Inflow=40.4 Outflow=5	3 cfs 1 .68 cfs	0.454 af 8.735 af

Kittery_50%Build (ID 28536 Prepared by Barton & Loguidic HydroCAD® 10.20-2g s/n 05255 ©	80)ME-DennettRoad 24-hr S1 2-yrRainfall=3.31"ce, DPCPrinted 8/7/20232022 HydroCAD Software Solutions LLCPage 14	
Pond 3-P:	Peak Elev=86.30' Storage=0.438 af Inflow=7.12 cfs 0.438 af Outflow=0.00 cfs 0.000 af	
Pond CB-1-1: L1-1	Peak Elev=121.15' Inflow=28.60 cfs 4.676 af 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=28.60 cfs 4.676 af	
Pond CB-1-3: L1-3	Peak Elev=41.11' Inflow=48.47 cfs 9.921 af 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=48.47 cfs 9.921 af	
Pond CB-1-4: L1-4	Peak Elev=42.87' Inflow=28.92 cfs 3.409 af 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=28.92 cfs 3.409 af	
Pond CB-2-3: L2-2	Peak Elev=31.23' Inflow=10.34 cfs 9.999 af 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=10.34 cfs 9.999 af	
Pond CB-2-4: L2-3	Peak Elev=32.52' Inflow=13.33 cfs 1.508 af 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=13.33 cfs 1.508 af	
Pond CB-3-1: L3-1	Peak Elev=87.48' Inflow=3.74 cfs 0.629 af Outflow=3.74 cfs 0.629 af	
Pond CB-3-2: L3-2	Peak Elev=55.44' Inflow=9.84 cfs 1.728 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=9.84 cfs 1.728 af	
Pond CB-4-1: L4-1	Peak Elev=54.88' Inflow=4.27 cfs 0.874 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=4.27 cfs 0.874 af	
Pond CB-4-2: L4-2	Peak Elev=55.20' Inflow=7.38 cfs 1.318 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=7.38 cfs 1.318 af	
Pond CB-5-1: L5-1	Peak Elev=57.19' Inflow=9.63 cfs 1.586 af 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=9.63 cfs 1.586 af	
Pond CB-5-2: L5-2	Peak Elev=43.07' Inflow=35.42 cfs 4.390 af 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=35.42 cfs 4.390 af	
Pond CB-6-1: L6-1	Peak Elev=49.42' Inflow=0.16 cfs 0.023 af 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.16 cfs 0.023 af	
Link L1-5: L1-5	Inflow=48.23 cfs 9.915 af Primary=48.23 cfs 9.915 af	
Link L1-6: L1-6	Inflow=76.29 cfs 13.325 af Primary=76.29 cfs 13.325 af	
Link L2-1: L2-1	Inflow=5.68 cfs 8.735 af Primary=5.68 cfs 8.735 af	
Link L2-4: L2-4	Inflow=10.28 cfs 9.990 af Primary=10.28 cfs 9.990 af	

Kittery_50%Build (ID 2853680)	ME-DennettRoa	ME-DennettRoad 24-hr S1 2-yr Rainfall=3.31"			
Prepared by Barton & Loguidice, DPC		Printed 8/7/2023			
HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD	O Software Solutions LLC	Page 15			
Link L2-5: L2-5		Inflow=13.31 cfs 1.508 af			
		Primary=13.31 cfs 1.508 af			
Link L2-6: L2-6		Inflow=22.23 cfs 11.498 af			
		Primary=22.23 cfs 11.498 af			
Link L4-3: L4-3		Inflow=4.27 cfs 0.874 af			
		Primary=4.27 cfs 0.874 af			
Link L4-4: L4-4		Inflow=7.38 cfs 1.318 af			
		Primary=7.38 cfs 1.318 af			
Link L4-5: L4-5		Inflow=11.60 cfs 2.192 af			
		Primary=11.60 cfs 2.192 af			
Link L5-3: L5-3		Inflow=32.49 cfs 4.390 af			
		Primary=32.49 cfs 4.390 af			
Link L6-2: L6-2		Inflow=0.16 cfs 0.023 af			
-		Primary=0.16 cfs 0.023 af			
Total Runoff Area = 323.908 ac	Runoff Volume = 36.838 af	Average Runoff Depth = 1.36"			

96.72% Pervious = 313.275 ac 3.28% Impervious = 10.633 ac

Kittery\_50%Build (ID 2853680)ME-DennettRoad 24-hr S1 5-yrRainfall=4.41"Prepared by Barton & Loguidice, DPCPrinted8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 16

Time span=5.00-36.00 hrs, dt=0.02 hrs, 1551 points Runoff by SCS TR-20 method, UH=SCS, Weighted-Q Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1-1: DA-1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth>2.22" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=45.59 cfs 7.566 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth>2.52" Flow Length=485' Tc=27.5 min CN=WQ Runoff=31.64 cfs 4.721 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth>3.19" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=45.92 cfs 5.380 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth>3.21" Flow Length=875' Tc=18.9 min CN=WQ Runoff=40.26 cfs 4.944 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth>2.04" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=44.92 cfs 11.511 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth>1.32" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=22.37 cfs 5.793 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth>3.05" Flow Length=850' Tc=25.6 min CN=WQ Runoff=13.21 cfs 1.897 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth>3.10" Flow Length=745' Tc=17.7 min CN=WQ Runoff=18.85 cfs 2.222 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth>1.50" Flow Length=688' Tc=33.2 min CN=WQ Runoff=6.21 cfs 1.061 af
Subcatchment 3-1-P: DA-3-	I-P Runoff Area=1.790 ac 100.00% Impervious Runoff Depth>3.94"   Tc=0.0 min CN=98 Runoff=9.15 cfs 0.588 af
Subcatchment 3-2: DA-3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth>0.90" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=14.21 cfs 1.939 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth>1.13" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=7.37 cfs 1.480 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth>2.44" Flow Length=955' Tc=37.6 min CN=WQ Runoff=11.16 cfs 2.037 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth>2.76" Flow Length=625' Tc=33.8 min CN=WQ Runoff=14.04 cfs 2.388 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth>3.58" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=43.04 cfs 3.925 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth>3.67" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.22 cfs 0.032 af

Kittery_50%Build	(ID 2853680) ME-DennettRoad 24-hr S1 5-yr Rainfall=4.41"
Prepared by Barton	& Loguidice, DPC Printed 8/7/2023
HydroCAD® 10.20-2g	/n 05255 © 2022 HydroCAD Software Solutions LLC Page 17
Reach R1-1: R1-1	Avg. Flow Depth=1.46' Max Vel=4.21 fps Inflow=45.59 cfs 7.566 af n=0.035 L=800.0' S=0.0117 '/' Capacity=90.60 cfs Outflow=45.09 cfs 7.566 af
Reach R1-2: R1-2	Avg. Flow Depth=0.18' Max Vel=2.65 fps Inflow=1.71 cfs 2.736 af n=0.035 L=20.0' S=0.0465 '/' Capacity=180.91 cfs Outflow=1.71 cfs 2.736 af
Reach R1-3: R-1-3	Avg. Flow Depth=2.27' Max Vel=2.15 fps Inflow=73.07 cfs 15.681 af n=0.040 L=225.0' S=0.0022 '/' Capacity=56.74 cfs Outflow=72.81 cfs 15.674 af
Reach R1-4: R1-4	Avg. Flow Depth=1.16' Max Vel=5.33 fps Inflow=40.26 cfs 4.944 af n=0.035 L=290.0' S=0.0241 '/' Capacity=130.35 cfs Outflow=40.12 cfs 4.944 af
Reach R1-5: R1-5	Inflow=72.81 cfs 15.674 af Outflow=72.81 cfs 15.674 af
Reach R2-1: R2-1	Avg. Flow Depth=0.69' Max Vel=2.35 fps Inflow=8.25 cfs 13.808 af n=0.035 L=460.0' S=0.0082 '/' Capacity=76.05 cfs Outflow=8.25 cfs 13.786 af
Reach R2-2: R2-2	Avg. Flow Depth=1.00' Max Vel=4.70 fps Inflow=18.85 cfs 2.222 af n=0.030 L=175.0' S=0.0171 '/' Capacity=82.75 cfs Outflow=18.81 cfs 2.222 af
Reach R2-3: R2-3	Avg. Flow Depth=1.00' Max Vel=3.76 fps Inflow=14.99 cfs 15.683 af n=0.030 L=410.0' S=0.0110 '/' Capacity=66.21 cfs Outflow=14.92 cfs 15.668 af
Reach R3-1: R3-1	Avg. Flow Depth=0.53' Max Vel=3.64 fps Inflow=6.21 cfs 1.061 af n=0.030 L=1,596.0' S=0.0203 '/' Capacity=89.94 cfs Outflow=5.87 cfs 1.061 af
Reach R3-P: R3-P	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.040 L=276.0' S=0.0056 '/' Capacity=3.77 cfs Outflow=0.00 cfs 0.000 af
Reach R4-1: R4-1	Avg. Flow Depth=0.44' Max Vel=5.82 fps Inflow=7.37 cfs 1.480 af n=0.030 L=95.0' S=0.0632 '/' Capacity=158.84 cfs Outflow=7.37 cfs 1.480 af
Reach R4-2: R4-2	Avg. Flow Depth=0.53' Max Vel=6.85 fps Inflow=11.16 cfs 2.037 af n=0.030 L=140.0' S=0.0714 '/' Capacity=168.92 cfs Outflow=11.16 cfs 2.037 af
Reach R5-1: R5-1	Avg. Flow Depth=0.69' Max Vel=3.97 fps Inflow=14.04 cfs 2.388 af n=0.035 L=640.0' S=0.0234 '/' Capacity=325.19 cfs Outflow=13.94 cfs 2.388 af
Reach R5-2: R5-2	Avg. Flow Depth=1.55' Max Vel=3.74 fps Inflow=48.03 cfs 6.312 af n=0.035 L=700.0' S=0.0086 '/' Capacity=77.67 cfs Outflow=44.56 cfs 6.312 af
Reach R6-1: R6-1	Avg. Flow Depth=0.08' Max Vel=1.23 fps Inflow=0.22 cfs 0.032 af n=0.030 L=360.0' S=0.0203 '/' Capacity=90.06 cfs Outflow=0.21 cfs 0.032 af
Pond 1-P:	Peak Elev=38.94' Storage=3.188 af Inflow=31.64 cfs 4.721 af 12.0" Round Culvert n=0.012 L=23.0' S=0.0000 '/' Outflow=1.71 cfs 2.736 af
Pond 2-P:	Peak Elev=37.84' Storage=9.584 af Inflow=67.15 cfs 17.303 af Outflow=8.25 cfs 13.808 af

Kittery_50%Build (ID 2853) Prepared by Barton & Loguid	ME-DennettRoad 24-hr S1 5-yr Rainfall=4.41" dice, DPC Printed 8/7/2023	
HydroCAD® 10.20-2g s/n 05255	© 2022 HydroCAD Software Solutions LLC Page 18	
Pond 3-P:	Peak Elev=86.40' Storage=0.588 af Inflow=9.15 cfs 0.588 af Outflow=0.00 cfs 0.000 af	
Pond CB-1-1: L1-1	Peak Elev=234.94' Inflow=45.59 cfs 7.566 af 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=45.59 cfs 7.566 af	
Pond CB-1-3: L1-3	Peak Elev=42.77' Inflow=73.07 cfs 15.681 af 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=73.07 cfs 15.681 af	
Pond CB-1-4: L1-4	Peak Elev=49.72' Inflow=40.26 cfs 4.944 af 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=40.26 cfs 4.944 af	
Pond CB-2-3: L2-2	Peak Elev=31.51' Inflow=14.99 cfs 15.683 af 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=14.99 cfs 15.683 af	
Pond CB-2-4: L2-3	Peak Elev=33.28' Inflow=18.85 cfs 2.222 af 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=18.85 cfs 2.222 af	
Pond CB-3-1: L3-1	Peak Elev=87.86' Inflow=6.21 cfs 1.061 af Outflow=6.21 cfs 1.061 af	
Pond CB-3-2: L3-2	Peak Elev=56.35' Inflow=17.59 cfs 3.000 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=17.59 cfs 3.000 af	
Pond CB-4-1: L4-1	Peak Elev=55.20' Inflow=7.37 cfs 1.480 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=7.37 cfs 1.480 af	
Pond CB-4-2: L4-2	Peak Elev=55.56' Inflow=11.16 cfs 2.037 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=11.16 cfs 2.037 af	
Pond CB-5-1: L5-1	Peak Elev=57.46' Inflow=14.04 cfs 2.388 af 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=14.04 cfs 2.388 af	
Pond CB-5-2: L5-2	Peak Elev=44.52' Inflow=48.03 cfs 6.312 af 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=48.03 cfs 6.312 af	
Pond CB-6-1: L6-1	Peak Elev=49.45' Inflow=0.22 cfs 0.032 af 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.22 cfs 0.032 af	
Link L1-5: L1-5	Inflow=72.81 cfs 15.674 af Primary=72.81 cfs 15.674 af	
Link L1-6: L1-6	Inflow=111.40 cfs 20.618 af Primary=111.40 cfs 20.618 af	
Link L2-1: L2-1	Inflow=8.25 cfs 13.808 af Primary=8.25 cfs 13.808 af	
Link L2-4: L2-4	Inflow=14.92 cfs 15.668 af Primary=14.92 cfs 15.668 af	

Kittery_50%Build (ID 2853680)	ME-DennettRoad 24-hr S1 5-yr Rainfall=4.41"	
Prepared by Barton & Loguidice, DPC	Software Solutions IIC	Printed 8/7/2023
Hydrocade 10.20-2g S/1105255 @ 2022 Hydrocad	Software Solutions LLC	Page 19
Link L2-5: L2-5		Inflow=18.81 cfs 2.222 af
		Primary=18.81 cfs 2.222 af
Link L2-6: L2-6		Inflow=31.84 cfs 17.890 af
		Primary=31.84 cfs 17.890 af
Link L4-3: L4-3		Inflow=7.37 cfs 1.480 af
		Primary=7.37 cfs 1.480 af
Link L4-4: L4-4		Inflow=11.16 cfs 2.037 af
		Primary=11.16 cfs 2.037 af
Link L4-5: L4-5		Inflow=18.47 cfs 3.517 af
		Primary=18.47 cfs 3.517 af
Link L5-3: L5-3		Inflow=44.56 cfs 6.312 af
		Primary=44.56 cfs 6.312 af
Link L6-2: L6-2		Inflow=0.21 cfs 0.032 af
		Primary=0.21 cfs 0.032 af
Total Runoff Area - 323 908 ac	Runoff Volume – 57 483 af	Average Runoff Depth - 2 13"

Total Runoff Area = 323.908 acRunoff Volume = 57.483 afAverage Runoff Depth = 2.13"96.72% Pervious = 313.275 ac3.28% Impervious = 10.633 ac

Kittery\_50%Build (ID 2853680)ME-DennettRoad 24-hr S1 10-yr Rainfall=5.32"Prepared by Barton & Loguidice, DPCPrinted 8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 20

Time span=5.00-36.00 hrs, dt=0.02 hrs, 1551 points Runoff by SCS TR-20 method, UH=SCS, Weighted-Q Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1-1: DA-1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth>2.97" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=59.93 cfs 10.120 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth>3.31" Flow Length=485' Tc=27.5 min CN=WQ Runoff=40.72 cfs 6.200 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth>4.02" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=56.41 cfs 6.777 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth>4.05" Flow Length=875' Tc=18.9 min CN=WQ Runoff=49.46 cfs 6.233 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth>2.72" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=58.98 cfs 15.387 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth>1.85" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=31.62 cfs 8.122 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth>3.89" Flow Length=850' Tc=25.6 min CN=WQ Runoff=16.40 cfs 2.419 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth>3.94" Flow Length=745' Tc=17.7 min CN=WQ Runoff=23.32 cfs 2.822 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth>2.07" Flow Length=688' Tc=33.2 min CN=WQ Runoff=8.38 cfs 1.461 af
Subcatchment 3-1-P: DA-3-	I-P Runoff Area=1.790 ac 100.00% Impervious Runoff Depth>4.77"   Tc=0.0 min CN=98 Runoff=10.82 cfs 0.711 af
Subcatchment 3-2: DA-3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth>1.29" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=20.07 cfs 2.758 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth>1.57" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=10.15 cfs 2.047 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth>3.19" Flow Length=955' Tc=37.6 min CN=WQ Runoff=14.34 cfs 2.665 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth>3.56" Flow Length=625' Tc=33.8 min CN=WQ Runoff=17.67 cfs 3.075 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth>4.42" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=51.79 cfs 4.851 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth>4.52" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.26 cfs 0.040 af

Kittery_50%Build	(ID 2853680)	ME-DennettRoad 24-hr S1 10-yr Rainfall=5.32"
Prepared by Barton	& Loguidice, DPC	Printed 8/7/2023
HydroCAD® 10.20-2g	s/n 05255 © 2022 Hy	droCAD Software Solutions LLC Page 21
Reach R1-1: R1-1	n=0.035	Avg. Flow Depth=1.65' Max Vel=4.52 fps Inflow=59.93 cfs 10.120 af L=800.0' S=0.0117 '/' Capacity=90.60 cfs Outflow=59.37 cfs 10.120 af
Reach R1-2: R1-2	n=0.03	Avg. Flow Depth=0.21' Max Vel=2.92 fps Inflow=2.28 cfs 3.784 af 5 L=20.0' S=0.0465 '/' Capacity=180.91 cfs Outflow=2.28 cfs 3.784 af
Reach R1-3: R-1-3	n=0.040	Avg. Flow Depth=2.61' Max Vel=2.26 fps Inflow=93.81 cfs 20.681 af L=225.0' S=0.0022 '/' Capacity=56.74 cfs Outflow=93.49 cfs 20.672 af
Reach R1-4: R1-4	n=0.035	Avg. Flow Depth=1.28' Max Vel=5.63 fps Inflow=49.46 cfs 6.233 af L=290.0' S=0.0241 '/' Capacity=130.35 cfs Outflow=49.30 cfs 6.233 af
Reach R1-5: R1-5		Inflow=93.49 cfs 20.672 af Outflow=93.49 cfs 20.672 af
Reach R2-1: R2-1	n=0.035	Avg. Flow Depth=1.17' Max Vel=3.13 fps Inflow=24.03 cfs 19.259 af L=460.0' S=0.0082 '/' Capacity=76.05 cfs Outflow=24.02 cfs 19.234 af
Reach R2-2: R2-2	n=0.030	Avg. Flow Depth=1.11' Max Vel=4.97 fps Inflow=23.32 cfs 2.822 af L=175.0' S=0.0171 '/' Capacity=82.75 cfs Outflow=23.28 cfs 2.822 af
Reach R2-3: R2-3	n=0.030	Avg. Flow Depth=1.29' Max Vel=4.33 fps Inflow=25.63 cfs 21.653 af L=410.0' S=0.0110 '/' Capacity=66.21 cfs Outflow=25.63 cfs 21.637 af
Reach R3-1: R3-1	n=0.030	Avg. Flow Depth=0.62' Max Vel=3.96 fps Inflow=8.38 cfs 1.461 af L=1,596.0' S=0.0203 '/' Capacity=89.94 cfs Outflow=7.98 cfs 1.461 af
Reach R3-P: R3-P	n=0.04	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af 40 L=276.0' S=0.0056 '/' Capacity=3.77 cfs Outflow=0.00 cfs 0.000 af
Reach R4-1: R4-1	n=0.030	Avg. Flow Depth=0.52' Max Vel=6.38 fps Inflow=10.15 cfs 2.047 af L=95.0' S=0.0632 '/' Capacity=158.84 cfs Outflow=10.15 cfs 2.047 af
Reach R4-2: R4-2	n=0.030	Avg. Flow Depth=0.61' Max Vel=7.35 fps Inflow=14.34 cfs 2.665 af L=140.0' S=0.0714 '/' Capacity=168.92 cfs Outflow=14.34 cfs 2.665 af
Reach R5-1: R5-1	n=0.035	Avg. Flow Depth=0.78' Max Vel=4.23 fps Inflow=17.67 cfs 3.075 af L=640.0' S=0.0234 '/' Capacity=325.19 cfs Outflow=17.57 cfs 3.075 af
Reach R5-2: R5-2	n=0.035	Avg. Flow Depth=1.70' Max Vel=3.94 fps Inflow=58.29 cfs 7.926 af L=700.0' S=0.0086 '/' Capacity=77.67 cfs Outflow=54.42 cfs 7.926 af
Reach R6-1: R6-1	n=0.030	Avg. Flow Depth=0.09' Max Vel=1.32 fps Inflow=0.26 cfs 0.040 af 0 L=360.0' S=0.0203 '/' Capacity=90.06 cfs Outflow=0.25 cfs 0.040 af
Pond 1-P:	12.0"	Peak Elev=39.21' Storage=4.113 af Inflow=40.72 cfs 6.200 af Round Culvert n=0.012 L=23.0' S=0.0000 '/' Outflow=2.28 cfs 3.784 af
Pond 2-P:		Peak Elev=38.11' Storage=11.246 af Inflow=90.38 cfs 23.509 af Outflow=24.03 cfs 19.259 af
Kittery_50%Build (ID 2853 Prepared by Barton & Loguid HydroCAD® 10.20-2g s/n 05255	680)ME-DennettRoad 24-hr S1 10-yr Rainfall=5.32"ice, DPCPrinted 8/7/2023© 2022 HydroCAD Software Solutions LLCPage 22	
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Pond 3-P:	Peak Elev=86.49' Storage=0.711 af Inflow=10.82 cfs 0.711 af Outflow=0.00 cfs 0.000 af	
Pond CB-1-1: L1-1	Peak Elev=371.42' Inflow=59.93 cfs 10.120 af 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=59.93 cfs 10.120 af	
Pond CB-1-3: L1-3	Peak Elev=44.69' Inflow=93.81 cfs 20.681 af 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=93.81 cfs 20.681 af	
Pond CB-1-4: L1-4	Peak Elev=57.21' Inflow=49.46 cfs 6.233 af 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=49.46 cfs 6.233 af	
Pond CB-2-3: L2-2	Peak Elev=32.08' Inflow=25.63 cfs 21.653 af 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=25.63 cfs 21.653 af	
Pond CB-2-4: L2-3	Peak Elev=33.96' Inflow=23.32 cfs 2.822 af 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=23.32 cfs 2.822 af	
Pond CB-3-1: L3-1	Peak Elev=88.29' Inflow=8.38 cfs 1.461 af Outflow=8.38 cfs 1.461 af	
Pond CB-3-2: L3-2	Peak Elev=57.68' Inflow=24.79 cfs 4.219 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=24.79 cfs 4.219 af	
Pond CB-4-1: L4-1	Peak Elev=55.46' Inflow=10.15 cfs 2.047 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=10.15 cfs 2.047 af	
Pond CB-4-2: L4-2	Peak Elev=55.88' Inflow=14.34 cfs 2.665 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=14.34 cfs 2.665 af	
Pond CB-5-1: L5-1	Peak Elev=57.66' Inflow=17.67 cfs 3.075 af 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=17.67 cfs 3.075 af	
Pond CB-5-2: L5-2	Peak Elev=46.03' Inflow=58.29 cfs 7.926 af 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=58.29 cfs 7.926 af	
Pond CB-6-1: L6-1	Peak Elev=49.47' Inflow=0.26 cfs 0.040 af 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.26 cfs 0.040 af	
Link L1-5: L1-5	Inflow=93.49 cfs 20.672 af Primary=93.49 cfs 20.672 af	
Link L1-6: L1-6	Inflow=140.50 cfs 26.905 af Primary=140.50 cfs 26.905 af	
Link L2-1: L2-1	Inflow=24.03 cfs 19.259 af Primary=24.03 cfs 19.259 af	
Link L2-4: L2-4	Inflow=25.63 cfs 21.637 af Primary=25.63 cfs 21.637 af	

Kittery_50%Build (ID 2853680)	ME-DennettRoad 24-hr S1 10-yr Rainfall=5.3	ME-DennettRoad 24-hr S1 10-yr Rainfall=5.32"	
Prepared by Barton & Loguidice, DPC	Printed 8/7/202	23	
HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD	Software Solutions LLC Page 2	<u>23</u>	
Link L2-5: L2-5	Inflow=23.28 cfs 2.822	af	
	Primary=23.28 cfs 2.822	af	
Link L2-6: L2-6	Inflow=39.97 cfs 24.459	af	
	Primary=39.97 cfs 24.459	af	
Link L4-3: L4-3	Inflow=10.15 cfs 2.047	af	
	Primary=10.15 cfs 2.047	af	
l ink   4-4 ·   4-4	Inflow=14.34 cfs. 2.665	af	
	Primary=14.34 cfs 2.665	af	
link   4 5   1 4 5	Inflow-24.43 efc. 4.712	of	
LIIIK L4-3. L4-3	Primary-24.43 cfs 4.712	ai af	
	1 milling=24.40 010 4.712	u	
Link L5-3: L5-3	Inflow=54.42 cfs 7.926	af	
	Primary=54.42 cfs 7.926	af	
Link L6-2: L6-2	Inflow=0.25 cfs 0.040	af	
	Primary=0.25 cfs 0.040	af	
Total Runoff Area = 323.908 ac	Runoff Volume = 75.690 af Average Runoff Depth = 2.80	0"	

96.72% Pervious = 313.275 ac 3.28% Impervious = 10.633 ac

Kittery\_50%Build (ID 2853680)ME-DennettRoad 24-hr S1 25-yrRainfall=6.58"Prepared by Barton & Loguidice, DPCPrinted 8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 24

Subcatchment 1-1: DA-1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth>4.05" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=79.98 cfs 13.817 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth>4.43" Flow Length=485' Tc=27.5 min CN=WQ Runoff=53.33 cfs 8.315 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth>5.18" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=70.77 cfs 8.728 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth>5.21" Flow Length=875' Tc=18.9 min CN=WQ Runoff=62.01 cfs 8.030 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth>3.72" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=78.71 cfs 21.046 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth>2.67" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=45.20 cfs 11.692 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth>5.07" Flow Length=850' Tc=25.6 min CN=WQ Runoff=20.75 cfs 3.148 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth>5.10" Flow Length=745' Tc=17.7 min CN=WQ Runoff=29.42 cfs 3.660 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth>2.93" Flow Length=688' Tc=33.2 min CN=WQ Runoff=11.56 cfs 2.066 af
Subcatchment 3-1-P: DA-3-	I-P Runoff Area=1.790 ac 100.00% Impervious Runoff Depth>5.91" Tc=0.0 min CN=98 Runoff=13.11 cfs 0.882 af
Subcatchment 3-2: DA-3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth>1.90" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=28.92 cfs 4.079 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth>2.24" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=14.22 cfs 2.935 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth>4.27" Flow Length=955' Tc=37.6 min CN=WQ Runoff=18.79 cfs 3.564 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth>4.68" Flow Length=625' Tc=33.8 min CN=WQ Runoff=22.69 cfs 4.047 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth>5.59" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=63.73 cfs 6.130 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth>5.69" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.32 cfs 0.050 af

Kittery_50%Build	(ID 2853680) ME-DennettRoad 24-hr S1 25-yr Rainfall=6.58"
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Reach R1-1: R1-1	Avg. Flow Depth=1.88' Max Vel=4.86 fps Inflow=79.98 cfs 13.817 af n=0.035 L=800.0' S=0.0117 '/' Capacity=90.60 cfs Outflow=79.33 cfs 13.817 af
Reach R1-2: R1-2	Avg. Flow Depth=0.26' Max Vel=3.27 fps Inflow=3.24 cfs 5.298 af n=0.035 L=20.0' S=0.0465 '/' Capacity=180.91 cfs Outflow=3.24 cfs 5.297 af
Reach R1-3: R-1-3	Avg. Flow Depth=3.09' Max Vel=2.36 fps Inflow=122.86 cfs 27.843 af n=0.040 L=225.0' S=0.0022 '/' Capacity=56.74 cfs Outflow=122.48 cfs 27.831 af
Reach R1-4: R1-4	Avg. Flow Depth=1.42' Max Vel=5.97 fps Inflow=62.01 cfs 8.030 af n=0.035 L=290.0' S=0.0241 '/' Capacity=130.35 cfs Outflow=61.83 cfs 8.030 af
Reach R1-5: R1-5	Inflow=122.48 cfs 27.831 af Outflow=122.48 cfs 27.831 af
Reach R2-1: R2-1	Avg. Flow Depth=1.88' Max Vel=4.08 fps Inflow=66.11 cfs 28.294 af n=0.035 L=460.0' S=0.0082 '/' Capacity=76.05 cfs Outflow=66.04 cfs 28.269 af
Reach R2-2: R2-2	Avg. Flow Depth=1.24' Max Vel=5.29 fps Inflow=29.42 cfs 3.660 af n=0.030 L=175.0' S=0.0171 '/' Capacity=82.75 cfs Outflow=29.37 cfs 3.660 af
Reach R2-3: R2-3	Avg. Flow Depth=2.04' Max Vel=5.57 fps Inflow=69.04 cfs 31.416 af n=0.030 L=410.0' S=0.0110 '/' Capacity=66.21 cfs Outflow=69.01 cfs 31.400 af
Reach R3-1: R3-1	Avg. Flow Depth=0.74' Max Vel=4.34 fps Inflow=11.56 cfs 2.066 af n=0.030 L=1,596.0' S=0.0203 '/' Capacity=89.94 cfs Outflow=11.09 cfs 2.066 af
Reach R3-P: R3-P	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.040 L=276.0' S=0.0056 '/' Capacity=3.77 cfs Outflow=0.00 cfs 0.000 af
Reach R4-1: R4-1	Avg. Flow Depth=0.62' Max Vel=7.02 fps Inflow=14.22 cfs 2.935 af n=0.030 L=95.0' S=0.0632 '/' Capacity=158.84 cfs Outflow=14.23 cfs 2.935 af
Reach R4-2: R4-2	Avg. Flow Depth=0.70' Max Vel=7.92 fps Inflow=18.79 cfs 3.564 af n=0.030 L=140.0' S=0.0714 '/' Capacity=168.92 cfs Outflow=18.79 cfs 3.564 af
Reach R5-1: R5-1	Avg. Flow Depth=0.88' Max Vel=4.53 fps Inflow=22.69 cfs 4.047 af n=0.035 L=640.0' S=0.0234 '/' Capacity=325.19 cfs Outflow=22.57 cfs 4.047 af
Reach R5-2: R5-2	Avg. Flow Depth=1.88' Max Vel=4.17 fps Inflow=72.34 cfs 10.177 af n=0.035 L=700.0' S=0.0086 '/' Capacity=77.67 cfs Outflow=68.05 cfs 10.177 af
Reach R6-1: R6-1	Avg. Flow Depth=0.10' Max Vel=1.42 fps Inflow=0.32 cfs 0.050 af n=0.030 L=360.0' S=0.0203 '/' Capacity=90.06 cfs Outflow=0.31 cfs 0.050 af
Pond 1-P:	Peak Elev=39.59' Storage=5.412 af Inflow=53.33 cfs 8.315 af 12.0" Round Culvert n=0.012 L=23.0' S=0.0000 '/' Outflow=3.24 cfs 5.298 af
Pond 2-P:	Peak Elev=38.27' Storage=12.399 af Inflow=123.65 cfs 32.738 af Outflow=66.11 cfs 28.294 af

Kittery_50%Build	(ID 2853680) ME-DennettRoad 24-hr S1 25-yr Rainfall=6.58"
Prepared by Barton	& Loguidice, DPC Printed 8/7/2023
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Pond 3-P:	Peak Elev=86.61' Storage=0.882 af Inflow=13.11 cfs 0.882 af Outflow=0.00 cfs 0.000 af
Pond CB-1-1: L1-1	Peak Elev=624.27' Inflow=79.98 cfs 13.817 af 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=79.98 cfs 13.817 af
Pond CB-1-3: L1-3	Peak Elev=48.26' Inflow=122.86 cfs 27.843 af 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=122.86 cfs 27.843 af
Pond CB-1-4: L1-4	Peak Elev=69.92' Inflow=62.01 cfs 8.030 af 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=62.01 cfs 8.030 af
Pond CB-2-3: L2-2	Peak Elev=35.61' Inflow=69.04 cfs 31.416 af 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=69.04 cfs 31.416 af
Pond CB-2-4: L2-3	Peak Elev=35.13' Inflow=29.42 cfs 3.660 af 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=29.42 cfs 3.660 af
Pond CB-3-1: L3-1	Peak Elev=89.17' Inflow=11.56 cfs 2.066 af Outflow=11.56 cfs 2.066 af
Pond CB-3-2: L3-2	Peak Elev=60.56' Inflow=35.65 cfs 6.145 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=35.65 cfs 6.145 af
Pond CB-4-1: L4-1	Peak Elev=55.87' Inflow=14.22 cfs 2.935 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=14.22 cfs 2.935 af
Pond CB-4-2: L4-2	Peak Elev=56.54' Inflow=18.79 cfs 3.564 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=18.79 cfs 3.564 af
Pond CB-5-1: L5-1	Peak Elev=57.93' Inflow=22.69 cfs 4.047 af 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=22.69 cfs 4.047 af
Pond CB-5-2: L5-2	Peak Elev=48.57' Inflow=72.34 cfs 10.177 af 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=72.34 cfs 10.177 af
Pond CB-6-1: L6-1	Peak Elev=49.50' Inflow=0.32 cfs 0.050 af 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.32 cfs 0.050 af
Link L1-5: L1-5	Inflow=122.48 cfs 27.831 af Primary=122.48 cfs 27.831 af
Link L1-6: L1-6	Inflow=180.92 cfs 35.862 af Primary=180.92 cfs 35.862 af
Link L2-1: L2-1	Inflow=66.11 cfs 28.294 af Primary=66.11 cfs 28.294 af
Link L2-4: L2-4	Inflow=69.01 cfs 31.400 af Primary=69.01 cfs 31.400 af

Kittery_50%Build (ID 2853680)	ME-DennettRoad	ME-DennettRoad 24-hr S1 25-yr Rainfall=6.58"	
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HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD	Software Solutions LLC	Page 27	
Link L2-5: L2-5		Inflow=29.37 cfs 3.660 af	
		Primary=29.37 cfs 3.660 at	
Link L2-6: L2-6		Inflow=72.26 cfs 35.060 af	
		Primary=72.26 cfs 35.060 af	
Link L4-3: L4-3		Inflow=14.23 cfs 2.935 af	
		Primary=14.23 cfs 2.935 af	
Link L4-4: L4-4		Inflow=18.79 cfs 3.564 af	
		Primary=18.79 cfs 3.564 af	
Link L4-5: L4-5		Inflow=32.94 cfs 6.499 af	
		Primary=32.94 cfs 6.499 af	
Link L5-3: L5-3		Inflow=68.05 cfs 10.177 af	
		Primary=68.05 cfs 10.177 af	
Link L6-2: L6-2		Inflow=0.31 cfs 0.050 af	
-		Primary=0.31 cfs 0.050 af	
Total Runoff Area = 323.908 ac	Runoff Volume = 102.189 af	Average Runoff Depth = 3.79"	

96.72% Pervious = 313.275 ac 3.28% Impervious = 10.633 ac

Kittery\_50%Build (ID 2853680)ME-DennettRoad 24-hr S1 50-yrRainfall=7.50"Prepared by Barton & Loguidice, DPCPrinted 8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 28

Subcatchment 1-1: DA-1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth>4.87" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=95.05 cfs 16.594 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth>5.27" Flow Length=485' Tc=27.5 min CN=WQ Runoff=62.77 cfs 9.890 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth>6.03" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=81.51 cfs 10.161 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth>6.07" Flow Length=875' Tc=18.9 min CN=WQ Runoff=71.38 cfs 9.347 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth>4.48" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=93.61 cfs 25.334 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth>3.31" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=55.76 cfs 14.494 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth>5.93" Flow Length=850' Tc=25.6 min CN=WQ Runoff=23.99 cfs 3.683 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth>5.96" Flow Length=745' Tc=17.7 min CN=WQ Runoff=33.96 cfs 4.275 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth>3.60" Flow Length=688' Tc=33.2 min CN=WQ Runoff=14.07 cfs 2.535 af
Subcatchment 3-1-P: DA-3-	I-PRunoff Area=1.790 ac100.00% ImperviousRunoff Depth>6.74"Tc=0.0 minCN=98Runoff=14.83 cfs1.006 af
Subcatchment 3-2: DA-3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth>2.41" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=36.12 cfs 5.155 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth>2.79" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=17.39 cfs 3.645 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth>5.08" Flow Length=955' Tc=37.6 min CN=WQ Runoff=22.15 cfs 4.236 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth>5.52" Flow Length=625' Tc=33.8 min CN=WQ Runoff=26.46 cfs 4.766 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth>6.44" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=72.65 cfs 7.063 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth>6.55" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.36 cfs 0.057 af

Kittery_50%Build	(ID 2853680) ME-DennettRoad 24-hr S1 50-yr Rainfall=7.50"
Prepared by Barton	& Loguidice, DPC Printed 8/7/2023
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Reach R1-1: R1-1	Avg. Flow Depth=2.04' Max Vel=5.08 fps Inflow=95.05 cfs 16.594 af n=0.035 L=800.0' S=0.0117 '/' Capacity=90.60 cfs Outflow=94.33 cfs 16.594 af
Reach R1-2: R1-2	Avg. Flow Depth=0.28' Max Vel=3.39 fps Inflow=3.65 cfs 6.293 af n=0.035 L=20.0' S=0.0465 '/' Capacity=180.91 cfs Outflow=3.65 cfs 6.292 af
Reach R1-3: R-1-3	Avg. Flow Depth=3.46' Max Vel=2.41 fps Inflow=144.78 cfs 33.048 af n=0.040 L=225.0' S=0.0022 '/' Capacity=56.74 cfs Outflow=144.34 cfs 33.034 af
Reach R1-4: R1-4	Avg. Flow Depth=1.52' Max Vel=6.20 fps Inflow=71.38 cfs 9.347 af n=0.035 L=290.0' S=0.0241 '/' Capacity=130.35 cfs Outflow=71.18 cfs 9.347 af
Reach R1-5: R1-5	Inflow=144.34 cfs 33.034 af Outflow=144.34 cfs 33.034 af
Reach R2-1: R2-1	Avg. Flow Depth=2.32' Max Vel=4.53 fps Inflow=103.38 cfs 35.316 af n=0.035 L=460.0' S=0.0082 '/' Capacity=76.05 cfs Outflow=103.25 cfs 35.290 af
Reach R2-2: R2-2	Avg. Flow Depth=1.33' Max Vel=5.49 fps Inflow=33.96 cfs 4.275 af n=0.030 L=175.0' S=0.0171 '/' Capacity=82.75 cfs Outflow=33.91 cfs 4.275 af
Reach R2-3: R2-3	Avg. Flow Depth=2.56' Max Vel=6.11 fps Inflow=107.33 cfs 38.972 af n=0.030 L=410.0' S=0.0110 '/' Capacity=66.21 cfs Outflow=107.27 cfs 38.956 af
Reach R3-1: R3-1	Avg. Flow Depth=0.81' Max Vel=4.58 fps Inflow=14.07 cfs 2.535 af n=0.030 L=1,596.0' S=0.0203 '/' Capacity=89.94 cfs Outflow=13.55 cfs 2.535 af
Reach R3-P: R3-P	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.040 L=276.0' S=0.0056 '/' Capacity=3.77 cfs Outflow=0.00 cfs 0.000 af
Reach R4-1: R4-1	Avg. Flow Depth=0.69' Max Vel=7.42 fps Inflow=17.39 cfs 3.645 af n=0.030 L=95.0' S=0.0632 '/' Capacity=158.84 cfs Outflow=17.38 cfs 3.645 af
Reach R4-2: R4-2	Avg. Flow Depth=0.76' Max Vel=8.29 fps Inflow=22.15 cfs 4.236 af n=0.030 L=140.0' S=0.0714 '/' Capacity=168.92 cfs Outflow=22.15 cfs 4.236 af
Reach R5-1: R5-1	Avg. Flow Depth=0.95' Max Vel=4.72 fps Inflow=26.46 cfs 4.766 af n=0.035 L=640.0' S=0.0234 '/' Capacity=325.19 cfs Outflow=26.33 cfs 4.766 af
Reach R5-2: R5-2	Avg. Flow Depth=2.01' Max Vel=4.32 fps Inflow=82.84 cfs 11.829 af n=0.035 L=700.0' S=0.0086 '/' Capacity=77.67 cfs Outflow=78.22 cfs 11.829 af
Reach R6-1: R6-1	Avg. Flow Depth=0.11' Max Vel=1.49 fps Inflow=0.36 cfs 0.057 af n=0.030 L=360.0' S=0.0203 '/' Capacity=90.06 cfs Outflow=0.36 cfs 0.057 af
Pond 1-P:	Peak Elev=39.91' Storage=6.493 af Inflow=62.77 cfs 9.890 af 12.0" Round Culvert n=0.012 L=23.0' S=0.0000 '/' Outflow=3.65 cfs 6.293 af
Pond 2-P:	Peak Elev=38.38' Storage=13.173 af Inflow=149.09 cfs 39.827 af Outflow=103.38 cfs 35.316 af

Kittery_50%Build Prepared by Barton	(ID 2853680) <i>ME-DennettRoad</i> 24-hr S1 50-yr <i>Rainfall</i> =7.50" & Loguidice. DPC Printed 8/7/2023
HydroCAD® 10.20-2g	s/n 05255 © 2022 HydroCAD Software Solutions LLC Page 30
Pond 3-P:	Peak Elev=86.69' Storage=1.006 af Inflow=14.83 cfs 1.006 af Outflow=0.00 cfs 0.000 af
Pond CB-1-1: L1-1	Peak Elev=861.96' Inflow=95.05 cfs 16.594 af 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=95.05 cfs 16.594 af
Pond CB-1-3: L1-3	Peak Elev=51.88' Inflow=144.78 cfs 33.048 af 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=144.78 cfs 33.048 af
Pond CB-1-4: L1-4	Peak Elev=81.27' Inflow=71.38 cfs 9.347 af 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=71.38 cfs 9.347 af
Pond CB-2-3: L2-2	Peak Elev=41.44' Inflow=107.33 cfs 38.972 af 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=107.33 cfs 38.972 af
Pond CB-2-4: L2-3	Peak Elev=36.36' Inflow=33.96 cfs 4.275 af 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=33.96 cfs 4.275 af
Pond CB-3-1: L3-1	Peak Elev=90.06' Inflow=14.07 cfs 2.535 af Outflow=14.07 cfs 2.535 af
Pond CB-3-2: L3-2	Peak Elev=63.63' Inflow=44.45 cfs 7.690 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=44.45 cfs 7.690 af
Pond CB-4-1: L4-1	Peak Elev=56.32' Inflow=17.39 cfs 3.645 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=17.39 cfs 3.645 af
Pond CB-4-2: L4-2	Peak Elev=57.14' Inflow=22.15 cfs 4.236 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=22.15 cfs 4.236 af
Pond CB-5-1: L5-1	Peak Elev=58.12' Inflow=26.46 cfs 4.766 af 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=26.46 cfs 4.766 af
Pond CB-5-2: L5-2	Peak Elev=50.82' Inflow=82.84 cfs 11.829 af 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=82.84 cfs 11.829 af
Pond CB-6-1: L6-1	Peak Elev=49.51' Inflow=0.36 cfs 0.057 af 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.36 cfs 0.057 af
Link L1-5: L1-5	Inflow=144.34 cfs 33.034 af Primary=144.34 cfs 33.034 af
Link L1-6: L1-6	Inflow=211.31 cfs 42.382 af Primary=211.31 cfs 42.382 af
Link L2-1: L2-1	Inflow=103.38 cfs 35.316 af Primary=103.38 cfs 35.316 af
Link L2-4: L2-4	Inflow=107.27 cfs 38.956 af Primary=107.27 cfs 38.956 af

Kittery_50%Build (ID 2853680)	ME-DennettRoad 2	ME-DennettRoad 24-hr S1 50-yr Rainfall=7.50"	
Prepared by Barton & Loguidice, DPC		Printed 8/7/2023	
HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCA	D Software Solutions LLC	Page 31	
Link L2-5: L2-5		Inflow=33.91 cfs 4.275 af	
		Primary=33.91 cfs 4.275 af	
Link L2-6: L2-6		Inflow=111.56 cfs 43.231 af	
		Primary=111.56 cfs 43.231 af	
Link L4-3: L4-3		Inflow=17.38 cfs 3.645 af	
		Primary=17.38 cfs 3.645 af	
l ink   4-4 ·   4-4		Inflow=22.15 cfs 4.236 af	
		Primary=22.15 cfs 4.236 af	
l ink   4-5.   4-5		Inflow=39.45 cfs. 7.881 af	
		Primary=39.45 cfs 7.881 af	
l ink   5-3   5-3		Inflow=78 22 cfs 11 829 af	
		Primary=78.22 cfs 11.829 af	
l ink   6-2   6-2		Inflow=0.36 cfs_0.057 af	
		Primary=0.36 cfs 0.057 af	
Total Runoff Area = 323.908 ac	Runoff Volume = 122.242 af	Average Runoff Depth = 4.53"	

 $96.72\% \text{ Pervious} = 313.275 \text{ ac} \qquad 3.28\% \text{ Impervious} = 10.633 \text{ ac}$ 

Kittery\_50%Build (ID 2853680)ME-DennettRoad 24-hr S1 100-yrRainfall=8.52"Prepared by Barton & Loguidice, DPCPrinted 8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 32

Subcatchment 1-1: DA-1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth>5.78" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=111.40 cfs 19.719 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth>6.21" Flow Length=485' Tc=27.5 min CN=WQ Runoff=72.97 cfs 11.653 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth>6.98" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=92.97 cfs 11.751 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth>7.02" Flow Length=875' Tc=18.9 min CN=WQ Runoff=81.38 cfs 10.807 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth>5.34" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=110.03 cfs 30.197 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth>4.05" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=67.76 cfs 17.750 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth>6.88" Flow Length=850' Tc=25.6 min CN=WQ Runoff=27.46 cfs 4.276 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth>6.91" Flow Length=745' Tc=17.7 min CN=WQ Runoff=38.81 cfs 4.956 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth>4.36" Flow Length=688' Tc=33.2 min CN=WQ Runoff=16.91 cfs 3.075 af
Subcatchment 3-1-P: DA-3-	I-P Runoff Area=1.790 ac 100.00% Impervious Runoff Depth>7.67"   Tc=0.0 min CN=98 Runoff=16.65 cfs 1.144 af
Subcatchment 3-2: DA-3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth>3.00" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=44.42 cfs 6.438 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth>3.43" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=21.01 cfs 4.482 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth>5.98" Flow Length=955' Tc=37.6 min CN=WQ Runoff=25.82 cfs 4.992 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth>6.45" Flow Length=625' Tc=33.8 min CN=WQ Runoff=30.53 cfs 5.570 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth>7.38" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=82.11 cfs 8.094 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth>7.49" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.41 cfs 0.066 af

Kittery_50%Build	(ID 2853680) ME-DennettRoad 24-hr S1	1 100-yr Rainfall=8.52"
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HydroCAD® 10.20-2g	s/n 05255 © 2022 HydroCAD Software Solutions LLC	Page 33
Reach R1-1: R1-1	Avg. Flow Depth=2.20' Max Vel=5.27 fps In n=0.035 L=800.0' S=0.0117 '/' Capacity=90.60 cfs Out	flow=111.40 cfs  19.719 af flow=110.57 cfs  19.719 af
Reach R1-2: R1-2	Avg. Flow Depth=0.53' Max Vel=4.84 fps n=0.035 L=20.0' S=0.0465 '/' Capacity=180.91 cfs C	Inflow=11.59 cfs 7.803 af Dutflow=11.86 cfs 7.803 af
Reach R1-3: R-1-3	Avg. Flow Depth=3.85' Max Vel=2.45 fps In n=0.040 L=225.0' S=0.0022 '/' Capacity=56.74 cfs Out	flow=168.55 cfs  39.273 af flow=168.02 cfs  39.259 af
Reach R1-4: R1-4	Avg. Flow Depth=1.61' Max Vel=6.41 fps I n=0.035 L=290.0' S=0.0241 '/' Capacity=130.35 cfs Ou	Inflow=81.38 cfs 10.807 af utflow=81.16 cfs 10.807 af
Reach R1-5: R1-5	In Out	flow=168.02 cfs 39.259 af flow=168.02 cfs 39.259 af
Reach R2-1: R2-1	Avg. Flow Depth=2.81' Max Vel=4.80 fps In n=0.035 L=460.0' S=0.0082 '/' Capacity=76.05 cfs Out	flow=144.50 cfs  43.378 af flow=144.32 cfs  43.353 af
Reach R2-2: R2-2	Avg. Flow Depth=1.41' Max Vel=5.68 fps n=0.030 L=175.0' S=0.0171 '/' Capacity=82.75 cfs C	Inflow=38.81 cfs 4.956 af Dutflow=38.75 cfs 4.956 af
Reach R2-3: R2-3	Avg. Flow Depth=3.14' Max Vel=6.42 fps In n=0.030 L=410.0' S=0.0110 '/' Capacity=66.21 cfs Out	flow=149.66 cfs 47.629 af flow=149.58 cfs 47.612 af
Reach R3-1: R3-1	Avg. Flow Depth=0.89' Max Vel=4.82 fps n=0.030 L=1,596.0' S=0.0203 '/' Capacity=89.94 cfs C	Inflow=16.91 cfs 3.075 af Dutflow=16.34 cfs 3.075 af
Reach R3-P: R3-P	Avg. Flow Depth=0.00' Max Vel=0.00 fps n=0.040 L=276.0' S=0.0056 '/' Capacity=3.77 cfs	Inflow=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af
Reach R4-1: R4-1	Avg. Flow Depth=0.76' Max Vel=7.81 fps n=0.030 L=95.0' S=0.0632 '/' Capacity=158.84 cfs C	Inflow=21.01 cfs 4.482 af Dutflow=21.00 cfs 4.482 af
Reach R4-2: R4-2	Avg. Flow Depth=0.82' Max Vel=8.64 fps n=0.030 L=140.0' S=0.0714 '/' Capacity=168.92 cfs C	Inflow=25.82 cfs 4.992 af Dutflow=25.81 cfs 4.992 af
Reach R5-1: R5-1	Avg. Flow Depth=1.02' Max Vel=4.90 fps n=0.035 L=640.0' S=0.0234 '/' Capacity=325.19 cfs C	Inflow=30.53 cfs 5.570 af Dutflow=30.40 cfs 5.570 af
Reach R5-2: R5-2	Avg. Flow Depth=2.13' Max Vel=4.46 fps I n=0.035 L=700.0' S=0.0086 '/' Capacity=77.67 cfs Ou	Inflow=94.05 cfs 13.664 af utflow=89.01 cfs 13.664 af
Reach R6-1: R6-1	Avg. Flow Depth=0.12' Max Vel=1.55 fps n=0.030 L=360.0' S=0.0203 '/' Capacity=90.06 cfs	Inflow=0.41 cfs 0.066 af Outflow=0.40 cfs 0.066 af
Pond 1-P:	Peak Elev=53.49' Storage=6.806 af 1 12.0" Round Culvert n=0.012 L=23.0' S=0.0000 '/' C	Inflow=72.97 cfs 11.653 af Dutflow=11.59 cfs 7.803 af
Pond 2-P:	Peak Elev=38.48' Storage=13.919 af In Out	flow=177.48 cfs  47.946 af flow=144.50 cfs  43.378 af

Kittery_50%Build	(ID 2853680) ME-DennettRoad 24-hr S1 100-yr Rainfall=8.52"
Prepared by Barton	& Loguidice, DPC Printed 8/7/2023
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Pond 3-P:	Peak Elev=86.79' Storage=1.144 af Inflow=16.65 cfs 1.144 af Outflow=0.00 cfs 0.000 af
Pond CB-1-1: L1-1	Peak Elev=1,165.95' Inflow=111.40 cfs 19.719 af 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=111.40 cfs 19.719 af
Pond CB-1-3: L1-3	Peak Elev=56.40' Inflow=168.55 cfs 39.273 af 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=168.55 cfs 39.273 af
Pond CB-1-4: L1-4	Peak Elev=95.14' Inflow=81.38 cfs 10.807 af 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=81.38 cfs 10.807 af
Pond CB-2-3: L2-2	Peak Elev=50.96' Inflow=149.66 cfs 47.629 af 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=149.66 cfs 47.629 af
Pond CB-2-4: L2-3	Peak Elev=37.98' Inflow=38.81 cfs 4.956 af 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=38.81 cfs 4.956 af
Pond CB-3-1: L3-1	Peak Elev=91.28' Inflow=16.91 cfs 3.075 af Outflow=16.91 cfs 3.075 af
Pond CB-3-2: L3-2	Peak Elev=68.03' Inflow=54.60 cfs 9.513 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=54.60 cfs 9.513 af
Pond CB-4-1: L4-1	Peak Elev=56.93' Inflow=21.01 cfs 4.482 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=21.01 cfs 4.482 af
Pond CB-4-2: L4-2	Peak Elev=57.91' Inflow=25.82 cfs 4.992 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=25.82 cfs 4.992 af
Pond CB-5-1: L5-1	Peak Elev=58.33' Inflow=30.53 cfs 5.570 af 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=30.53 cfs 5.570 af
Pond CB-5-2: L5-2	Peak Elev=53.56' Inflow=94.05 cfs 13.664 af 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=94.05 cfs 13.664 af
Pond CB-6-1: L6-1	Peak Elev=49.53' Inflow=0.41 cfs 0.066 af 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.41 cfs 0.066 af
Link L1-5: L1-5	Inflow=168.02 cfs 39.259 af Primary=168.02 cfs 39.259 af
Link L1-6: L1-6	Inflow=244.12 cfs 50.066 af Primary=244.12 cfs 50.066 af
Link L2-1: L2-1	Inflow=144.50 cfs 43.378 af Primary=144.50 cfs 43.378 af
Link L2-4: L2-4	Inflow=149.58 cfs 47.612 af Primary=149.58 cfs 47.612 af

Kittery_50%Build (ID 2853680)	ME-DennettRoad 24	1-hr S1 100-yr Rainfall=8.52"
Prepared by Barton & Loguidice, DPC		Printed 8/7/2023
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Link L2-5: L2-5		Inflow=38.75 cfs 4.956 af
		Primary=38.75 cfs 4.956 af
Link L2-6: L2-6		Inflow=154.99 cfs 52.568 af
		Primary=154.99 cfs 52.568 af
Link L4-3: L4-3		Inflow=21.00 cfs 4.482 af
		Primary=21.00 cfs 4.482 af
l ink   4-4 ·   4-4		Inflow=25.81 cfs. 4.992 af
		Primary=25.81 cfs 4.992 af
Link 1 4-5 · 1 4-5		Inflow-16 71 cfs 9 171 of
LIIIK L4-3. L4-3		Primary=46.71 cfs 9.474 af
Link   5 2,   5 2		Inflow-90.01 of a 12.664 of
LINK LO-3: LO-3		Primary-80.01 cfs 13.664 af
		Filliary=09.01 CIS 15.004 al
Link L6-2: L6-2		Inflow=0.40 cfs 0.066 af
		Primary=0.40 cfs 0.066 af
Total Runoff Area = 323.908 ac	Runoff Volume = 144.969 af	Average Runoff Depth = 5.37"

96.72% Pervious = 313.275 ac 3.28% Impervious = 10.633 ac

Kittery\_50%Build (ID 2853680)ME-DennettRoad 24-hr S1 10-yrExtreme: 1-yr Rainfall=3.02"Prepared by Barton & Loguidice, DPCPrinted 8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 36

Subcatchment 1-1: DA-1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth>1.16" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=22.69 cfs 3.966 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth>1.39" Flow Length=485' Tc=27.5 min CN=WQ Runoff=16.74 cfs 2.599 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth>1.95" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=27.54 cfs 3.284 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth>1.95" Flow Length=875' Tc=18.9 min CN=WQ Runoff=24.11 cfs 3.010 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth>1.07" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=22.65 cfs 6.067 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth=0.62" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=9.30 cfs 2.738 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth>1.80" Flow Length=850' Tc=25.6 min CN=WQ Runoff=7.66 cfs 1.119 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth>1.85" Flow Length=745' Tc=17.7 min CN=WQ Runoff=11.04 cfs 1.324 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth>0.75" Flow Length=688' Tc=33.2 min CN=WQ Runoff=2.93 cfs 0.527 af
Subcatchment 3-1-P: DA-3-	<b>1-P</b> Runoff Area=1.790 ac100.00% ImperviousRunoff Depth>2.66" Tc=0.0 minCN=98Runoff=6.09 cfs0.397 af
Subcatchment 3-2: DA-3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth>0.42" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=6.21 cfs 0.908 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth=0.56" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=3.31 cfs 0.731 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth>1.36" Flow Length=955' Tc=37.6 min CN=WQ Runoff=6.01 cfs 1.139 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth>1.60" Flow Length=625' Tc=33.8 min CN=WQ Runoff=7.92 cfs 1.382 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth>2.28" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=27.18 cfs 2.506 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth>2.36" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.14 cfs 0.021 af

Kittery_50%Build	(ID 2853680) ME-DennettRoad 24-hr S1 10-yr Extreme: 1-yr Rainfall=3.02"
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HydroCAD® 10.20-2g	s/n 05255 © 2022 HydroCAD Software Solutions LLC Page 37
Reach R1-1: R1-1	Avg. Flow Depth=1.04' Max Vel=3.50 fps Inflow=22.69 cfs 3.966 af n=0.035 L=800.0' S=0.0117 '/' Capacity=90.60 cfs Outflow=22.31 cfs 3.966 af
Reach R1-2: R1-2	Avg. Flow Depth=0.12' Max Vel=2.03 fps Inflow=0.79 cfs 1.237 af n=0.035 L=20.0' S=0.0465 '/' Capacity=180.91 cfs Outflow=0.79 cfs 1.237 af
Reach R1-3: R-1-3	Avg. Flow Depth=1.68' Max Vel=1.84 fps Inflow=39.48 cfs 8.488 af n=0.040 L=225.0' S=0.0022 '/' Capacity=56.74 cfs Outflow=39.25 cfs 8.483 af
Reach R1-4: R1-4	Avg. Flow Depth=0.90' Max Vel=4.65 fps Inflow=24.11 cfs 3.010 af n=0.035 L=290.0' S=0.0241 '/' Capacity=130.35 cfs Outflow=24.01 cfs 3.010 af
Reach R1-5: R1-5	Inflow=39.25 cfs 8.483 af Outflow=39.25 cfs 8.483 af
Reach R2-1: R2-1	Avg. Flow Depth=0.52' Max Vel=2.02 fps Inflow=4.82 cfs 7.391 af n=0.035 L=460.0' S=0.0082 '/' Capacity=76.05 cfs Outflow=4.82 cfs 7.379 af
Reach R2-2: R2-2	Avg. Flow Depth=0.77' Max Vel=4.08 fps Inflow=11.04 cfs 1.324 af n=0.030 L=175.0' S=0.0171 '/' Capacity=82.75 cfs Outflow=11.02 cfs 1.324 af
Reach R2-3: R2-3	Avg. Flow Depth=0.76' Max Vel=3.24 fps Inflow=8.64 cfs 8.498 af n=0.030 L=410.0' S=0.0110 '/' Capacity=66.21 cfs Outflow=8.59 cfs 8.490 af
Reach R3-1: R3-1	Avg. Flow Depth=0.35' Max Vel=2.89 fps Inflow=2.93 cfs 0.527 af n=0.030 L=1,596.0' S=0.0203 '/' Capacity=89.94 cfs Outflow=2.70 cfs 0.527 af
Reach R3-P: R3-P	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.040 L=276.0' S=0.0056 '/' Capacity=3.77 cfs Outflow=0.00 cfs 0.000 af
Reach R4-1: R4-1	Avg. Flow Depth=0.28' Max Vel=4.57 fps Inflow=3.31 cfs 0.731 af n=0.030 L=95.0' S=0.0632 '/' Capacity=158.84 cfs Outflow=3.31 cfs 0.731 af
Reach R4-2: R4-2	Avg. Flow Depth=0.38' Max Vel=5.72 fps Inflow=6.01 cfs 1.139 af n=0.030 L=140.0' S=0.0714 '/' Capacity=168.92 cfs Outflow=6.01 cfs 1.139 af
Reach R5-1: R5-1	Avg. Flow Depth=0.51' Max Vel=3.37 fps Inflow=7.92 cfs 1.382 af n=0.035 L=640.0' S=0.0234 '/' Capacity=325.19 cfs Outflow=7.84 cfs 1.382 af
Reach R5-2: R5-2	Avg. Flow Depth=1.23' Max Vel=3.29 fps Inflow=29.85 cfs 3.888 af n=0.035 L=700.0' S=0.0086 '/' Capacity=77.67 cfs Outflow=27.23 cfs 3.888 af
Reach R6-1: R6-1	Avg. Flow Depth=0.06' Max Vel=1.04 fps Inflow=0.14 cfs 0.021 af n=0.030 L=360.0' S=0.0203 '/' Capacity=90.06 cfs Outflow=0.13 cfs 0.021 af
Pond 1-P:	Peak Elev=38.57' Storage=1.930 af Inflow=16.74 cfs 2.599 af 12.0" Round Culvert n=0.012 L=23.0' S=0.0000 '/' Outflow=0.79 cfs 1.237 af
Pond 2-P:	Peak Elev=36.85' Storage=4.420 af Inflow=31.84 cfs 8.805 af Outflow=4.82 cfs 7.391 af

Kittery_50%Build (ID 28	<b>353680)</b> <i>ME-DennettRoad 24-hr S1 10-yr Extrem</i> Juidice, DPC	ne: 1-yr Rainfall=3	8. <i>0</i> 2"
Prepared by Barton & Log		Printed 8/7/2	2023
HydroCAD® 10.20-2g s/n 052	55 © 2022 HydroCAD Software Solutions LLC	Pag	<u>e 38</u>
Pond 3-P:	Peak Elev=86.27' Storage=0.397 af	Inflow=6.09 cfs 0.3 Dutflow=0.00 cfs 0.0	97 af 00 af
Pond CB-1-1: L1-1	Peak Elev=93.83'	Inflow=22.69 cfs 3.9	66 af
	14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Ou	utflow=22.69 cfs 3.9	66 af
Pond CB-1-3: L1-3	Peak Elev=40.31'	Inflow=39.48 cfs 8.4	88 af
	36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Ou	utflow=39.48 cfs 8.4	88 af
Pond CB-1-4: L1-4	Peak Elev=41.07'	Inflow=24.11 cfs 3.0	10 af
	24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Ou	utflow=24.11 cfs 3.0	10 af
Pond CB-2-3: L2-2	Peak Elev=31.12'	Inflow=8.64 cfs 8.4	98 af
	36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' C	Dutflow=8.64 cfs 8.4	98 af
Pond CB-2-4: L2-3	Peak Elev=32.16' ۱	Inflow=11.04 cfs 1.3	24 af
	24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Ou	utflow=11.04 cfs 1.3	24 af
Pond CB-3-1: L3-1	Peak Elev=87.36'	Inflow=2.93 cfs 0.5	27 af
	C	Dutflow=2.93 cfs 0.5	27 af
Pond CB-3-2: L3-2	// 24.0" Round Culvert n=0.012 L=100.0' S=0.0200	Inflow=7.59 cfs 1.4 Dutflow=7.59 cfs 1.4	35 af 35 af
Pond CB-4-1: L4-1	Peak Elev=54.77'	Inflow=3.31 cfs 0.7	31 af
	24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' C	Dutflow=3.31 cfs 0.7	31 af
Pond CB-4-2: L4-2	Peak Elev=55.07'	Inflow=6.01 cfs 1.1	39 af
	24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' C	Dutflow=6.01 cfs 1.1	39 af
Pond CB-5-1: L5-1	Peak Elev=57.07'	Inflow=7.92 cfs 1.3	82 af
	36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' C	Dutflow=7.92 cfs 1.3	82 af
Pond CB-5-2: L5-2	Peak Elev=42.55' I	Inflow=29.85 cfs 3.8	88 af
	32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Ou	utflow=29.85 cfs 3.8	88 af
Pond CB-6-1: L6-1	Peak Elev=49.41'	Inflow=0.14 cfs 0.0	21 af
	18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' C	Dutflow=0.14 cfs 0.0	21 af
Link L1-5: L1-5	l	Inflow=39.25 cfs 8.4	83 af
	Pr	imary=39.25 cfs 8.4	83 af
Link L1-6: L1-6	In	flow=62.71 cfs 11.4	93 af
	Prir	nary=62.71 cfs 11.4	93 af
Link L2-1: L2-1	F	Inflow=4.82 cfs 7.3 Primary=4.82 cfs 7.3	91 af 91 af
Link L2-4: L2-4	F	Inflow=8.59 cfs 8.4 Primary=8.59 cfs 8.4	90 af 90 af

Kittery_50%Build (ID 2853680) <i>ME-DennettRoad 24-hr S1 10-</i> Prepared by Barton & Loguidice, DPC	yr Extreme: 1-yr Rainfall=3.02" Printed 8/7/2023
HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLC	Page 39
Link L2-5: L2-5	Inflow=11.02 cfs 1.324 af
	Primary=11.02 cfs 1.324 at
Link L2-6: L2-6	Inflow=18.48 cfs 9.814 af
	Primary=18.48 cfs 9.814 af
Link L4-3: L4-3	Inflow=3.31 cfs 0.731 af
	Primary=3.31 cfs 0.731 af
Link L4-4: L4-4	Inflow=6.01 cfs 1.139 af
	Primary=6.01 cfs 1.139 af
Link L4-5: L4-5	Inflow=9.28 cfs 1.870 af
	Primary=9.28 cfs 1.870 af
Link L5-3: L5-3	Inflow=27.23 cfs 3.888 af
	Primary=27.23 cfs 3.888 af
Link L6-2: L6-2	Inflow=0.13 cfs 0.021 af
	Primary=0.13 cfs 0.021 af
Total Bunoff Aroa - 222 008 as Bunoff Volume - 21 710 a	of Average Buneff Denth - 1 18"

Total Runoff Area = 323.908 acRunoff Volume = 31.719 afAverage Runoff Depth = 1.18"96.72% Pervious = 313.275 ac3.28% Impervious = 10.633 ac

Kittery\_50%Build (ID 2853680)ME-DennettRoad 24-hr S1 10-yrExtreme: 10-yr Rainfall=5.32"Prepared by Barton & Loguidice, DPCPrinted 8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 40

Subcatchment 1-1: DA-1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth>2.97" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=59.93 cfs 10.120 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth>3.31" Flow Length=485' Tc=27.5 min CN=WQ Runoff=40.72 cfs 6.200 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth>4.02" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=56.41 cfs 6.777 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth>4.05" Flow Length=875' Tc=18.9 min CN=WQ Runoff=49.46 cfs 6.233 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth>2.72" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=58.98 cfs 15.387 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth>1.85" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=31.62 cfs 8.122 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth>3.89" Flow Length=850' Tc=25.6 min CN=WQ Runoff=16.40 cfs 2.419 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth>3.94" Flow Length=745' Tc=17.7 min CN=WQ Runoff=23.32 cfs 2.822 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth>2.07" Flow Length=688' Tc=33.2 min CN=WQ Runoff=8.38 cfs 1.461 af
Subcatchment 3-1-P: DA-3-1	-P Runoff Area=1.790 ac 100.00% Impervious Runoff Depth>4.77" Tc=0.0 min CN=98 Runoff=10.82 cfs 0.711 af
Subcatchment 3-2: DA-3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth>1.29" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=20.07 cfs 2.758 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth>1.57" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=10.15 cfs 2.047 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth>3.19" Flow Length=955' Tc=37.6 min CN=WQ Runoff=14.34 cfs 2.665 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth>3.56" Flow Length=625' Tc=33.8 min CN=WQ Runoff=17.67 cfs 3.075 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth>4.42" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=51.79 cfs 4.851 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth>4.52" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.26 cfs 0.040 af

Kittery_50%Build (ID 285	3680) ME-DennettRoad 24-hr S1 10-yr Extreme: 10-yr Rainfall=5.32"
Prepared by Barton & Logui	dice, DPC Printed 8/7/2023
TydioCAD® 10.20-29 3/110525	rage 41
Reach R1-1: R1-1	Avg. Flow Depth=1.65' Max Vel=4.52 fps Inflow=59.93 cfs 10.120 af n=0.035 L=800.0' S=0.0117 '/' Capacity=90.60 cfs Outflow=59.37 cfs 10.120 af
Reach R1-2: R1-2	Avg. Flow Depth=0.21' Max Vel=2.92 fps Inflow=2.28 cfs 3.784 af n=0.035 L=20.0' S=0.0465 '/' Capacity=180.91 cfs Outflow=2.28 cfs 3.784 af
Reach R1-3: R-1-3	Avg. Flow Depth=2.61' Max Vel=2.26 fps Inflow=93.81 cfs 20.681 af n=0.040 L=225.0' S=0.0022 '/' Capacity=56.74 cfs Outflow=93.49 cfs 20.672 af
Reach R1-4: R1-4	Avg. Flow Depth=1.28' Max Vel=5.63 fps Inflow=49.46 cfs 6.233 af n=0.035 L=290.0' S=0.0241 '/' Capacity=130.35 cfs Outflow=49.30 cfs 6.233 af
Reach R1-5: R1-5	Inflow=93.49 cfs 20.672 af Outflow=93.49 cfs 20.672 af
Reach R2-1: R2-1	Avg. Flow Depth=1.17' Max Vel=3.13 fps Inflow=24.03 cfs 19.259 af n=0.035 L=460.0' S=0.0082 '/' Capacity=76.05 cfs Outflow=24.02 cfs 19.234 af
Reach R2-2: R2-2	Avg. Flow Depth=1.11' Max Vel=4.97 fps Inflow=23.32 cfs 2.822 af n=0.030 L=175.0' S=0.0171 '/' Capacity=82.75 cfs Outflow=23.28 cfs 2.822 af
Reach R2-3: R2-3	Avg. Flow Depth=1.29' Max Vel=4.33 fps Inflow=25.63 cfs 21.653 af n=0.030 L=410.0' S=0.0110 '/' Capacity=66.21 cfs Outflow=25.63 cfs 21.637 af
Reach R3-1: R3-1	Avg. Flow Depth=0.62' Max Vel=3.96 fps Inflow=8.38 cfs 1.461 af n=0.030 L=1,596.0' S=0.0203 '/' Capacity=89.94 cfs Outflow=7.98 cfs 1.461 af
Reach R3-P: R3-P	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.040 L=276.0' S=0.0056 '/' Capacity=3.77 cfs Outflow=0.00 cfs 0.000 af
Reach R4-1: R4-1	Avg. Flow Depth=0.52' Max Vel=6.38 fps Inflow=10.15 cfs 2.047 af n=0.030 L=95.0' S=0.0632 '/' Capacity=158.84 cfs Outflow=10.15 cfs 2.047 af
Reach R4-2: R4-2	Avg. Flow Depth=0.61' Max Vel=7.35 fps Inflow=14.34 cfs 2.665 af n=0.030 L=140.0' S=0.0714 '/' Capacity=168.92 cfs Outflow=14.34 cfs 2.665 af
Reach R5-1: R5-1	Avg. Flow Depth=0.78' Max Vel=4.23 fps Inflow=17.67 cfs 3.075 af n=0.035 L=640.0' S=0.0234 '/' Capacity=325.19 cfs Outflow=17.57 cfs 3.075 af
Reach R5-2: R5-2	Avg. Flow Depth=1.70' Max Vel=3.94 fps Inflow=58.29 cfs 7.926 af n=0.035 L=700.0' S=0.0086 '/' Capacity=77.67 cfs Outflow=54.42 cfs 7.926 af
Reach R6-1: R6-1	Avg. Flow Depth=0.09' Max Vel=1.32 fps Inflow=0.26 cfs 0.040 af n=0.030 L=360.0' S=0.0203 '/' Capacity=90.06 cfs Outflow=0.25 cfs 0.040 af
Pond 1-P:	Peak Elev=39.21' Storage=4.113 af Inflow=40.72 cfs 6.200 af 12.0" Round Culvert n=0.012 L=23.0' S=0.0000 '/' Outflow=2.28 cfs 3.784 af
Pond 2-P:	Peak Elev=38.11' Storage=11.246 af Inflow=90.38 cfs 23.509 af Outflow=24.03 cfs 19.259 af

<b>Kittery_50%Build (ID 28</b> Prepared by Barton & Log	<b>53680)</b> <i>ME-DennettRoad 24-hr S1 10-yr Extreme: 10-yr Rainfall=5.32"</i> uidice, DPC Printed 8/7/2023	
Pond 3-P:	Peak Elev=86.49' Storage=0.711 af Inflow=10.82 cfs 0.711 af Outflow=0.00 cfs 0.000 af	
Pond CB-1-1: L1-1	Peak Elev=371.42' Inflow=59.93 cfs 10.120 af 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=59.93 cfs 10.120 af	
Pond CB-1-3: L1-3	Peak Elev=44.69' Inflow=93.81 cfs 20.681 af 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=93.81 cfs 20.681 af	
Pond CB-1-4: L1-4	Peak Elev=57.21' Inflow=49.46 cfs 6.233 af 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=49.46 cfs 6.233 af	
Pond CB-2-3: L2-2	Peak Elev=32.08' Inflow=25.63 cfs 21.653 af 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=25.63 cfs 21.653 af	
Pond CB-2-4: L2-3	Peak Elev=33.96' Inflow=23.32 cfs 2.822 af 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=23.32 cfs 2.822 af	
Pond CB-3-1: L3-1	Peak Elev=88.29' Inflow=8.38 cfs 1.461 af Outflow=8.38 cfs 1.461 af	
Pond CB-3-2: L3-2	Peak Elev=57.68' Inflow=24.79 cfs 4.219 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=24.79 cfs 4.219 af	
Pond CB-4-1: L4-1	Peak Elev=55.46' Inflow=10.15 cfs 2.047 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=10.15 cfs 2.047 af	
Pond CB-4-2: L4-2	Peak Elev=55.88' Inflow=14.34 cfs 2.665 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=14.34 cfs 2.665 af	
Pond CB-5-1: L5-1	Peak Elev=57.66' Inflow=17.67 cfs 3.075 af 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=17.67 cfs 3.075 af	
Pond CB-5-2: L5-2	Peak Elev=46.03' Inflow=58.29 cfs 7.926 af 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=58.29 cfs 7.926 af	
Pond CB-6-1: L6-1	Peak Elev=49.47' Inflow=0.26 cfs 0.040 af 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.26 cfs 0.040 af	
Link L1-5: L1-5	Inflow=93.49 cfs 20.672 af Primary=93.49 cfs 20.672 af	
Link L1-6: L1-6	Inflow=140.50 cfs 26.905 af Primary=140.50 cfs 26.905 af	
Link L2-1: L2-1	Inflow=24.03 cfs 19.259 af Primary=24.03 cfs 19.259 af	
Link L2-4: L2-4	Inflow=25.63 cfs 21.637 af Primary=25.63 cfs 21.637 af	

Kittery_50%Build (ID 2853680) ME-De	ennettRoad 24-hr S1 10-yr Extreme: 10-yr Rainfall=5.32"
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Link L2-5: L2-5	Inflow=23.28 cfs 2.822 af
	Primary=23.28 cfs 2.822 af
Link L2-6: L2-6	Inflow=39.97 cfs 24.459 af
	Primary=39.97 cfs 24.459 af
Link L4-3: L4-3	Inflow=10.15 cfs 2.047 af
	Primary=10.15 cfs 2.047 af
Link L4-4: L4-4	Inflow=14.34 cfs 2.665 af
	Primary=14.34 cfs 2.665 af
Link L4-5: L4-5	Inflow=24.43 cfs 4.712 af
	Primary=24.43 cfs 4.712 af
Link L5-3: L5-3	Inflow=54.42 cfs 7.926 af
	Primary=54.42 cfs 7.926 af
Link L6-2: L6-2	Inflow=0.25 cfs 0.040 af
	Primary=0.25 cfs 0.040 af
Total Bunaff Area - 222 009 aa	Buneff Valume 75 600 of Average Buneff Denth 2 90"

Total Runoff Area = 323.908 acRunoff Volume = 75.690 afAverage Runoff Depth = 2.80"96.72% Pervious = 313.275 ac3.28% Impervious = 10.633 ac

Appendix I

100% Build-Out HydroCAD Summary Report



# **Project Notes**

Copied 10 events from ME-DennettRoad 24-hr S1 storm

Event#	Event	Storm Type	Curve	Mode	Duration	B/B	Depth	AMC
	Name				(hours)		(inches)	
1	1-yr	ME-DennettRoad 24-hr S1	1-yr	Default	24.00	1	2.63	2
2	2-yr	ME-DennettRoad 24-hr S1	2-yr	Default	24.00	1	3.31	2
3	5-yr	ME-DennettRoad 24-hr S1	5-yr	Default	24.00	1	4.41	2
4	10-yr	ME-DennettRoad 24-hr S1	10-yr	Default	24.00	1	5.32	2
5	25-yr	ME-DennettRoad 24-hr S1	25-yr	Default	24.00	1	6.58	2
6	50-yr	ME-DennettRoad 24-hr S1	50-yr	Default	24.00	1	7.50	2
7	100-yr	ME-DennettRoad 24-hr S1	100-yr	Default	24.00	1	8.52	2
8	Extreme: 1-yr	ME-DennettRoad 24-hr S1	1-yr	Default	24.00	1	2.63	2
9	Extreme: 10-yr	ME-DennettRoad 24-hr S1	10-yr	Default	24.00	1	5.32	2

## **Rainfall Events Listing (selected events)**

### Area Listing (all nodes)

Area	CN	Description		
(acres)		(subcatchment-numbers)		
0.423	95	*BARREN, D (89, 92, 95) (1-2)		
0.723	89	*COMMERCIAL, A (89) (2-2)		
9.929	95	*COMMERCIAL, D (95) (1-2, 1-3)		
6.495	95	*RESIDENTIAL, D (85, 90, 95) (1-2, 1-3)		
33.767	95	*WOODS, A (30, 60, 95) (2-1, 2-2, 4-1)		
4.359	95	*WOODS, D (30, 86, 95) (2-1)		
16.369	95	*WOODS, D (77, 86, 95) (1-3, 2-2, 4-1)		
1.065	77	*WOODS, D (77, 86,95) (1-2)		
8.453	54	1/2 acre lots, 25% imp, HSG A (2-1, 2-2, 3-1, 3-2, 4-1)		
0.193	80	1/2 acre lots, 25% imp, HSG C (1-1)		
12.899	85	1/2 acre lots, 25% imp, HSG D (1-1, 1-2, 2-1, 3-1, 3-2)		
1.776	77	1/8 acre lots, 65% imp, HSG A (1-1, 3-1, 3-2, 4-1)		
0.089	90	1/8 acre lots, 65% imp, HSG C (1-1)		
3.453	92	1/8 acre lots, 65% imp, HSG D (1-1, 1-2, 2-1, 3-1, 3-2)		
7.040	39	>75% Grass cover, Good, HSG A (2-1, 2-2, 3-1, 3-2, 4-1)		
0.916	74	>75% Grass cover, Good, HSG C (1-1)		
16.248	80	>75% Grass cover, Good, HSG D (1-1, 1-2, 2-1, 2-2, 3-1, 3-2)		
0.006	65	Brush, Good, HSG C (1-1)		
0.291	89	COMMERCIAL, A (89) (2-4, 5-1)		
23.114	95	COMMERCIAL, D (95) (1-4, 2-3, 2-4, 4-2, 5-1, 5-2, 6-1)		
3.708	95	RESIDENTIAL, D (85, 90, 95) (1-4)		
4.367	95	WOODS, A (30, 60, 95) (2-4, 4-2, 5-1)		
3.785	85	WOODS, D (77, 81, 85) (1-4)		
32.923	95	WOODS, D (77, 86, 95) (1-4, 2-3, 2-4, 4-2, 5-1, 5-2, 6-1)		
1.790	98	Water Surface, HSG A (3-1-P)		
28.687	30	Woods, Good, HSG A (1-1, 2-1, 2-2, 3-1, 3-2, 4-1)		
2.174	70	Woods, Good, HSG C (1-1)		
98.866	77	Woods, Good, HSG D (1-1, 1-2, 1-3, 2-1, 2-2, 3-1, 3-2, 4-1)		
323.908	80	TOTAL AREA		

# Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
47.746	HSG A	1-1, 2-1, 2-2, 3-1, 3-1-P, 3-2, 4-1
0.000	HSG B	
3.378	HSG C	1-1
131.466	HSG D	1-1, 1-2, 1-3, 2-1, 2-2, 3-1, 3-2, 4-1
141.318	Other	1-2, 1-3, 1-4, 2-1, 2-2, 2-3, 2-4, 4-1, 4-2, 5-1, 5-2, 6-1
323.908		TOTAL AREA

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Ground Covers (all nodes)

HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
(acres)	(acres)	(acres)	(acres)	(acres)	(acres)	Cover	Numbers
0.000	0.000	0.000	0.000	0.423	0.423	*BARREN, D (89, 92, 95)	1-2
0.000	0.000	0.000	0.000	0.723	0.723	*COMMERCIAL, A (89)	2-2
0.000	0.000	0.000	0.000	9.929	9.929	*COMMERCIAL, D (95)	1-2, 1-3
0.000	0.000	0.000	0.000	6.495	6.495	*RESIDENTIAL, D (85, 90, 95)	1-2, 1-3
0.000	0.000	0.000	0.000	33.767	33.767	*WOODS, A (30, 60, 95)	2-1, 2-2,
							4-1
0.000	0.000	0.000	0.000	4.359	4.359	*WOODS, D (30, 86, 95)	2-1
0.000	0.000	0.000	0.000	16.369	16.369	*WOODS, D (77, 86, 95)	1-3, 2-2,
							4-1
0.000	0.000	0.000	0.000	1.065	1.065	*WOODS, D (77, 86,95)	1-2
8.453	0.000	0.193	12.899	0.000	21.545	1/2 acre lots, 25% imp	1-1, 1-2,
							2-1, 2-2,
							3-1, 3-2,
							4-1
1.776	0.000	0.089	3.453	0.000	5.318	1/8 acre lots, 65% imp	1-1, 1-2,
							2-1, 3-1,
							3-2, 4-1
7.040	0.000	0.916	16.248	0.000	24.204	>75% Grass cover, Good	1-1, 1-2,
							2-1, 2-2,
							3-1, 3-2,
							4-1
0.000	0.000	0.006	0.000	0.000	0.006	Brush, Good	1-1
0.000	0.000	0.000	0.000	0.291	0.291	COMMERCIAL, A (89)	2-4, 5-1
0.000	0.000	0.000	0.000	23.114	23.114	COMMERCIAL, D (95)	1-4, 2-3,
							2-4, 4-2,
							5-1, 5-2,
							6-1
0.000	0.000	0.000	0.000	3.708	3.708	RESIDENTIAL, D (85, 90, 95)	1-4
0.000	0.000	0.000	0.000	4.367	4.367	WOODS, A (30, 60, 95)	2-4, 4-2,
							5-1
0.000	0.000	0.000	0.000	3.785	3.785	WOODS, D (77, 81, 85)	1-4
0.000	0.000	0.000	0.000	32.923	32.923	WOODS, D (77, 86, 95)	1-4, 2-3,
							2-4, 4-2,
							5-1, 5-2,
							6-1
1.790	0.000	0.000	0.000	0.000	1.790	Water Surface	3-1-P
28.687	0.000	2.174	98.866	0.000	129.727	Woods, Good	1-1, 1-2,
							1-3, 2-1,
							2-2, 3-1,
	_						3-2, 4-1
47.746	0.000	3.378	131.466	141.318	323.908	TOTAL AREA	

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Line	# Node	In-Invert	Out-Invert	Length	Slope	n	Width	Diam/Height	Inside-Fill
	Number	(feet)	(feet)	(feet)	(ft/ft)		(inches)	(inches)	(inches)
	1 1-4	0.00	0.00	225.0	0.0180	0.025	0.0	12.0	0.0
	2 3-1	0.00	0.00	10.0	0.0500	0.013	0.0	6.0	0.0
:	3 3-2	0.00	0.00	68.0	0.0290	0.025	0.0	12.0	0.0
	4 1-P	37.91	37.91	23.0	0.0000	0.012	0.0	12.0	0.0
:	5 2-P	35.58	32.78	50.0	0.0560	0.025	0.0	18.0	0.0
	6 CB-1-1	47.05	46.31	50.0	0.0148	0.025	0.0	14.0	0.0
	7 CB-1-3	36.98	36.80	95.0	0.0019	0.012	0.0	36.0	0.0
	8 CB-1-4	36.00	33.00	84.0	0.0357	0.025	0.0	24.0	0.0
9	9 CB-2-3	30.00	28.50	150.0	0.0100	0.012	0.0	36.0	0.0
1	0 CB-2-4	30.00	30.00	75.0	0.0000	0.012	0.0	24.0	0.0
1	1 CB-3-1	86.11	86.46	20.0	-0.0175	0.012	0.0	6.0	0.0
1:	2 CB-3-1	86.67	86.32	20.0	0.0175	0.012	0.0	12.0	0.0
1	3 CB-3-1	86.64	86.51	20.0	0.0065	0.012	0.0	6.0	0.0
1	4 CB-3-1	86.97	86.75	20.0	0.0110	0.012	0.0	14.0	0.0
1	5 CB-3-2	54.00	52.00	100.0	0.0200	0.012	0.0	24.0	0.0
1	6 CB-4-1	54.00	52.00	100.0	0.0200	0.012	0.0	24.0	0.0
1	7 CB-4-2	54.00	52.00	100.0	0.0200	0.012	0.0	24.0	0.0
1	8 CB-5-1	56.00	55.00	70.0	0.0143	0.012	0.0	36.0	0.0
1	9 CB-5-2	40.00	34.00	217.0	0.0276	0.012	0.0	32.0	0.0
2	0 CB-6-1	49.25	46.31	95.0	0.0309	0.012	0.0	18.0	0.0

### Pipe Listing (all nodes)

Kittery\_100%Build (ID 2853681)ME-DennettRoad 24-hr S1 1-yrRainfall=2.63"Prepared by Barton & Loguidice, DPCPrinted 8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 8

Subcatchment 1-1: 1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth=0.90" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=18.87 cfs 3.059 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth=1.13" Flow Length=485' Tc=27.5 min CN=WQ Runoff=14.70 cfs 2.120 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth=1.85" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=28.51 cfs 3.120 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth=1.92" Flow Length=875' Tc=18.9 min CN=WQ Runoff=25.95 cfs 2.963 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth=0.88" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=19.89 cfs 4.964 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth=1.61" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=30.88 cfs 7.075 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth=2.09" Flow Length=850' Tc=25.6 min CN=WQ Runoff=9.58 cfs 1.298 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth=2.08" Flow Length=745' Tc=17.7 min CN=WQ Runoff=13.49 cfs 1.493 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth=0.57" Flow Length=688' Tc=33.2 min CN=WQ Runoff=2.40 cfs 0.399 af
Subcatchment 3-1-P: DA-3-1	-P Runoff Area=1.790 ac 100.00% Impervious Runoff Depth=2.40" Tc=0.0 min CN=98 Runoff=5.87 cfs 0.358 af
Subcatchment 3-2: 3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth=0.31" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=5.12 cfs 0.673 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth=1.18" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=8.63 cfs 1.547 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth=2.09" Flow Length=955' Tc=37.6 min CN=WQ Runoff=10.32 cfs 1.744 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth=2.08" Flow Length=625' Tc=33.8 min CN=WQ Runoff=11.31 cfs 1.797 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth=2.09" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=27.30 cfs 2.291 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth=2.09" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.13 cfs 0.018 af

Kittery_100%Build (ID	<b>2853681)</b> <i>ME-DennettRoad</i> 24-hr S1 1-yr <i>Rainfall</i> =2.63"
Prepared by Barton & Lo	Juidice, DPC Printed 8/7/2023
HydroCAD® 10.20-2g s/n 05	25 © 2022 HydroCAD Software Solutions LLC Page 9
Reach R1-1: R1-1	Avg. Flow Depth=0.95' Max Vel=3.32 fps Inflow=18.87 cfs 3.059 af n=0.035 L=800.0' S=0.0117 '/' Capacity=90.60 cfs Outflow=18.51 cfs 3.059 af
Reach R1-2: R1-2	Avg. Flow Depth=0.10' Max Vel=1.83 fps Inflow=0.59 cfs 0.941 af n=0.035 L=20.0' S=0.0465 '/' Capacity=180.91 cfs Outflow=0.59 cfs 0.941 af
Reach R1-3: R-1-3	Avg. Flow Depth=1.63' Max Vel=1.81 fps Inflow=37.33 cfs 7.120 af n=0.040 L=225.0' S=0.0022 '/' Capacity=56.74 cfs Outflow=36.99 cfs 7.115 af
Reach R1-4: R1-4	Avg. Flow Depth=0.94' Max Vel=4.74 fps Inflow=25.95 cfs 2.963 af n=0.035 L=290.0' S=0.0241 '/' Capacity=130.35 cfs Outflow=25.84 cfs 2.963 af
Reach R1-5: R1-5	Inflow=36.99 cfs 7.115 af Outflow=36.99 cfs 7.115 af
Reach R2-1: R2-1	Avg. Flow Depth=0.61' Max Vel=2.19 fps Inflow=6.43 cfs 10.108 af n=0.035 L=460.0' S=0.0082 '/' Capacity=76.05 cfs Outflow=6.43 cfs 10.093 af
Reach R2-2: R2-2	Avg. Flow Depth=0.85' Max Vel=4.30 fps Inflow=13.49 cfs 1.493 af n=0.030 L=175.0' S=0.0171 '/' Capacity=82.75 cfs Outflow=13.46 cfs 1.493 af
Reach R2-3: R2-3	Avg. Flow Depth=0.86' Max Vel=3.47 fps Inflow=11.19 cfs 11.390 af n=0.030 L=410.0' S=0.0110 '/' Capacity=66.21 cfs Outflow=11.14 cfs 11.381 af
Reach R3-1: R3-1	Avg. Flow Depth=0.31' Max Vel=2.71 fps Inflow=2.40 cfs 0.399 af n=0.030 L=1,596.0' S=0.0203 '/' Capacity=89.94 cfs Outflow=2.18 cfs 0.399 af
Reach R3-P: R3-P	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.040 L=276.0' S=0.0056 '/' Capacity=3.77 cfs Outflow=0.00 cfs 0.000 af
Reach R4-1: R4-1	Avg. Flow Depth=0.48' Max Vel=6.09 fps Inflow=8.63 cfs 1.547 af n=0.030 L=95.0' S=0.0632 '/' Capacity=158.84 cfs Outflow=8.63 cfs 1.547 af
Reach R4-2: R4-2	Avg. Flow Depth=0.51' Max Vel=6.70 fps Inflow=10.32 cfs 1.744 af n=0.030 L=140.0' S=0.0714 '/' Capacity=168.92 cfs Outflow=10.32 cfs 1.744 af
Reach R5-1: R5-1	Avg. Flow Depth=0.62' Max Vel=3.73 fps Inflow=11.31 cfs 1.797 af n=0.035 L=640.0' S=0.0234 '/' Capacity=325.19 cfs Outflow=11.22 cfs 1.797 af
Reach R5-2: R5-2	Avg. Flow Depth=1.27' Max Vel=3.34 fps Inflow=31.46 cfs 4.088 af n=0.035 L=700.0' S=0.0086 '/' Capacity=77.67 cfs Outflow=28.91 cfs 4.088 af
Reach R6-1: R6-1	Avg. Flow Depth=0.06' Max Vel=1.03 fps Inflow=0.13 cfs 0.018 af n=0.030 L=360.0' S=0.0203 '/' Capacity=90.06 cfs Outflow=0.13 cfs 0.018 af
Pond 1-P:	Peak Elev=38.47' Storage=1.615 af Inflow=14.70 cfs 2.120 af 12.0" Round Culvert n=0.012 L=23.0' S=0.0000 '/' Outflow=0.59 cfs 0.941 af
Pond 2-P:	Peak Elev=37.25' Storage=6.505 af Inflow=50.35 cfs 12.039 af Outflow=6.43 cfs 10.108 af

<b>Kittery_100%Build (IE</b> Prepared by Barton & L	<b>2853681)</b> <i>ME-DennettRoad</i> 24-hr S1 1-yr Rainfall=2.63" guidice. DPC Printed 8/7/2023
HydroCAD® 10.20-2g s/n 0	255 © 2022 HydroCAD Software Solutions LLC Page 10
Pond 3-P:	Peak Elev=86.25' Storage=0.358 af Inflow=5.87 cfs 0.358 af Outflow=0.00 cfs 0.000 af
Pond CB-1-1: L1-1	Peak Elev=79.53' Inflow=18.87 cfs 3.059 af 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=18.87 cfs 3.059 af
Pond CB-1-3: L1-3	Peak Elev=40.17' Inflow=37.33 cfs 7.120 af 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=37.33 cfs 7.120 af
Pond CB-1-4: L1-4	Peak Elev=41.72' Inflow=25.95 cfs 2.963 af 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=25.95 cfs 2.963 af
Pond CB-2-3: L2-2	Peak Elev=31.29' Inflow=11.19 cfs 11.390 af 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=11.19 cfs 11.390 af
Pond CB-2-4: L2-3	Peak Elev=32.56' Inflow=13.49 cfs 1.493 af 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=13.49 cfs 1.493 af
Pond CB-3-1: L3-1	Peak Elev=87.28' Inflow=2.40 cfs 0.399 af Outflow=2.40 cfs 0.399 af
Pond CB-3-2: L3-2	Peak Elev=55.08' Inflow=6.14 cfs 1.073 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=6.14 cfs 1.073 af
Pond CB-4-1: L4-1	Peak Elev=55.32' Inflow=8.63 cfs 1.547 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=8.63 cfs 1.547 af
Pond CB-4-2: L4-2	Peak Elev=55.48' Inflow=10.32 cfs 1.744 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=10.32 cfs 1.744 af
Pond CB-5-1: L5-1	Peak Elev=57.29' Inflow=11.31 cfs 1.797 af 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=11.31 cfs 1.797 af
Pond CB-5-2: L5-2	Peak Elev=42.70' Inflow=31.46 cfs 4.088 af 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=31.46 cfs 4.088 af
Pond CB-6-1: L6-1	Peak Elev=49.41' Inflow=0.13 cfs 0.018 af 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.13 cfs 0.018 af
Link L1-5: L1-5	Inflow=36.99 cfs 7.115 af Primary=36.99 cfs 7.115 af
Link L1-6: L1-6	Inflow=62.52 cfs 10.079 af Primary=62.52 cfs 10.079 af
Link L2-1: L2-1	Inflow=6.43 cfs 10.108 af Primary=6.43 cfs 10.108 af
Link L2-4: L2-4	Inflow=11.14 cfs 11.381 af Primary=11.14 cfs 11.381 af

Kittery_100%Build (ID 2853681)	ME-DennettRoad 24-hr S1 1-yr Rainfall=2.63"
Prepared by Barton & Loguidice, DPC	Printed 8/7/2023
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link   25,   25	Inflow-13.46 cfs. 1.403 of
LIIK L2-3. L2-3	Primary-13.46 cfs 1.493 af
	1 hindry=10.40 cl3 1.400 dl
Link L2-6: L2-6	Inflow=23.22 cfs 12.874 af
	Primary=23.22 cfs 12.874 af
Link L4-3: L4-3	Inflow=8.63 cfs 1.547 af
	Primary=8.63 cfs 1.547 af
Link L4-4: L4-4	Inflow=10.32 cfs 1.744 af
	Primary=10.32 cfs 1.744 af
Link L4-5: L4-5	Inflow=18.91 cfs 3.291 af
	Primary=18.91 cfs 3.291 at
	Inflow 20.01 efe. 4.000 ef
LINK L5-3: L5-3	Innow=28.91 cls 4.088 al Drimon - 28.01 cfs 4.088 al
	Fillinary=20.91 CIS 4.000 al
link   6-2:   6-2	Inflow-0.13 cfs. 0.018 af
	Primary=0.13 cfs 0.018 af
Total Runoff Area = 323.908 ac Runoff V	olume = 34.920 af Average Runoff Depth = 1.29"
96.72% Pervi	ous = 313.275 ac 3.28% Impervious = 10.633 ac

Kittery\_100%Build (ID 2853681)ME-DennettRoad 24-hr S1 2-yrRainfall=3.31"Prepared by Barton & Loguidice, DPCPrinted 8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 12

Subcatchment 1-1: 1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth=1.37" Flow Length=1.330' Tc=33.2 min_CN=WO_Runoff=28.60 cfs 4.677 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth=1.65" Flow Length=485' Tc=27.5 min CN=WQ Runoff=20.97 cfs 3.094 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth=2.48" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=36.20 cfs 4.176 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth=2.57" Flow Length=875' Tc=18.9 min CN=WQ Runoff=32.80 cfs 3.955 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth=1.32" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=29.34 cfs 7.442 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth=2.15" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=39.13 cfs 9.433 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth=2.75" Flow Length=850' Tc=25.6 min CN=WQ Runoff=11.91 cfs 1.710 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth=2.75" Flow Length=745' Tc=17.7 min CN=WQ Runoff=16.78 cfs 1.969 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth=0.89" Flow Length=688' Tc=33.2 min CN=WQ Runoff=3.74 cfs 0.629 af
Subcatchment 3-1-P: DA-3-	<b>1-P</b> Runoff Area=1.790 ac100.00% ImperviousRunoff Depth=3.08"Tc=0.0 minCN=98Runoff=7.12 cfs0.459 af
Subcatchment 3-2: 3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth=0.51" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=7.99 cfs 1.099 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth=1.58" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=10.91 cfs 2.068 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth=2.75" Flow Length=955' Tc=37.6 min CN=WQ Runoff=12.84 cfs 2.298 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth=2.74" Flow Length=625' Tc=33.8 min CN=WQ Runoff=14.09 cfs 2.370 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth=2.75" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=33.94 cfs 3.019 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth=2.75" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.17 cfs 0.024 af

Kittery_100%Build (ID 2	2853681) ME-DennettRoad 24-hr S1 2-yr Rainfall=3.31"	
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Reach R1-1: R1-1	Avg. Flow Depth=1.17' Max Vel=3.72 fps Inflow=28.60 cfs 4.677 af n=0.035 L=800.0' S=0.0117 '/' Capacity=90.60 cfs Outflow=28.18 cfs 4.677 af	
Reach R1-2: R1-2	Avg. Flow Depth=0.13' Max Vel=2.20 fps Inflow=0.99 cfs 1.579 af n=0.035 L=20.0' S=0.0465 '/' Capacity=180.91 cfs Outflow=0.99 cfs 1.579 af	
Reach R1-3: R-1-3	Avg. Flow Depth=1.90' Max Vel=1.97 fps Inflow=51.27 cfs 10.432 af n=0.040 L=225.0' S=0.0022 '/' Capacity=56.74 cfs Outflow=50.97 cfs 10.426 af	
Reach R1-4: R1-4	Avg. Flow Depth=1.05' Max Vel=5.05 fps Inflow=32.80 cfs 3.955 af n=0.035 L=290.0' S=0.0241 '/' Capacity=130.35 cfs Outflow=32.68 cfs 3.955 af	
Reach R1-5: R1-5	Inflow=50.97 cfs 10.426 af Outflow=50.97 cfs 10.426 af	
Reach R2-1: R2-1	Avg. Flow Depth=0.69' Max Vel=2.34 fps Inflow=8.18 cfs 13.716 af n=0.035 L=460.0' S=0.0082 '/' Capacity=76.05 cfs Outflow=8.18 cfs 13.696 af	
Reach R2-2: R2-2	Avg. Flow Depth=0.94' Max Vel=4.56 fps Inflow=16.78 cfs 1.969 af n=0.030 L=175.0' S=0.0171 '/' Capacity=82.75 cfs Outflow=16.74 cfs 1.969 af	
Reach R2-3: R2-3	Avg. Flow Depth=0.97' Max Vel=3.71 fps Inflow=14.34 cfs 15.406 af n=0.030 L=410.0' S=0.0110 '/' Capacity=66.21 cfs Outflow=14.28 cfs 15.393 af	
Reach R3-1: R3-1	Avg. Flow Depth=0.40' Max Vel=3.12 fps Inflow=3.74 cfs 0.629 af n=0.030 L=1,596.0' S=0.0203 '/' Capacity=89.94 cfs Outflow=3.48 cfs 0.629 af	
Reach R3-P: R3-P	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.040 L=276.0' S=0.0056 '/' Capacity=3.77 cfs Outflow=0.00 cfs 0.000 af	
Reach R4-1: R4-1	Avg. Flow Depth=0.54' Max Vel=6.51 fps Inflow=10.91 cfs 2.068 af n=0.030 L=95.0' S=0.0632 '/' Capacity=158.84 cfs Outflow=10.91 cfs 2.068 af	
Reach R4-2: R4-2	Avg. Flow Depth=0.57' Max Vel=7.13 fps Inflow=12.84 cfs 2.298 af n=0.030 L=140.0' S=0.0714 '/' Capacity=168.92 cfs Outflow=12.84 cfs 2.298 af	
Reach R5-1: R5-1	Avg. Flow Depth=0.69' Max Vel=3.97 fps Inflow=14.09 cfs 2.370 af n=0.035 L=640.0' S=0.0234 '/' Capacity=325.19 cfs Outflow=13.99 cfs 2.370 af	
Reach R5-2: R5-2	Avg. Flow Depth=1.41' Max Vel=3.55 fps Inflow=39.28 cfs 5.388 af n=0.035 L=700.0' S=0.0086 '/' Capacity=77.67 cfs Outflow=36.36 cfs 5.388 af	
Reach R6-1: R6-1	Avg. Flow Depth=0.07' Max Vel=1.12 fps Inflow=0.17 cfs 0.024 af n=0.030 L=360.0' S=0.0203 '/' Capacity=90.06 cfs Outflow=0.16 cfs 0.024 af	
Pond 1-P:	Peak Elev=38.65' Storage=2.214 af Inflow=20.97 cfs 3.094 af 12.0" Round Culvert n=0.012 L=23.0' S=0.0000 '/' Outflow=0.99 cfs 1.579 af	
Pond 2-P:	Peak Elev=37.81' Storage=9.450 af Inflow=68.04 cfs 16.875 af Outflow=8.18 cfs 13.716 af	
Kittery_100%Build (ID 285 Prepared by Barton & Loguid HydroCAD® 10.20-2g s/n 05255	i3681)ME-DennettRoad 24-hr S1 2-yrRainfall=3.31"lice, DPCPrinted 8/7/2023© 2022 HydroCAD Software Solutions LLCPage 14	
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Pond 3-P:	Peak Elev=86.32' Storage=0.459 af Inflow=7.12 cfs 0.459 af Outflow=0.00 cfs 0.000 af	
Pond CB-1-1: L1-1	Peak Elev=121.15' Inflow=28.60 cfs 4.677 af 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=28.60 cfs 4.677 af	
Pond CB-1-3: L1-3	Peak Elev=41.26' Inflow=51.27 cfs 10.432 af 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=51.27 cfs 10.432 af	
Pond CB-1-4: L1-4	Peak Elev=44.77' Inflow=32.80 cfs 3.955 af 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=32.80 cfs 3.955 af	
Pond CB-2-3: L2-2	Peak Elev=31.48' Inflow=14.34 cfs 15.406 af 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=14.34 cfs 15.406 af	
Pond CB-2-4: L2-3	Peak Elev=33.02' Inflow=16.78 cfs 1.969 af 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=16.78 cfs 1.969 af	
Pond CB-3-1: L3-1	Peak Elev=87.48' Inflow=3.74 cfs 0.629 af Outflow=3.74 cfs 0.629 af	
Pond CB-3-2: L3-2	Peak Elev=55.44' Inflow=9.84 cfs 1.728 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=9.84 cfs 1.728 af	
Pond CB-4-1: L4-1	Peak Elev=55.53' Inflow=10.91 cfs 2.068 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=10.91 cfs 2.068 af	
Pond CB-4-2: L4-2	Peak Elev=55.72' Inflow=12.84 cfs 2.298 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=12.84 cfs 2.298 af	
Pond CB-5-1: L5-1	Peak Elev=57.46' Inflow=14.09 cfs 2.370 af 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=14.09 cfs 2.370 af	
Pond CB-5-2: L5-2	Peak Elev=43.47' Inflow=39.28 cfs 5.388 af 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=39.28 cfs 5.388 af	
Pond CB-6-1: L6-1	Peak Elev=49.43' Inflow=0.17 cfs 0.024 af 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.17 cfs 0.024 af	
Link L1-5: L1-5	Inflow=50.97 cfs 10.426 af Primary=50.97 cfs 10.426 af	
Link L1-6: L1-6	Inflow=83.02 cfs 14.382 af Primary=83.02 cfs 14.382 af	
Link L2-1: L2-1	Inflow=8.18 cfs 13.716 af Primary=8.18 cfs 13.716 af	
Link L2-4: L2-4	Inflow=14.28 cfs 15.393 af Primary=14.28 cfs 15.393 af	

Kittery_100%Build (ID 2853681)	ME-DennettRoad	124-hr S1 2-yr Rainfall=3.31"
HydroCAD® 10 20-20 s/n 05255 @ 2022 HydroCAD	Software Solutions LLC	Printed 8/7/2023 Page 15
Tiyuloonde 10.20 29 3/100203 @ 2022 Tiyuloond		l age 13
Link L2-5: L2-5		Inflow=16.74 cfs 1.969 af
		Primary=16.74 cfs 1.969 af
Link L2-6: L2-6		Inflow=29.31 cfs 17.361 af
		Primary=29.31 cfs 17.361 af
Link L4-3: L4-3		Inflow=10.91 cfs 2.068 af
		Primary=10.91 cfs 2.068 af
Link L4-4: L4-4		Inflow=12.84 cfs 2.298 af
		Primary=12.84 cfs 2.298 af
Link L4-5: L4-5		Inflow=23.69 cfs 4.365 af
		Primary=23.69 cfs 4.365 af
Link L5-3: L5-3		Inflow=36.36 cfs 5.388 af
		Primary=36.36 cfs 5.388 af
Link L6-2: L6-2		Inflow=0.16 cfs 0.024 af
		Primary=0.16 cfs 0.024 af
Total Punoff Aroa - 222 008 ac	Runoff Volumo - 48 420 of	Average Puneff Depth - 1 70"

Total Runoff Area = 323.908 acRunoff Volume = 48.420 afAverage Runoff Depth = 1.79"96.72% Pervious = 313.275 ac3.28% Impervious = 10.633 ac

Kittery\_100%Build (ID 2853681)ME-DennettRoad 24-hr S1 5-yrRainfall=4.41"Prepared by Barton & Loguidice, DPCPrinted 8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 16

Subcatchment 1-1: 1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth=2.22" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=45.59 cfs 7.569 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth=2.56" Flow Length=485' Tc=27.5 min CN=WQ Runoff=31.76 cfs 4.800 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth=3.52" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=48.88 cfs 5.926 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth=3.63" Flow Length=875' Tc=18.9 min CN=WQ Runoff=44.01 cfs 5.590 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth=2.10" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=45.93 cfs 11.849 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth=3.04" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=52.85 cfs 13.332 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth=3.84" Flow Length=850' Tc=25.6 min CN=WQ Runoff=15.72 cfs 2.382 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth=3.83" Flow Length=745' Tc=17.7 min CN=WQ Runoff=22.14 cfs 2.745 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth=1.51" Flow Length=688' Tc=33.2 min CN=WQ Runoff=6.21 cfs 1.061 af
Subcatchment 3-1-P: DA-3-1	I-P Runoff Area=1.790 ac 100.00% Impervious Runoff Depth=4.17" Tc=0.0 min CN=98 Runoff=9.15 cfs 0.623 af
Subcatchment 3-2: 3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth=0.91" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=14.21 cfs 1.940 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth=2.25" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=14.77 cfs 2.936 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth=3.84" Flow Length=955' Tc=37.6 min CN=WQ Runoff=16.98 cfs 3.202 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth=3.83" Flow Length=625' Tc=33.8 min CN=WQ Runoff=18.64 cfs 3.305 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth=3.84" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=44.67 cfs 4.206 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth=3.84" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.22 cfs 0.034 af

Kittery_100%Build (ID 28	53681) ME-DennettRoad 24-hr S1 5-yr Rainfall=4.41"
Prepared by Barton & Logui	dice, DPC Printed 8/7/2023
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Reach R1-1: R1-1	Avg. Flow Depth=1.46' Max Vel=4.21 fps Inflow=45.59 cfs 7.569 af n=0.035 L=800.0' S=0.0117 '/' Capacity=90.60 cfs Outflow=45.09 cfs 7.569 af
Reach R1-2: R1-2	Avg. Flow Depth=0.18' Max Vel=2.67 fps Inflow=1.74 cfs 2.798 af n=0.035 L=20.0' S=0.0465 '/' Capacity=180.91 cfs Outflow=1.74 cfs 2.798 af
Reach R1-3: R-1-3	Avg. Flow Depth=2.31' Max Vel=2.17 fps Inflow=75.56 cfs 16.293 af n=0.040 L=225.0' S=0.0022 '/' Capacity=56.74 cfs Outflow=75.24 cfs 16.286 af
Reach R1-4: R1-4	Avg. Flow Depth=1.21' Max Vel=5.46 fps Inflow=44.01 cfs 5.590 af n=0.035 L=290.0' S=0.0241 '/' Capacity=130.35 cfs Outflow=43.87 cfs 5.590 af
Reach R1-5: R1-5	Inflow=75.24 cfs 16.286 af Outflow=75.24 cfs 16.286 af
Reach R2-1: R2-1	Avg. Flow Depth=1.42' Max Vel=3.49 fps Inflow=36.05 cfs 21.065 af n=0.035 L=460.0' S=0.0082 '/' Capacity=76.05 cfs Outflow=36.02 cfs 21.041 af
Reach R2-2: R2-2	Avg. Flow Depth=1.08' Max Vel=4.91 fps Inflow=22.14 cfs 2.745 af n=0.030 L=175.0' S=0.0171 '/' Capacity=82.75 cfs Outflow=22.09 cfs 2.745 af
Reach R2-3: R2-3	Avg. Flow Depth=1.55' Max Vel=4.78 fps Inflow=37.80 cfs 23.423 af n=0.030 L=410.0' S=0.0110 '/' Capacity=66.21 cfs Outflow=37.79 cfs 23.407 af
Reach R3-1: R3-1	Avg. Flow Depth=0.53' Max Vel=3.64 fps Inflow=6.21 cfs 1.061 af n=0.030 L=1,596.0' S=0.0203 '/' Capacity=89.94 cfs Outflow=5.87 cfs 1.061 af
Reach R3-P: R3-P	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.040 L=276.0' S=0.0056 '/' Capacity=3.77 cfs Outflow=0.00 cfs 0.000 af
Reach R4-1: R4-1	Avg. Flow Depth=0.64' Max Vel=7.09 fps Inflow=14.77 cfs 2.936 af n=0.030 L=95.0' S=0.0632 '/' Capacity=158.84 cfs Outflow=14.77 cfs 2.936 af
Reach R4-2: R4-2	Avg. Flow Depth=0.66' Max Vel=7.70 fps Inflow=16.98 cfs 3.202 af n=0.030 L=140.0' S=0.0714 '/' Capacity=168.92 cfs Outflow=16.98 cfs 3.202 af
Reach R5-1: R5-1	Avg. Flow Depth=0.80' Max Vel=4.29 fps Inflow=18.64 cfs 3.305 af n=0.035 L=640.0' S=0.0234 '/' Capacity=325.19 cfs Outflow=18.53 cfs 3.305 af
Reach R5-2: R5-2	Avg. Flow Depth=1.62' Max Vel=3.82 fps Inflow=51.98 cfs 7.511 af n=0.035 L=700.0' S=0.0086 '/' Capacity=77.67 cfs Outflow=48.53 cfs 7.511 af
Reach R6-1: R6-1	Avg. Flow Depth=0.08' Max Vel=1.24 fps Inflow=0.22 cfs 0.034 af n=0.030 L=360.0' S=0.0203 '/' Capacity=90.06 cfs Outflow=0.21 cfs 0.034 af
Pond 1-P:	Peak Elev=38.95' Storage=3.232 af Inflow=31.76 cfs 4.800 af 12.0" Round Culvert n=0.012 L=23.0' S=0.0000 '/' Outflow=1.74 cfs 2.798 af
Pond 2-P:	Peak Elev=38.17' Storage=11.632 af Inflow=98.34 cfs 25.180 af Outflow=36.05 cfs 21.065 af

Kittery_100%Build (ID 285 Prepared by Barton & Loguid HydroCAD® 10.20-2g s/n 05255	i3681)ME-DennettRoad 24-hr S1 5-yrRainfall=4.41"lice, DPCPrinted 8/7/2023© 2022 HydroCAD Software Solutions LLCPage 18
Pond 3-P:	Peak Elev=86.43' Storage=0.623 af Inflow=9.15 cfs 0.623 af Outflow=0.00 cfs 0.000 af
Pond CB-1-1: L1-1	Peak Elev=234.94' Inflow=45.59 cfs 7.569 af 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=45.59 cfs 7.569 af
Pond CB-1-3: L1-3	Peak Elev=42.97' Inflow=75.56 cfs 16.293 af 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=75.56 cfs 16.293 af
Pond CB-1-4: L1-4	Peak Elev=52.59' Inflow=44.01 cfs 5.590 af 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=44.01 cfs 5.590 af
Pond CB-2-3: L2-2	Peak Elev=32.72' Inflow=37.80 cfs 23.423 af 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=37.80 cfs 23.423 af
Pond CB-2-4: L2-3	Peak Elev=33.77' Inflow=22.14 cfs 2.745 af 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=22.14 cfs 2.745 af
Pond CB-3-1: L3-1	Peak Elev=87.86' Inflow=6.21 cfs 1.061 af Outflow=6.21 cfs 1.061 af
Pond CB-3-2: L3-2	Peak Elev=56.35' Inflow=17.59 cfs 3.001 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=17.59 cfs 3.001 af
Pond CB-4-1: L4-1	Peak Elev=55.94' Inflow=14.77 cfs 2.936 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=14.77 cfs 2.936 af
Pond CB-4-2: L4-2	Peak Elev=56.26' Inflow=16.98 cfs 3.202 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=16.98 cfs 3.202 af
Pond CB-5-1: L5-1	Peak Elev=57.72' Inflow=18.64 cfs 3.305 af 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=18.64 cfs 3.305 af
Pond CB-5-2: L5-2	Peak Elev=45.07' Inflow=51.98 cfs 7.511 af 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=51.98 cfs 7.511 af
Pond CB-6-1: L6-1	Peak Elev=49.45' Inflow=0.22 cfs 0.034 af 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.22 cfs 0.034 af
Link L1-5: L1-5	Inflow=75.24 cfs 16.286 af Primary=75.24 cfs 16.286 af
Link L1-6: L1-6	Inflow=117.74 cfs 21.875 af Primary=117.74 cfs 21.875 af
Link L2-1: L2-1	Inflow=36.05 cfs 21.065 af Primary=36.05 cfs 21.065 af
Link L2-4: L2-4	Inflow=37.79 cfs 23.407 af Primary=37.79 cfs 23.407 af

Kittery_100%Build (ID 2853681)	ME-DennettRoac	124-hr S1 5-yr Rainfal⊫4.41" Drinted 8/7/2022
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¥¥¥		
Link L2-5: L2-5		Inflow=22.09 cfs 2.745 af
		Primary=22.09 cfs 2.745 af
Link L2-6: L2-6		Inflow=39.75 cfs 26.152 af
		Primary=39.75 cfs 26.152 af
Link L4-3: L4-3		Inflow=14.77 cfs 2.936 af
		Primary=14.77 cfs 2.936 af
Link L4-4: L4-4		Inflow=16.98 cfs 3.202 af
		Primary=16.98 cfs 3.202 af
Link L4-5: L4-5		Inflow=31.67 cfs 6.138 af
		Primary=31.67 cfs 6.138 af
Link L5-3: L5-3		Inflow=48.53 cfs 7.511 af
		Primary=48.53 cfs 7.511 af
Link L6-2: L6-2		Inflow=0.21 cfs 0.034 af
		Primary=0.21 cfs 0.034 af
Total Runoff Area - 323 908 ac	Runoff Volume - 71 499 af	Average Runoff Denth - 2 65"

Total Runoff Area = 323.908 acRunoff Volume = 71.499 afAverage Runoff Depth = 2.65"96.72% Pervious = 313.275 ac3.28% Impervious = 10.633 ac

Kittery\_100%Build (ID 2853681)ME-DennettRoad 24-hr S1 10-yr Rainfall=5.32"Prepared by Barton & Loguidice, DPCPrinted 8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 20

Subcatchment 1-1: 1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth=2.97" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=59.93 cfs 10.129 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth=3.35" Flow Length=485' Tc=27.5 min CN=WQ Runoff=40.78 cfs 6.290 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth=4.39" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=59.18 cfs 7.398 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth=4.52" Flow Length=875' Tc=18.9 min CN=WQ Runoff=53.04 cfs 6.957 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth=2.79" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=59.94 cfs 15.750 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth=3.79" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=63.87 cfs 16.616 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth=4.74" Flow Length=850' Tc=25.6 min CN=WQ Runoff=18.78 cfs 2.942 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth=4.73" Flow Length=745' Tc=17.7 min CN=WQ Runoff=26.44 cfs 3.391 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth=2.07" Flow Length=688' Tc=33.2 min CN=WQ Runoff=8.38 cfs 1.462 af
Subcatchment 3-1-P: DA-3-7	I-P Runoff Area=1.790 ac 100.00% Impervious Runoff Depth=5.08" Tc=0.0 min CN=98 Runoff=10.82 cfs 0.758 af
Subcatchment 3-2: 3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth=1.29" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=20.07 cfs 2.759 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth=2.81" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=17.92 cfs 3.680 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth=4.74" Flow Length=955' Tc=37.6 min CN=WQ Runoff=20.29 cfs 3.954 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth=4.73" Flow Length=625' Tc=33.8 min CN=WQ Runoff=22.27 cfs 4.082 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth=4.74" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=53.33 cfs 5.194 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth=4.74" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.26 cfs 0.041 af

Kittery_100%Build (ID 285	3681) ME-DennettRoad 24-hr S1 10-yr Rainfall=5.32"
Prepared by Barton & Loguid	dice, DPC Printed 8/7/2023
HydroCAD® 10.20-2g s/n 05255	© 2022 HydroCAD Software Solutions LLC Page 21
Reach R1-1: R1-1	Avg. Flow Depth=1.65' Max Vel=4.52 fps Inflow=59.93 cfs 10.129 af n=0.035 L=800.0' S=0.0117 '/' Capacity=90.60 cfs Outflow=59.37 cfs 10.129 af
Reach R1-2: R1-2	Avg. Flow Depth=0.22' Max Vel=2.92 fps Inflow=2.30 cfs 3.848 af n=0.035 L=20.0' S=0.0465 '/' Capacity=180.91 cfs Outflow=2.30 cfs 3.848 af
Reach R1-3: R-1-3	Avg. Flow Depth=2.65' Max Vel=2.27 fps Inflow=96.07 cfs 21.375 af n=0.040 L=225.0' S=0.0022 '/' Capacity=56.74 cfs Outflow=95.72 cfs 21.365 af
Reach R1-4: R1-4	Avg. Flow Depth=1.32' Max Vel=5.73 fps Inflow=53.04 cfs 6.957 af n=0.035 L=290.0' S=0.0241 '/' Capacity=130.35 cfs Outflow=52.88 cfs 6.957 af
Reach R1-5: R1-5	Inflow=95.72 cfs 21.365 af Outflow=95.72 cfs 21.365 af
Reach R2-1: R2-1	Avg. Flow Depth=1.97' Max Vel=4.19 fps Inflow=73.57 cfs 28.009 af n=0.035 L=460.0' S=0.0082 '/' Capacity=76.05 cfs Outflow=73.49 cfs 27.984 af
Reach R2-2: R2-2	Avg. Flow Depth=1.18' Max Vel=5.14 fps Inflow=26.44 cfs 3.391 af n=0.030 L=175.0' S=0.0171 '/' Capacity=82.75 cfs Outflow=26.39 cfs 3.391 af
Reach R2-3: R2-3	Avg. Flow Depth=2.14' Max Vel=5.71 fps Inflow=76.30 cfs 30.926 af n=0.030 L=410.0' S=0.0110 '/' Capacity=66.21 cfs Outflow=76.27 cfs 30.910 af
Reach R3-1: R3-1	Avg. Flow Depth=0.62' Max Vel=3.96 fps Inflow=8.38 cfs 1.462 af n=0.030 L=1,596.0' S=0.0203 '/' Capacity=89.94 cfs Outflow=7.98 cfs 1.462 af
Reach R3-P: R3-P	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.040 L=276.0' S=0.0056 '/' Capacity=3.77 cfs Outflow=0.00 cfs 0.000 af
Reach R4-1: R4-1	Avg. Flow Depth=0.70' Max Vel=7.48 fps Inflow=17.92 cfs 3.680 af n=0.030 L=95.0' S=0.0632 '/' Capacity=158.84 cfs Outflow=17.93 cfs 3.680 af
Reach R4-2: R4-2	Avg. Flow Depth=0.73' Max Vel=8.09 fps Inflow=20.29 cfs 3.954 af n=0.030 L=140.0' S=0.0714 '/' Capacity=168.92 cfs Outflow=20.29 cfs 3.954 af
Reach R5-1: R5-1	Avg. Flow Depth=0.87' Max Vel=4.50 fps Inflow=22.27 cfs 4.082 af n=0.035 L=640.0' S=0.0234 '/' Capacity=325.19 cfs Outflow=22.15 cfs 4.082 af
Reach R5-2: R5-2	Avg. Flow Depth=1.76' Max Vel=4.01 fps Inflow=62.19 cfs 9.277 af n=0.035 L=700.0' S=0.0086 '/' Capacity=77.67 cfs Outflow=58.35 cfs 9.277 af
Reach R6-1: R6-1	Avg. Flow Depth=0.09' Max Vel=1.33 fps Inflow=0.26 cfs 0.041 af n=0.030 L=360.0' S=0.0203 '/' Capacity=90.06 cfs Outflow=0.26 cfs 0.041 af
Pond 1-P:	Peak Elev=39.23' Storage=4.171 af Inflow=40.78 cfs 6.290 af 12.0" Round Culvert n=0.012 L=23.0' S=0.0000 '/' Outflow=2.30 cfs 3.848 af
Pond 2-P:	Peak Elev=38.29' Storage=12.565 af Inflow=123.32 cfs 32.366 af Outflow=73.57 cfs 28.009 af

Kittery_100%Build (ID 2 Prepared by Barton & Log	853681)ME-DennettRoad 24-hr S1 10-yr Rainfall=5.32"uidice, DPCPrinted 8/7/2023
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Pond 3-P:	Peak Elev=86.52' Storage=0.758 af Inflow=10.82 cfs 0.758 af Outflow=0.00 cfs 0.000 af
Pond CB-1-1: L1-1	Peak Elev=371.42' Inflow=59.93 cfs 10.129 af 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=59.93 cfs 10.129 af
Pond CB-1-3: L1-3	Peak Elev=44.93' Inflow=96.07 cfs 21.375 af 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=96.07 cfs 21.375 af
Pond CB-1-4: L1-4	Peak Elev=60.55' Inflow=53.04 cfs 6.957 af 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=53.04 cfs 6.957 af
Pond CB-2-3: L2-2	Peak Elev=36.53' Inflow=76.30 cfs 30.926 af 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=76.30 cfs 30.926 af
Pond CB-2-4: L2-3	Peak Elev=34.53' Inflow=26.44 cfs 3.391 af 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=26.44 cfs 3.391 af
Pond CB-3-1: L3-1	Peak Elev=88.29' Inflow=8.38 cfs 1.462 af Outflow=8.38 cfs 1.462 af
Pond CB-3-2: L3-2	Peak Elev=57.68' Inflow=24.79 cfs 4.221 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=24.79 cfs 4.221 af
Pond CB-4-1: L4-1	Peak Elev=56.40' Inflow=17.92 cfs 3.680 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=17.92 cfs 3.680 af
Pond CB-4-2: L4-2	Peak Elev=56.80' Inflow=20.29 cfs 3.954 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=20.29 cfs 3.954 af
Pond CB-5-1: L5-1	Peak Elev=57.91' Inflow=22.27 cfs 4.082 af 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=22.27 cfs 4.082 af
Pond CB-5-2: L5-2	Peak Elev=46.68' Inflow=62.19 cfs 9.277 af 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=62.19 cfs 9.277 af
Pond CB-6-1: L6-1	Peak Elev=49.47' Inflow=0.26 cfs 0.041 af 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.26 cfs 0.041 af
Link L1-5: L1-5	Inflow=95.72 cfs 21.365 af Primary=95.72 cfs 21.365 af
Link L1-6: L1-6	Inflow=146.48 cfs 28.323 af Primary=146.48 cfs 28.323 af
Link L2-1: L2-1	Inflow=73.57 cfs 28.009 af Primary=73.57 cfs 28.009 af
Link L2-4: L2-4	Inflow=76.27 cfs 30.910 af Primary=76.27 cfs 30.910 af

Kittery_100%Build (ID 2853681)	ME-DennettRoad 24-hr S1 10-yr Rainfall=5.32'
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Link L2-5: L2-5	Inflow=26.39 cfs 3.391 at
	Primary=26.39 cfs 3.391 at
Link L2-6: L2-6	Inflow=79.25 cfs 34.301 af
	Primary=79.25 cfs 34.301 af
Link L4-3: L4-3	Inflow=17.93 cfs_3.680 af
	Primary=17.93 cfs 3.680 af
l ink   4-4 ·   4-4	Inflow-20 29 cfs 3 954 af
	Primary=20.29 cfs 3.954 af
Link L4-5: L4-5	Inflow=38.11 cfs 7.633 af
	Primary=38.11 cfs 7.633 af
Link L5-3: L5-3	Inflow=58.35 cfs 9.277 af
	Primary=58.35 cfs 9.277 af
Link L6-2: L6-2	Inflow=0.26 cfs_0.041 af
	Primary=0.26 cfs 0.041 af
Total Runoff Area = 323.908 ac	Runoff Volume = 91.403 af Average Runoff Depth = 3.39"

96.72% Pervious = 313.275 ac 3.28% Impervious = 10.633 ac

Kittery\_100%Build (ID 2853681)ME-DennettRoad 24-hr S1 25-yrRainfall=6.58"Prepared by Barton & Loguidice, DPCPrinted 8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 24

Subcatchment 1-1: 1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth=4.06" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=79.98 cfs 13.839 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth=4.49" Flow Length=485' Tc=27.5 min CN=WQ Runoff=53.32 cfs 8.427 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth=5.61" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=73.30 cfs 9.457 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth=5.76" Flow Length=875' Tc=18.9 min CN=WQ Runoff=65.36 cfs 8.864 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth=3.79" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=79.60 cfs 21.444 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth=4.86" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=78.99 cfs 21.273 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth=5.99" Flow Length=850' Tc=25.6 min CN=WQ Runoff=22.94 cfs 3.719 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth=5.98" Flow Length=745' Tc=17.7 min CN=WQ Runoff=32.31 cfs 4.288 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth=2.93" Flow Length=688' Tc=33.2 min CN=WQ Runoff=11.56 cfs 2.067 af
Subcatchment 3-1-P: DA-3-7	I-P Runoff Area=1.790 ac 100.00% Impervious Runoff Depth=6.34" Tc=0.0 min CN=98 Runoff=13.11 cfs 0.946 af
Subcatchment 3-2: 3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth=1.90" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=28.92 cfs 4.082 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth=3.65" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=22.29 cfs 4.770 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth=5.99" Flow Length=955' Tc=37.6 min CN=WQ Runoff=24.79 cfs 4.998 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth=5.98" Flow Length=625' Tc=33.8 min CN=WQ Runoff=27.22 cfs 5.163 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth=5.99" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=65.14 cfs 6.566 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth=5.99" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.32 cfs 0.052 af

Kittery_100%Build (ID	<b>2853681)</b> <i>ME-DennettRoad</i> 24-hr S1 25-yr <i>Rainfall</i> =6.58"
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Reach R1-1: R1-1	Avg. Flow Depth=1.88' Max Vel=4.86 fps Inflow=79.98 cfs 13.839 af n=0.035 L=800.0' S=0.0117 '/' Capacity=90.60 cfs Outflow=79.33 cfs 13.839 af
Reach R1-2: R1-2	Avg. Flow Depth=0.26' Max Vel=3.28 fps Inflow=3.27 cfs 5.383 af n=0.035 L=20.0' S=0.0465 '/' Capacity=180.91 cfs Outflow=3.27 cfs 5.382 af
Reach R1-3: R-1-3	Avg. Flow Depth=3.13' Max Vel=2.36 fps Inflow=124.89 cfs 28.679 af n=0.040 L=225.0' S=0.0022 '/' Capacity=56.74 cfs Outflow=124.46 cfs 28.667 af
Reach R1-4: R1-4	Avg. Flow Depth=1.46' Max Vel=6.05 fps Inflow=65.36 cfs 8.864 af n=0.035 L=290.0' S=0.0241 '/' Capacity=130.35 cfs Outflow=65.17 cfs 8.864 af
Reach R1-5: R1-5	Inflow=124.46 cfs 28.667 af Outflow=124.46 cfs 28.667 af
Reach R2-1: R2-1	Avg. Flow Depth=2.61' Max Vel=4.71 fps Inflow=127.59 cfs 38.235 af n=0.035 L=460.0' S=0.0082 '/' Capacity=76.05 cfs Outflow=127.43 cfs 38.209 af
Reach R2-2: R2-2	Avg. Flow Depth=1.30' Max Vel=5.41 fps Inflow=32.31 cfs 4.288 af n=0.030 L=175.0' S=0.0171 '/' Capacity=82.75 cfs Outflow=32.26 cfs 4.288 af
Reach R2-3: R2-3	Avg. Flow Depth=2.89' Max Vel=6.31 fps Inflow=131.81 cfs 41.928 af n=0.030 L=410.0' S=0.0110 '/' Capacity=66.21 cfs Outflow=131.74 cfs 41.912 af
Reach R3-1: R3-1	Avg. Flow Depth=0.74' Max Vel=4.34 fps Inflow=11.56 cfs 2.067 af n=0.030 L=1,596.0' S=0.0203 '/' Capacity=89.94 cfs Outflow=11.09 cfs 2.067 af
Reach R3-P: R3-P	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.040 L=276.0' S=0.0056 '/' Capacity=3.77 cfs Outflow=0.00 cfs 0.000 af
Reach R4-1: R4-1	Avg. Flow Depth=0.79' Max Vel=7.94 fps Inflow=22.29 cfs 4.770 af n=0.030 L=95.0' S=0.0632 '/' Capacity=158.84 cfs Outflow=22.30 cfs 4.770 af
Reach R4-2: R4-2	Avg. Flow Depth=0.80' Max Vel=8.54 fps Inflow=24.79 cfs 4.998 af n=0.030 L=140.0' S=0.0714 '/' Capacity=168.92 cfs Outflow=24.79 cfs 4.998 af
Reach R5-1: R5-1	Avg. Flow Depth=0.97' Max Vel=4.75 fps Inflow=27.22 cfs 5.163 af n=0.035 L=640.0' S=0.0234 '/' Capacity=325.19 cfs Outflow=27.09 cfs 5.163 af
Reach R5-2: R5-2	Avg. Flow Depth=1.93' Max Vel=4.23 fps Inflow=76.12 cfs 11.729 af n=0.035 L=700.0' S=0.0086 '/' Capacity=77.67 cfs Outflow=71.88 cfs 11.729 af
Reach R6-1: R6-1	Avg. Flow Depth=0.10' Max Vel=1.42 fps Inflow=0.32 cfs 0.052 af n=0.030 L=360.0' S=0.0203 '/' Capacity=90.06 cfs Outflow=0.31 cfs 0.052 af
Pond 1-P:	Peak Elev=39.61' Storage=5.480 af Inflow=53.32 cfs 8.427 af 12.0" Round Culvert n=0.012 L=23.0' S=0.0000 '/' Outflow=3.27 cfs 5.383 af
Pond 2-P:	Peak Elev=38.44' Storage=13.623 af Inflow=158.04 cfs 42.716 af Outflow=127.59 cfs 38.235 af

Kittery_100%Build (I Prepared by Barton & I	D 2853681) ME-DennettRoad 24-hr S1 25-yr Rainfall=6.58" _oguidice, DPC Printed 8/7/2023
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Pond 3-P:	Peak Elev=86.65' Storage=0.946 af Inflow=13.11 cfs 0.946 af Outflow=0.00 cfs 0.000 af
Pond CB-1-1: L1-1	Peak Elev=624.27' Inflow=79.98 cfs 13.839 af 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=79.98 cfs 13.839 af
Pond CB-1-3: L1-3	Peak Elev=48.57' Inflow=124.89 cfs 28.679 af 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=124.89 cfs 28.679 af
Pond CB-1-4: L1-4	Peak Elev=73.79' Inflow=65.36 cfs 8.864 af 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=65.36 cfs 8.864 af
Pond CB-2-3: L2-2	Peak Elev=46.50' Inflow=131.81 cfs 41.928 af 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=131.81 cfs 41.928 af
Pond CB-2-4: L2-3	Peak Elev=35.85' Inflow=32.31 cfs 4.288 af 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=32.31 cfs 4.288 af
Pond CB-3-1: L3-1	Peak Elev=89.17' Inflow=11.56 cfs 2.067 af Outflow=11.56 cfs 2.067 af
Pond CB-3-2: L3-2	Peak Elev=60.56' Inflow=35.65 cfs 6.149 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=35.65 cfs 6.149 af
Pond CB-4-1: L4-1	Peak Elev=57.17' Inflow=22.29 cfs 4.770 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=22.29 cfs 4.770 af
Pond CB-4-2: L4-2	Peak Elev=57.69' Inflow=24.79 cfs 4.998 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=24.79 cfs 4.998 af
Pond CB-5-1: L5-1	Peak Elev=58.16' Inflow=27.22 cfs 5.163 af 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=27.22 cfs 5.163 af
Pond CB-5-2: L5-2	Peak Elev=49.35' Inflow=76.12 cfs 11.729 af 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=76.12 cfs 11.729 af
Pond CB-6-1: L6-1	Peak Elev=49.50' Inflow=0.32 cfs 0.052 af 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.32 cfs 0.052 af
Link L1-5: L1-5	Inflow=124.46 cfs 28.667 af Primary=124.46 cfs 28.667 af
Link L1-6: L1-6	Inflow=186.43 cfs 37.532 af Primary=186.43 cfs 37.532 af
Link L2-1: L2-1	Inflow=127.59 cfs 38.235 af Primary=127.59 cfs 38.235 af
Link L2-4: L2-4	Inflow=131.74 cfs 41.912 af Primary=131.74 cfs 41.912 af

Kittery_100%Build (ID 2853681)	ME-DennettRoad 2	24-hr S1 25-yr Rainfall=6.58"
Prepared by Barton & Loguidice, DPC		Printed 8/7/2023
HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCA	D Software Solutions LLC	Page 27
LINK L2-5: L2-5		Inflow=32.26 cfs 4.288 af
		Primary=32.26 cts 4.288 at
Link L2-6: L2-6		Inflow=136.12 cfs 46.199 af
		Primary=136.12 cfs 46.199 af
Link L4-3: L4-3		Inflow=22.30 cfs 4.770 af
		Primary=22.30 cfs 4.770 af
l ink   4-4 <sup>.</sup>   4-4		Inflow=24.79 cfs 4.998 af
		Primary=24.79 cfs 4.998 af
		-
Link L4-5: L4-5		Inflow=46.96 cfs 9.768 af
		Primary=46.96 cfs 9.768 af
link   5-3   5-3		Inflow-71 88 cfs 11 729 af
LIIR LJ-J. LJ-J		Primary-71.88 cfs 11.729 af
		1 minary=71.00 ci3 11.729 ai
Link L6-2: L6-2		Inflow=0.31 cfs 0.052 af
-		Primary=0.31 cfs 0.052 af
Total Runoff Area = 323.908 ac	Runoff Volume = 119.955 af	Average Runoff Depth = 4.44"

96.72% Pervious = 313.275 ac 3.28% Impervious = 10.633 ac

Kittery\_100%Build (ID 2853681)ME-DennettRoad 24-hr S1 50-yrRainfall=7.50"Prepared by Barton & Loguidice, DPCPrinted 8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 28

Subcatchment 1-1: 1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth=4.88" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=95.05 cfs 16.633 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth=5.34" Flow Length=485' Tc=27.5 min CN=WQ Runoff=62.73 cfs 10.024 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth=6.51" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=83.88 cfs 10.972 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth=6.66" Flow Length=875' Tc=18.9 min CN=WQ Runoff=74.57 cfs 10.263 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth=4.56" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=94.46 cfs 25.759 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth=5.65" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=90.35 cfs 24.739 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth=6.90" Flow Length=850' Tc=25.6 min CN=WQ Runoff=26.06 cfs 4.288 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth=6.89" Flow Length=745' Tc=17.7 min CN=WQ Runoff=36.70 cfs 4.944 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth=3.60" Flow Length=688' Tc=33.2 min CN=WQ Runoff=14.07 cfs 2.537 af
Subcatchment 3-1-P: DA-3-	I-PRunoff Area=1.790 ac100.00% ImperviousRunoff Depth=7.26"Tc=0.0 minCN=98Runoff=14.83 cfs1.083 af
Subcatchment 3-2: 3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth=2.41" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=36.12 cfs 5.162 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth=4.29" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=25.60 cfs 5.605 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth=6.90" Flow Length=955' Tc=37.6 min CN=WQ Runoff=28.17 cfs 5.762 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth=6.89" Flow Length=625' Tc=33.8 min CN=WQ Runoff=30.93 cfs 5.954 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth=6.90" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=73.98 cfs 7.570 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth=6.90" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.37 cfs 0.060 af

Kittery_100%Build (ID 2	2853681) ME-DennettRoad 24-hr S1 50-yr Rainfall=7.50"
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Reach R1-1: R1-1	Avg. Flow Depth=2.04' Max Vel=5.08 fps Inflow=95.05 cfs 16.633 af n=0.035 L=800.0' S=0.0117 '/' Capacity=90.60 cfs Outflow=94.33 cfs 16.633 af
Reach R1-2: R1-2	Avg. Flow Depth=0.28' Max Vel=3.40 fps Inflow=3.68 cfs 6.386 af n=0.035 L=20.0' S=0.0465 '/' Capacity=180.91 cfs Outflow=3.68 cfs 6.386 af
Reach R1-3: R-1-3	Avg. Flow Depth=3.49' Max Vel=2.41 fps Inflow=146.68 cfs 33.991 af n=0.040 L=225.0' S=0.0022 '/' Capacity=56.74 cfs Outflow=146.19 cfs 33.977 af
Reach R1-4: R1-4	Avg. Flow Depth=1.55' Max Vel=6.27 fps Inflow=74.57 cfs 10.263 af n=0.035 L=290.0' S=0.0241 '/' Capacity=130.35 cfs Outflow=74.36 cfs 10.263 af
Reach R1-5: R1-5	Inflow=146.19 cfs 33.977 af Outflow=146.19 cfs 33.977 af
Reach R2-1: R2-1	Avg. Flow Depth=3.02' Max Vel=4.88 fps Inflow=162.88 cfs 45.960 af n=0.035 L=460.0' S=0.0082 '/' Capacity=76.05 cfs Outflow=162.70 cfs 45.934 af
Reach R2-2: R2-2	Avg. Flow Depth=1.38' Max Vel=5.60 fps Inflow=36.70 cfs 4.944 af n=0.030 L=175.0' S=0.0171 '/' Capacity=82.75 cfs Outflow=36.65 cfs 4.944 af
Reach R2-3: R2-3	Avg. Flow Depth=3.39' Max Vel=6.51 fps Inflow=168.24 cfs 50.221 af n=0.030 L=410.0' S=0.0110 '/' Capacity=66.21 cfs Outflow=168.16 cfs 50.205 af
Reach R3-1: R3-1	Avg. Flow Depth=0.81' Max Vel=4.58 fps Inflow=14.07 cfs 2.537 af n=0.030 L=1,596.0' S=0.0203 '/' Capacity=89.94 cfs Outflow=13.55 cfs 2.537 af
Reach R3-P: R3-P	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.040 L=276.0' S=0.0056 '/' Capacity=3.77 cfs Outflow=0.00 cfs 0.000 af
Reach R4-1: R4-1	Avg. Flow Depth=0.84' Max Vel=8.24 fps Inflow=25.60 cfs 5.605 af n=0.030 L=95.0' S=0.0632 '/' Capacity=158.84 cfs Outflow=25.62 cfs 5.605 af
Reach R4-2: R4-2	Avg. Flow Depth=0.86' Max Vel=8.84 fps Inflow=28.17 cfs 5.762 af n=0.030 L=140.0' S=0.0714 '/' Capacity=168.92 cfs Outflow=28.17 cfs 5.762 af
Reach R5-1: R5-1	Avg. Flow Depth=1.03' Max Vel=4.92 fps Inflow=30.93 cfs 5.954 af n=0.035 L=640.0' S=0.0234 '/' Capacity=325.19 cfs Outflow=30.80 cfs 5.954 af
Reach R5-2: R5-2	Avg. Flow Depth=2.05' Max Vel=4.37 fps Inflow=86.55 cfs 13.524 af n=0.035 L=700.0' S=0.0086 '/' Capacity=77.67 cfs Outflow=81.95 cfs 13.524 af
Reach R6-1: R6-1	Avg. Flow Depth=0.11' Max Vel=1.49 fps Inflow=0.37 cfs 0.060 af n=0.030 L=360.0' S=0.0203 '/' Capacity=90.06 cfs Outflow=0.36 cfs 0.060 af
Pond 1-P:	Peak Elev=39.93' Storage=6.578 af Inflow=62.73 cfs 10.024 af 12.0" Round Culvert n=0.012 L=23.0' S=0.0000 '/' Outflow=3.68 cfs 6.386 af
Pond 2-P:	Peak Elev=38.52' Storage=14.228 af Inflow=184.22 cfs 50.498 af Outflow=162.88 cfs 45.960 af

Kittery_100%Buil	d (ID 2853681) ME-DennettRoad 24-hr S1 50-yr Rainfall=7.50"
Prepared by Bartor	N & Loguidice, DPC Printed 8/7/2023
<u>HyuloCAD® 10.20-29</u>	S/IT 05255 © 2022 Hydrocad Software Solditons LLC Page 50
Pond 3-P:	Peak Elev=86.74' Storage=1.083 af Inflow=14.83 cfs 1.083 af Outflow=0.00 cfs 0.000 af
Pond CB-1-1: L1-1	Peak Elev=861.96' Inflow=95.05 cfs 16.633 af 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=95.05 cfs 16.633 af
Pond CB-1-3: L1-3	Peak Elev=52.21' Inflow=146.68 cfs 33.991 af 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=146.68 cfs 33.991 af
Pond CB-1-4: L1-4	Peak Elev=85.49' Inflow=74.57 cfs 10.263 af 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=74.57 cfs 10.263 af
Pond CB-2-3: L2-2	Peak Elev=56.32' Inflow=168.24 cfs 50.221 af 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=168.24 cfs 50.221 af
Pond CB-2-4: L2-3	Peak Elev=37.25' Inflow=36.70 cfs 4.944 af 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=36.70 cfs 4.944 af
Pond CB-3-1: L3-1	Peak Elev=90.06' Inflow=14.07 cfs 2.537 af Outflow=14.07 cfs 2.537 af
Pond CB-3-2: L3-2	Peak Elev=63.63' Inflow=44.45 cfs 7.698 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=44.45 cfs 7.698 af
Pond CB-4-1: L4-1	Peak Elev=57.86' Inflow=25.60 cfs 5.605 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=25.60 cfs 5.605 af
Pond CB-4-2: L4-2	Peak Elev=58.47' Inflow=28.17 cfs 5.762 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=28.17 cfs 5.762 af
Pond CB-5-1: L5-1	Peak Elev=58.35' Inflow=30.93 cfs 5.954 af 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=30.93 cfs 5.954 af
Pond CB-5-2: L5-2	Peak Elev=51.69' Inflow=86.55 cfs 13.524 af 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=86.55 cfs 13.524 af
Pond CB-6-1: L6-1	Peak Elev=49.51' Inflow=0.37 cfs 0.060 af 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.37 cfs 0.060 af
Link L1-5: L1-5	Inflow=146.19 cfs 33.977 af Primary=146.19 cfs 33.977 af
Link L1-6: L1-6	Inflow=216.52 cfs 44.240 af Primary=216.52 cfs 44.240 af
Link L2-1: L2-1	Inflow=162.88 cfs 45.960 af Primary=162.88 cfs 45.960 af
Link L2-4: L2-4	Inflow=168.16 cfs 50.205 af Primary=168.16 cfs 50.205 af

Kittery_100%Build (ID 2853681)	ME-DennettRoad 2	24-hr S1 50-yr Rainfall=7.50"
Prepared by Barton & Loguidice, DPC		Printed 8/7/2023
HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCA	D Software Solutions LLC	Page 31
		Inflow 26 65 etc. 4 044 of
LINK L2-3: L2-3		Innow=36.65 cts 4.944 at
		Fillinaly=30.05 CIS 4.944 al
Link L2-6: L2-6		Inflow=173.54 cfs 55.149 af
		Primary=173.54 cfs 55.149 af
Link L4-3: L4-3		Inflow=25.62 cfs 5.605 af
		Primary=25.62 cfs 5.605 af
l ink   4-4 ·   4-4		Inflow=28.17 cfs 5.762 af
		Primary=28.17 cfs 5.762 af
l ink   4-5   4-5		Inflow=53.63 cfs_11.367 af
		Primary=53.63 cfs 11.367 af
l ink   5-3   5-3		Inflow=81 95 cfs 13 524 af
		Primary=81.95 cfs 13.524 af
l ink   6-2   6-2		Inflow=0.36 cfs 0.060 af
		Primary=0.36 cfs 0.060 af
Total Runoff Area = 323.908 ac	Runoff Volume = 141.354 af	Average Runoff Depth = 5.24"

96.72% Pervious = 313.275 ac 3.28% Impervious = 10.633 ac

Kittery\_100%Build (ID 2853681)ME-DennettRoad 24-hr S1 100-yrRainfall=8.52"Prepared by Barton & Loguidice, DPCPrinted 8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 32

Subcatchment 1-1: 1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth=5.80" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=111.40 cfs 19.791 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth=6.30" Flow Length=485' Tc=27.5 min CN=WQ Runoff=72.89 cfs 11.821 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth=7.51" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=95.18 cfs 12.658 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth=7.67" Flow Length=875' Tc=18.9 min CN=WQ Runoff=84.39 cfs 11.818 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth=5.42" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=110.82 cfs 30.661 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth=6.54" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=102.76 cfs 28.635 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth=7.92" Flow Length=850' Tc=25.6 min CN=WQ Runoff=29.39 cfs 4.919 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth=7.91" Flow Length=745' Tc=17.7 min CN=WQ Runoff=41.38 cfs 5.673 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth=4.37" Flow Length=688' Tc=33.2 min CN=WQ Runoff=16.91 cfs 3.080 af
Subcatchment 3-1-P: DA-3-	I-P Runoff Area=1.790 ac 100.00% Impervious Runoff Depth=8.28" Tc=0.0 min CN=98 Runoff=16.65 cfs 1.235 af
Subcatchment 3-2: 3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth=3.01" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=44.42 cfs 6.451 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth=5.02" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=29.28 cfs 6.563 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth=7.92" Flow Length=955' Tc=37.6 min CN=WQ Runoff=31.79 cfs 6.610 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth=7.91" Flow Length=625' Tc=33.8 min CN=WQ Runoff=34.91 cfs 6.831 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth=7.92" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=83.36 cfs 8.684 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth=7.92" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.41 cfs 0.069 af

Kittery_100%Build (ID 2	853681) ME-DennettRoad 24-hr S1 100-yr Rainfall=8.52"
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Reach R1-1: R1-1	Avg. Flow Depth=2.20' Max Vel=5.27 fps Inflow=111.40 cfs 19.791 af n=0.035 L=800.0' S=0.0117 '/' Capacity=90.60 cfs Outflow=110.57 cfs 19.791 af
Reach R1-2: R1-2	Avg. Flow Depth=0.63' Max Vel=5.27 fps Inflow=17.05 cfs 7.971 af n=0.035 L=20.0' S=0.0465 '/' Capacity=180.91 cfs Outflow=16.07 cfs 7.971 af
Reach R1-3: R-1-3	Avg. Flow Depth=3.88' Max Vel=2.45 fps Inflow=170.30 cfs 40.420 af n=0.040 L=225.0' S=0.0022 '/' Capacity=56.74 cfs Outflow=169.74 cfs 40.406 af
Reach R1-4: R1-4	Avg. Flow Depth=1.64' Max Vel=6.47 fps Inflow=84.39 cfs 11.818 af n=0.035 L=290.0' S=0.0241 '/' Capacity=130.35 cfs Outflow=84.18 cfs 11.818 af
Reach R1-5: R1-5	Inflow=169.74 cfs 40.406 af Outflow=169.74 cfs 40.406 af
Reach R2-1: R2-1	Avg. Flow Depth=3.42' Max Vel=5.00 fps Inflow=196.48 cfs 54.709 af n=0.035 L=460.0' S=0.0082 '/' Capacity=76.05 cfs Outflow=196.28 cfs 54.683 af
Reach R2-2: R2-2	Avg. Flow Depth=1.46' Max Vel=5.77 fps Inflow=41.38 cfs 5.673 af n=0.030 L=175.0' S=0.0171 '/' Capacity=82.75 cfs Outflow=41.33 cfs 5.673 af
Reach R2-3: R2-3	Avg. Flow Depth=3.86' Max Vel=6.64 fps Inflow=203.01 cfs 59.602 af n=0.030 L=410.0' S=0.0110 '/' Capacity=66.21 cfs Outflow=202.92 cfs 59.585 af
Reach R3-1: R3-1	Avg. Flow Depth=0.89' Max Vel=4.82 fps Inflow=16.91 cfs 3.080 af n=0.030 L=1,596.0' S=0.0203 '/' Capacity=89.94 cfs Outflow=16.34 cfs 3.080 af
Reach R3-P: R3-P	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.040 L=276.0' S=0.0056 '/' Capacity=3.77 cfs Outflow=0.00 cfs 0.000 af
Reach R4-1: R4-1	Avg. Flow Depth=0.90' Max Vel=8.54 fps Inflow=29.28 cfs 6.563 af n=0.030 L=95.0' S=0.0632 '/' Capacity=158.84 cfs Outflow=29.29 cfs 6.563 af
Reach R4-2: R4-2	Avg. Flow Depth=0.91' Max Vel=9.13 fps Inflow=31.79 cfs 6.610 af n=0.030 L=140.0' S=0.0714 '/' Capacity=168.92 cfs Outflow=31.79 cfs 6.610 af
Reach R5-1: R5-1	Avg. Flow Depth=1.09' Max Vel=5.08 fps Inflow=34.91 cfs 6.831 af n=0.035 L=640.0' S=0.0234 '/' Capacity=325.19 cfs Outflow=34.77 cfs 6.831 af
Reach R5-2: R5-2	Avg. Flow Depth=2.17' Max Vel=4.50 fps Inflow=97.66 cfs 15.515 af n=0.035 L=700.0' S=0.0086 '/' Capacity=77.67 cfs Outflow=92.61 cfs 15.515 af
Reach R6-1: R6-1	Avg. Flow Depth=0.12' Max Vel=1.56 fps Inflow=0.41 cfs 0.069 af n=0.030 L=360.0' S=0.0203 '/' Capacity=90.06 cfs Outflow=0.40 cfs 0.069 af
Pond 1-P:	Peak Elev=71.01' Storage=6.806 af Inflow=72.89 cfs 11.821 af 12.0" Round Culvert n=0.012 L=23.0' S=0.0000 '/' Outflow=17.05 cfs 7.971 af
Pond 2-P:	Peak Elev=38.60' Storage=14.763 af Inflow=212.96 cfs 59.296 af Outflow=196.48 cfs 54.709 af

Kittery_100%Buil	d (ID 2853681) ME-DennettRoad 24-hr S1 100-yr Rainfall=8.52"
Prepared by Barton	& Loguidice, DPC Printed 8/7/2023
HydroCAD® 10.20-2g	s/n 05255 © 2022 HydroCAD Software Solutions LLC Page 34
Pond 3-P:	Peak Elev=86.85' Storage=1.235 af Inflow=16.65 cfs 1.235 af Outflow=0.00 cfs 0.000 af
Pond CB-1-1: L1-1	Peak Elev=1,165.95' Inflow=111.40 cfs 19.791 af 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=111.40 cfs 19.791 af
Pond CB-1-3: L1-3	Peak Elev=56.76' Inflow=170.30 cfs 40.420 af 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=170.30 cfs 40.420 af
Pond CB-1-4: L1-4	Peak Elev=99.67' Inflow=84.39 cfs 11.818 af 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=84.39 cfs 11.818 af
Pond CB-2-3: L2-2	Peak Elev=67.93' Inflow=203.01 cfs 59.602 af 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=203.01 cfs 59.602 af
Pond CB-2-4: L2-3	Peak Elev=38.93' Inflow=41.38 cfs 5.673 af 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=41.38 cfs 5.673 af
Pond CB-3-1: L3-1	Peak Elev=91.28' Inflow=16.91 cfs 3.080 af Outflow=16.91 cfs 3.080 af
Pond CB-3-2: L3-2	Peak Elev=68.03' Inflow=54.60 cfs 9.531 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=54.60 cfs 9.531 af
Pond CB-4-1: L4-1	Peak Elev=58.75' Inflow=29.28 cfs 6.563 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=29.28 cfs 6.563 af
Pond CB-4-2: L4-2	Peak Elev=59.42' Inflow=31.79 cfs 6.610 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=31.79 cfs 6.610 af
Pond CB-5-1: L5-1	Peak Elev=58.56' Inflow=34.91 cfs 6.831 af 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=34.91 cfs 6.831 af
Pond CB-5-2: L5-2	Peak Elev=54.52' Inflow=97.66 cfs 15.515 af 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=97.66 cfs 15.515 af
Pond CB-6-1: L6-1	Peak Elev=49.53' Inflow=0.41 cfs 0.069 af 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.41 cfs 0.069 af
Link L1-5: L1-5	Inflow=169.74 cfs 40.406 af Primary=169.74 cfs 40.406 af
Link L1-6: L1-6	Inflow=249.02 cfs 52.223 af Primary=249.02 cfs 52.223 af
Link L2-1: L2-1	Inflow=196.48 cfs 54.709 af Primary=196.48 cfs 54.709 af
Link L2-4: L2-4	Inflow=202.92 cfs 59.585 af Primary=202.92 cfs 59.585 af

Kittery_100%Build (ID 2853681)	ME-DennettRoad 24	4-hr S1 100-yr Rainfall=8.52"
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Link 1 2 5 1 2 5		Inflow-11 33 cfs 5 673 of
LIIR L2-3. L2-3		Primary=41.33 cfs 5.673 af
LINK L2-6: L2-6		Inflow=209.35 cfs 65.257 af
		Primary=209.35 cfs 65.257 af
Link L4-3: L4-3		Inflow=29.29 cfs 6.563 af
		Primary=29.29 cfs 6.563 af
Link L4-4: L4-4		Inflow=31.79 cfs 6.610 af
		Primary=31.79 cfs 6.610 af
Link   4-5.   4-5		Inflow-60.90 cfs 13.173 af
		Primary=60.90 cfs 13.173 af
link   5 2,   5 2		Inflow-02.61 ofc. 15.515 of
LIIIK LJ-J. LJ-J		Primary-02.61 cfs 15.515 at
		Fillinary=92.01 CIS 15.515 al
Link L6-2: L6-2		Inflow=0.40 cfs 0.069 af
		Primary=0.40 cfs 0.069 af
Total Runoff Area = 323.908 ac	Runoff Volume = 165.498 af	Average Runoff Depth = 6.13"

96.72% Pervious = 313.275 ac 3.28% Impervious = 10.633 ac

Kittery\_100%Build (ID 2853681)ME-DennettRoad 24-hr S1 1-yrExtreme: 1-yr Rainfall=2.63"Prepared by Barton & Loguidice, DPCPrinted 8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 36

Subcatchment 1-1: 1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth=0.90" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=18.87 cfs 3.059 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth=1.13" Flow Length=485' Tc=27.5 min CN=WQ Runoff=14.70 cfs 2.120 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth=1.85" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=28.51 cfs 3.120 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth=1.92" Flow Length=875' Tc=18.9 min CN=WQ Runoff=25.95 cfs 2.963 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth=0.88" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=19.89 cfs 4.964 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth=1.61" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=30.88 cfs 7.075 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth=2.09" Flow Length=850' Tc=25.6 min CN=WQ Runoff=9.58 cfs 1.298 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth=2.08" Flow Length=745' Tc=17.7 min CN=WQ Runoff=13.49 cfs 1.493 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth=0.57" Flow Length=688' Tc=33.2 min CN=WQ Runoff=2.40 cfs 0.399 af
Subcatchment 3-1-P: DA-3-7	I-P Runoff Area=1.790 ac 100.00% Impervious Runoff Depth=2.40" Tc=0.0 min CN=98 Runoff=5.87 cfs 0.358 af
Subcatchment 3-2: 3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth=0.31" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=5.12 cfs 0.673 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth=1.18" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=8.63 cfs 1.547 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth=2.09" Flow Length=955' Tc=37.6 min CN=WQ Runoff=10.32 cfs 1.744 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth=2.08" Flow Length=625' Tc=33.8 min CN=WQ Runoff=11.31 cfs 1.797 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth=2.09" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=27.30 cfs 2.291 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth=2.09" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.13 cfs 0.018 af

Kittery_100%Build (ID 2	2853681) ME-DennettRoad 24-hr S1 1-yr Extreme: 1-yr Rainfall=2.63"
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Reach R1-1: R1-1	Avg. Flow Depth=0.95' Max Vel=3.32 fps Inflow=18.87 cfs 3.059 af n=0.035 L=800.0' S=0.0117 '/' Capacity=90.60 cfs Outflow=18.51 cfs 3.059 af
Reach R1-2: R1-2	Avg. Flow Depth=0.10' Max Vel=1.83 fps Inflow=0.59 cfs 0.941 af n=0.035 L=20.0' S=0.0465 '/' Capacity=180.91 cfs Outflow=0.59 cfs 0.941 af
Reach R1-3: R-1-3	Avg. Flow Depth=1.63' Max Vel=1.81 fps Inflow=37.33 cfs 7.120 af n=0.040 L=225.0' S=0.0022 '/' Capacity=56.74 cfs Outflow=36.99 cfs 7.115 af
Reach R1-4: R1-4	Avg. Flow Depth=0.94' Max Vel=4.74 fps Inflow=25.95 cfs 2.963 af n=0.035 L=290.0' S=0.0241 '/' Capacity=130.35 cfs Outflow=25.84 cfs 2.963 af
Reach R1-5: R1-5	Inflow=36.99 cfs 7.115 af Outflow=36.99 cfs 7.115 af
Reach R2-1: R2-1	Avg. Flow Depth=0.61' Max Vel=2.19 fps Inflow=6.43 cfs 10.108 af n=0.035 L=460.0' S=0.0082 '/' Capacity=76.05 cfs Outflow=6.43 cfs 10.093 af
Reach R2-2: R2-2	Avg. Flow Depth=0.85' Max Vel=4.30 fps Inflow=13.49 cfs 1.493 af n=0.030 L=175.0' S=0.0171 '/' Capacity=82.75 cfs Outflow=13.46 cfs 1.493 af
Reach R2-3: R2-3	Avg. Flow Depth=0.86' Max Vel=3.47 fps Inflow=11.19 cfs 11.390 af n=0.030 L=410.0' S=0.0110 '/' Capacity=66.21 cfs Outflow=11.14 cfs 11.381 af
Reach R3-1: R3-1	Avg. Flow Depth=0.31' Max Vel=2.71 fps Inflow=2.40 cfs 0.399 af n=0.030 L=1,596.0' S=0.0203 '/' Capacity=89.94 cfs Outflow=2.18 cfs 0.399 af
Reach R3-P: R3-P	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.040 L=276.0' S=0.0056 '/' Capacity=3.77 cfs Outflow=0.00 cfs 0.000 af
Reach R4-1: R4-1	Avg. Flow Depth=0.48' Max Vel=6.09 fps Inflow=8.63 cfs 1.547 af n=0.030 L=95.0' S=0.0632 '/' Capacity=158.84 cfs Outflow=8.63 cfs 1.547 af
Reach R4-2: R4-2	Avg. Flow Depth=0.51' Max Vel=6.70 fps Inflow=10.32 cfs 1.744 af n=0.030 L=140.0' S=0.0714 '/' Capacity=168.92 cfs Outflow=10.32 cfs 1.744 af
Reach R5-1: R5-1	Avg. Flow Depth=0.62' Max Vel=3.73 fps Inflow=11.31 cfs 1.797 af n=0.035 L=640.0' S=0.0234 '/' Capacity=325.19 cfs Outflow=11.22 cfs 1.797 af
Reach R5-2: R5-2	Avg. Flow Depth=1.27' Max Vel=3.34 fps Inflow=31.46 cfs 4.088 af n=0.035 L=700.0' S=0.0086 '/' Capacity=77.67 cfs Outflow=28.91 cfs 4.088 af
Reach R6-1: R6-1	Avg. Flow Depth=0.06' Max Vel=1.03 fps Inflow=0.13 cfs 0.018 af n=0.030 L=360.0' S=0.0203 '/' Capacity=90.06 cfs Outflow=0.13 cfs 0.018 af
Pond 1-P:	Peak Elev=38.47' Storage=1.615 af Inflow=14.70 cfs 2.120 af 12.0" Round Culvert n=0.012 L=23.0' S=0.0000 '/' Outflow=0.59 cfs 0.941 af
Pond 2-P:	Peak Elev=37.25' Storage=6.505 af Inflow=50.35 cfs 12.039 af Outflow=6.43 cfs 10.108 af

<b>Kittery_100%Build (II</b> Prepared by Barton & L	<b>2853681)</b> <i>ME-DennettRoad</i> 24-hr S1 1-yr <i>Extreme:</i> 1-yr <i>Rainfall=</i> 2.63" pguidice, DPC Printed 8/7/2023
HydroCAD® 10.20-2g s/ITC	255 © 2022 HydroCAD Sortware Solutions LLC Page 38
Pond 3-P:	Peak Elev=86.25' Storage=0.358 af Inflow=5.87 cfs 0.358 af Outflow=0.00 cfs 0.000 af
Pond CB-1-1: L1-1	Peak Elev=79.53' Inflow=18.87 cfs 3.059 af 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=18.87 cfs 3.059 af
Pond CB-1-3: L1-3	Peak Elev=40.17' Inflow=37.33 cfs 7.120 af 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=37.33 cfs 7.120 af
Pond CB-1-4: L1-4	Peak Elev=41.72' Inflow=25.95 cfs 2.963 af 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=25.95 cfs 2.963 af
Pond CB-2-3: L2-2	Peak Elev=31.29' Inflow=11.19 cfs 11.390 af 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=11.19 cfs 11.390 af
Pond CB-2-4: L2-3	Peak Elev=32.56' Inflow=13.49 cfs 1.493 af 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=13.49 cfs 1.493 af
Pond CB-3-1: L3-1	Peak Elev=87.28' Inflow=2.40 cfs 0.399 af Outflow=2.40 cfs 0.399 af
Pond CB-3-2: L3-2	Peak Elev=55.08' Inflow=6.14 cfs 1.073 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=6.14 cfs 1.073 af
Pond CB-4-1: L4-1	Peak Elev=55.32' Inflow=8.63 cfs 1.547 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=8.63 cfs 1.547 af
Pond CB-4-2: L4-2	Peak Elev=55.48' Inflow=10.32 cfs 1.744 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=10.32 cfs 1.744 af
Pond CB-5-1: L5-1	Peak Elev=57.29' Inflow=11.31 cfs 1.797 af 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=11.31 cfs 1.797 af
Pond CB-5-2: L5-2	Peak Elev=42.70' Inflow=31.46 cfs 4.088 af 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=31.46 cfs 4.088 af
Pond CB-6-1: L6-1	Peak Elev=49.41' Inflow=0.13 cfs 0.018 af 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.13 cfs 0.018 af
Link L1-5: L1-5	Inflow=36.99 cfs 7.115 af Primary=36.99 cfs 7.115 af
Link L1-6: L1-6	Inflow=62.52 cfs 10.079 af Primary=62.52 cfs 10.079 af
Link L2-1: L2-1	Inflow=6.43 cfs 10.108 af Primary=6.43 cfs 10.108 af
Link L2-4: L2-4	Inflow=11.14 cfs 11.381 af Primary=11.14 cfs 11.381 af

Kittery_100%Build (ID 2853681)	ME-DennettRoad 24-hr S1 1-yr	Extreme: 1-yr Rainfall=2.63"
Prepared by Barton & Loguidice, DPC	CAD Software Solutions LLC	Printed 8/7/2023
HydroCAD® 10.20-2g S/1105255 @ 2022 Hydro	CAD Software Solutions LLC	Page 39
Link L2-5: L2-5		Inflow=13.46 cfs 1.493 af
		Primary=13.46 cfs 1.493 af
Link L2-6: L2-6		Inflow=23.22 cfs 12.874 af
		Primary=23.22 cfs 12.874 af
Link L4-3: L4-3		Inflow=8.63 cfs 1.547 af
		Primary=8.63 cfs 1.547 af
Link L4-4: L4-4		Inflow=10.32 cfs 1.744 af
		Primary=10.32 cfs 1.744 af
Link L4-5: L4-5		Inflow=18.91 cfs 3.291 af
		Primary=18.91 cfs 3.291 af
Link L5-3: L5-3		Inflow=28.91 cfs 4.088 af
		Primary=28.91 cfs 4.088 af
Link L6-2: L6-2		Inflow=0.13 cfs 0.018 af
		Primary=0.13 cfs 0.018 af
Total Punoff Area - 323 00	Rac Runoff Volumo - 34 920 af	Average Puneff Depth - 1 20"

Total Runoff Area = 323.908 acRunoff Volume = 34.920 afAverage Runoff Depth = 1.29"96.72% Pervious = 313.275 ac3.28% Impervious = 10.633 ac

Kittery\_100%Build (ID 2853681)ME-DennettRoad 24-hr S1 10-yrExtreme: 10-yr Rainfall=5.32"Prepared by Barton & Loguidice, DPCPrinted 8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 40

Subcatchment 1-1: 1-1	Runoff Area=40.914 ac 5.72% Impervious Runoff Depth=2.97" Flow Length=1,330' Tc=33.2 min CN=WQ Runoff=59.93 cfs 10.129 af
Subcatchment 1-2: DA-1-2	Runoff Area=22.507 ac 1.83% Impervious Runoff Depth=3.35" Flow Length=485' Tc=27.5 min CN=WQ Runoff=40.78 cfs 6.290 af
Subcatchment 1-3: DA-1-3	Runoff Area=20.216 ac 0.00% Impervious Runoff Depth=4.39" Flow Length=1,275' Tc=17.2 min CN=WQ Runoff=59.18 cfs 7.398 af
Subcatchment 1-4: DA-1-4	Runoff Area=18.482 ac 0.00% Impervious Runoff Depth=4.52" Flow Length=875' Tc=18.9 min CN=WQ Runoff=53.04 cfs 6.957 af
Subcatchment 2-1: DA-2-1	Runoff Area=67.852 ac 3.96% Impervious Runoff Depth=2.79" Flow Length=3,230' Tc=68.9 min CN=WQ Runoff=59.94 cfs 15.750 af
Subcatchment 2-2: DA-2-2	Runoff Area=52.573 ac 0.05% Impervious Runoff Depth=3.79" Flow Length=1,550' Tc=62.4 min CN=WQ Runoff=63.87 cfs 16.616 af
Subcatchment 2-3: DA-2-3	Runoff Area=7.453 ac 0.00% Impervious Runoff Depth=4.74" Flow Length=850' Tc=25.6 min CN=WQ Runoff=18.78 cfs 2.942 af
Subcatchment 2-4: DA-2-4	Runoff Area=8.606 ac 0.00% Impervious Runoff Depth=4.73" Flow Length=745' Tc=17.7 min CN=WQ Runoff=26.44 cfs 3.391 af
Subcatchment 3-1: DA-3-1	Runoff Area=8.461 ac 6.06% Impervious Runoff Depth=2.07" Flow Length=688' Tc=33.2 min CN=WQ Runoff=8.38 cfs 1.462 af
Subcatchment 3-1-P: DA-3-1	I-P Runoff Area=1.790 ac 100.00% Impervious Runoff Depth=5.08" Tc=0.0 min CN=98 Runoff=10.82 cfs 0.758 af
Subcatchment 3-2: 3-2	Runoff Area=25.718 ac 10.45% Impervious Runoff Depth=1.29" Flow Length=1,578' Tc=19.9 min CN=WQ Runoff=20.07 cfs 2.759 af
Subcatchment 4-1: DA-4-1	Runoff Area=15.689 ac 1.12% Impervious Runoff Depth=2.81" Flow Length=1,170' Tc=41.1 min CN=WQ Runoff=17.92 cfs 3.680 af
Subcatchment 4-2: DA-4-2	Runoff Area=10.016 ac 0.00% Impervious Runoff Depth=4.74" Flow Length=955' Tc=37.6 min CN=WQ Runoff=20.29 cfs 3.954 af
Subcatchment 5-1: DA-5-1	Runoff Area=10.367 ac 0.00% Impervious Runoff Depth=4.73" Flow Length=625' Tc=33.8 min CN=WQ Runoff=22.27 cfs 4.082 af
Subcatchment 5-2: DA-5-2	Runoff Area=13.159 ac 0.00% Impervious Runoff Depth=4.74" Flow Length=1,025' Tc=10.3 min CN=WQ Runoff=53.33 cfs 5.194 af
Subcatchment 6-1: DA-6	Runoff Area=0.105 ac 0.00% Impervious Runoff Depth=4.74" Flow Length=135' Slope=0.0150 '/' Tc=25.6 min CN=WQ Runoff=0.26 cfs 0.041 af

Kittery_100%Build (ID 2853681)ME-DennettRoad 24-hr S1 10-yrExtreme: 10-yr Rainfall=5.32"Prepared by Barton & Loguidice, DPCPrinted 8/7/2023HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLCPage 41							
Reach R1-1: R1-1	Avg. Flow Depth=1.65' Max Vel=4.52 fps Inflow=59.93 cfs 10.129 af n=0.035 L=800.0' S=0.0117 '/' Capacity=90.60 cfs Outflow=59.37 cfs 10.129 af						
Reach R1-2: R1-2	Avg. Flow Depth=0.22' Max Vel=2.92 fps Inflow=2.30 cfs 3.848 af n=0.035 L=20.0' S=0.0465 '/' Capacity=180.91 cfs Outflow=2.30 cfs 3.848 af						
Reach R1-3: R-1-3	Avg. Flow Depth=2.65' Max Vel=2.27 fps Inflow=96.07 cfs 21.375 af n=0.040 L=225.0' S=0.0022 '/' Capacity=56.74 cfs Outflow=95.72 cfs 21.365 af						
Reach R1-4: R1-4	Avg. Flow Depth=1.32' Max Vel=5.73 fps Inflow=53.04 cfs 6.957 af n=0.035 L=290.0' S=0.0241 '/' Capacity=130.35 cfs Outflow=52.88 cfs 6.957 af						
Reach R1-5: R1-5	Inflow=95.72 cfs 21.365 af Outflow=95.72 cfs 21.365 af						
Reach R2-1: R2-1	Avg. Flow Depth=1.97' Max Vel=4.19 fps Inflow=73.57 cfs 28.009 af n=0.035 L=460.0' S=0.0082 '/' Capacity=76.05 cfs Outflow=73.49 cfs 27.984 af						
Reach R2-2: R2-2	Avg. Flow Depth=1.18' Max Vel=5.14 fps Inflow=26.44 cfs 3.391 af n=0.030 L=175.0' S=0.0171 '/' Capacity=82.75 cfs Outflow=26.39 cfs 3.391 af						
Reach R2-3: R2-3	Avg. Flow Depth=2.14' Max Vel=5.71 fps Inflow=76.30 cfs 30.926 af n=0.030 L=410.0' S=0.0110 '/' Capacity=66.21 cfs Outflow=76.27 cfs 30.910 af						
Reach R3-1: R3-1	Avg. Flow Depth=0.62' Max Vel=3.96 fps Inflow=8.38 cfs 1.462 af n=0.030 L=1,596.0' S=0.0203 '/' Capacity=89.94 cfs Outflow=7.98 cfs 1.462 af						
Reach R3-P: R3-P	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.040 L=276.0' S=0.0056 '/' Capacity=3.77 cfs Outflow=0.00 cfs 0.000 af						
Reach R4-1: R4-1	Avg. Flow Depth=0.70' Max Vel=7.48 fps Inflow=17.92 cfs 3.680 af n=0.030 L=95.0' S=0.0632 '/' Capacity=158.84 cfs Outflow=17.93 cfs 3.680 af						
Reach R4-2: R4-2	Avg. Flow Depth=0.73' Max Vel=8.09 fps Inflow=20.29 cfs 3.954 af n=0.030 L=140.0' S=0.0714 '/' Capacity=168.92 cfs Outflow=20.29 cfs 3.954 af						
Reach R5-1: R5-1	Avg. Flow Depth=0.87' Max Vel=4.50 fps Inflow=22.27 cfs 4.082 af n=0.035 L=640.0' S=0.0234 '/' Capacity=325.19 cfs Outflow=22.15 cfs 4.082 af						
Reach R5-2: R5-2	Avg. Flow Depth=1.76' Max Vel=4.01 fps Inflow=62.19 cfs 9.277 af n=0.035 L=700.0' S=0.0086 '/' Capacity=77.67 cfs Outflow=58.35 cfs 9.277 af						
Reach R6-1: R6-1	Avg. Flow Depth=0.09' Max Vel=1.33 fps Inflow=0.26 cfs 0.041 af n=0.030 L=360.0' S=0.0203 '/' Capacity=90.06 cfs Outflow=0.26 cfs 0.041 af						
Pond 1-P:	Peak Elev=39.23' Storage=4.171 af Inflow=40.78 cfs 6.290 af 12.0" Round Culvert n=0.012 L=23.0' S=0.0000 '/' Outflow=2.30 cfs 3.848 af						
Pond 2-P:	Peak Elev=38.29' Storage=12.565 af Inflow=123.32 cfs 32.366 af Outflow=73.57 cfs 28.009 af						

Kittery_100%Build (ID 2) Prepared by Barton & Logi HydroCAD® 10 20-20, s/p 0525	<b>853681)</b> <i>ME-DennettRoad 24-hr S1 10-yr Extreme: 10-yr Rainfall=5.32"</i> uidice, DPC Printed 8/7/2023
TyulooAD@ 10.20 2g 3/10320	
Pond 3-P:	Peak Elev=86.52' Storage=0.758 af Inflow=10.82 cfs 0.758 af Outflow=0.00 cfs 0.000 af
Pond CB-1-1: L1-1	Peak Elev=371.42' Inflow=59.93 cfs 10.129 af 14.0" Round Culvert n=0.025 L=50.0' S=0.0148 '/' Outflow=59.93 cfs 10.129 af
Pond CB-1-3: L1-3	Peak Elev=44.93' Inflow=96.07 cfs 21.375 af 36.0" Round Culvert n=0.012 L=95.0' S=0.0019 '/' Outflow=96.07 cfs 21.375 af
Pond CB-1-4: L1-4	Peak Elev=60.55' Inflow=53.04 cfs 6.957 af 24.0" Round Culvert n=0.025 L=84.0' S=0.0357 '/' Outflow=53.04 cfs 6.957 af
Pond CB-2-3: L2-2	Peak Elev=36.53' Inflow=76.30 cfs 30.926 af 36.0" Round Culvert n=0.012 L=150.0' S=0.0100 '/' Outflow=76.30 cfs 30.926 af
Pond CB-2-4: L2-3	Peak Elev=34.53' Inflow=26.44 cfs 3.391 af 24.0" Round Culvert n=0.012 L=75.0' S=0.0000 '/' Outflow=26.44 cfs 3.391 af
Pond CB-3-1: L3-1	Peak Elev=88.29' Inflow=8.38 cfs 1.462 af Outflow=8.38 cfs 1.462 af
Pond CB-3-2: L3-2	Peak Elev=57.68' Inflow=24.79 cfs 4.221 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=24.79 cfs 4.221 af
Pond CB-4-1: L4-1	Peak Elev=56.40' Inflow=17.92 cfs 3.680 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=17.92 cfs 3.680 af
Pond CB-4-2: L4-2	Peak Elev=56.80' Inflow=20.29 cfs 3.954 af 24.0" Round Culvert n=0.012 L=100.0' S=0.0200 '/' Outflow=20.29 cfs 3.954 af
Pond CB-5-1: L5-1	Peak Elev=57.91' Inflow=22.27 cfs 4.082 af 36.0" Round Culvert n=0.012 L=70.0' S=0.0143 '/' Outflow=22.27 cfs 4.082 af
Pond CB-5-2: L5-2	Peak Elev=46.68' Inflow=62.19 cfs 9.277 af 32.0" Round Culvert n=0.012 L=217.0' S=0.0276 '/' Outflow=62.19 cfs 9.277 af
Pond CB-6-1: L6-1	Peak Elev=49.47' Inflow=0.26 cfs 0.041 af 18.0" Round Culvert n=0.012 L=95.0' S=0.0309 '/' Outflow=0.26 cfs 0.041 af
Link L1-5: L1-5	Inflow=95.72 cfs 21.365 af Primary=95.72 cfs 21.365 af
Link L1-6: L1-6	Inflow=146.48 cfs 28.323 af Primary=146.48 cfs 28.323 af
Link L2-1: L2-1	Inflow=73.57 cfs 28.009 af Primary=73.57 cfs 28.009 af
Link L2-4: L2-4	Inflow=76.27 cfs 30.910 af Primary=76.27 cfs 30.910 af

Kittery_100%Build (ID 2853681) ME-DennettRoad 24-hr S1 10-yr	Extreme: 10-yr Rainfall=5.32"
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HydroCAD® 10.20-2g s/n 05255 © 2022 HydroCAD Software Solutions LLC	Page 43
	-
Link L2-5: L2-5	Inflow=26.39 cfs 3.391 af
	Primary=26.39 cfs 3.391 af
Link L2-6: L2-6	Inflow=79.25 cfs 34.301 af
	Primary=79.25 cfs 34.301 af
Link L4-3: L4-3	Inflow=17.93 cfs 3.680 af
	Primary=17.93 cfs 3.680 af
Link L4-4: L4-4	Inflow=20.29 cfs 3.954 af
	Primary=20.29 cfs 3.954 af
Link L4-5: L4-5	Inflow=38.11 cfs 7.633 af
	Primary=38.11 cfs 7.633 af
Link L5-3: L5-3	Inflow=58.35 cfs 9.277 af
	Primary=58.35 cfs 9.277 af
Link L6-2: L6-2	Inflow=0.26 cfs 0.041 af
	Primary=0.26 cfs 0.041 af
Total During Manage 200,000 and During Malaring	Average Durn off Double 2 201

Total Runoff Area = 323.908 acRunoff Volume = 91.403 afAverage Runoff Depth = 3.39"96.72% Pervious = 313.275 ac3.28% Impervious = 10.633 ac

Appendix J

Extreme Storm Event HydroCAD Summary Report

Appendix K

Stormwater Opportunity Ranking Matrix

			40	5	5	10	5	5	5	15	2	2	2	2	2	50	20	20	10	100
			Stormwater Benefits		Constructability			Cost		Co-Benefits										
Subcatchment	Project Type	Location	Water Quantity / Flood Reduction	TSS Reduction	Nutrient Reduction	Land Acquisition or Public Partnership Potential	Known Constraints (utilities, depth to groundwater, site access, soils)	Permitting	Maintenance	Fundability	Energy and Air Quality Impacts	Habitat & Biodiversity	Community & Aesthetic Benefits	Human Health Benefits	Educational Opportunities/ Visibility	Stormwater Benefits Total	Constructability Total	Cost Total	Co-Benefits Total	Project Ranking Tota
DA-2-Pond	Expanding Stormwater Storage	98 Dennett Parcel	40	5	5	5	5	1	3	15	2	2	2	2	2	50	11	18	10	89
DA-4-1, DA-4-2	Expanding Stormwater Storage	East side of Dennett Road	40	5	5	5	3	1	3	15	2	2	2	2	2	50	9	18	10	87
DA-1-1	Upstream Detention/Wetland Expansion	Above Martin Rd.	40	5	5	5	1	1	1	15	2	2	2	2	2	50	7	16	10	83
AII	Low-Impact Development Considerations	Areas currently zoned B-PK and C- 2	20	5	5	10	5	5	5	10	2	2	2	2	2	30	20	15	10	75
Multiple	Sizing of Critical Infrastructure - Culvert and Drainage Modifications	Martin Rd., Rt-236, Dennett and Old Dennett Rds., I-95	30	0	0	10	3	3	5	10	0	0	2	2	1	30	16	15	5	66
DA-2-1, DA-2-2, DA-2- 4, DA-4-1, DA-4-2, DA- 1-2, DA-1-4	Land Conservation	Areas currently zoned B-PK and C- 2	20	5	5	5	5	5	5	5	2	2	2	2	2	30	15	10	10	65
DA-1-2, DA-1-3	Wetland Restoration	41 Rt. 236	10	5	5	5	3	1	1	15	2	2	2	2	2	20	9	16	10	55
All Residential	Homeowner Floodproofing	All Residential	10	0	0	10	5	5	5	5	2	2	2	2	2	10	20	10	10	50
DA-3-1; DA-2-1, DA-1-1	Installation of Sewer Line Seep Collars	Martin Rd.	10	0	0	10	3	5	5	5	0	0	0	1	0	10	18	10	1	39
DA-3	Drainage Infrastructure Modifications	Roseberry Lane	10	0	0	10	3	3	5	5	0	0	0	1	0	10	16	10	1	37
DA-3-1	Culvert Modifications	Old Dennett Road	10	0	0	10	3	3	5	5	0	0	0	1	0	10	16	10	1	37
DA-3-1	Access Road Culvert Improvements	Happy Avenue	10	0	0	5	3	5	5	5	0	0	0	1	0	10	13	10	1	34
DA-2-1	Drainage Improvements	Summer Lane	10	0	0	5	3	3	5	5	0	0	0	1	0	10	11	10	1	32
															Appenidx					

Notes - Initiall construction/implementation cost not included in priority ranking. Intent is to develop projects with a varying range of costs. Projects in BOLD represent the 6 projects selected for further evaluation as part of Engineering Study Report. Recommendations for Homeowner Floodproofing are also provided in the Report.

Stormwater Benefits:

 negligible reduction in peak flow.
addresses lot level localized flooding or potenitally minimizes groundwater intrusion
assists with off-setting potential drainage issues associated with future development Quantity: 30 - addresses localized flooding (road/culvert overtopping) or GI practice that promotes infiltration or impervious reduction 1,000 - 100,000 sf 40 - creation of stormwater attenuation or impervious reduction over 100,000 sf TSS & Nutrients: 0 - negligibile benefit 5 - water quality benefit

Constructability: Ownership

Known Constraints

1 - Constraints identified 3 - Possible constraints identified 5 - No constraints identified

0 - uninterested private owner 5 - interested private owner or unknown interest level private owner 10 - public

Permitting

 Multiple permits required (MDEP, ACOE, Local ROW, etc.) and Project is located on Private Property
Multiple permits required (MDEP, ACOE, Local ROW, etc.) and Project is located on Public Property 5 - Low permitting demand anticipated

Maintenance 1 - Hiigh \$ 3 - Medium \$ 5 - Low \$

Cost:

Fundability 5 - not fundable through existing stormwater management and flooding prevention grants 10 - Grant assistance possible 15 - Grant assistance likely

Co-Benefits modified from "The Value of Green Infrastructure: A Guide to Recognizing its Economic, Environmental, and Social Benefits," Center for Neighborhood Technology and American Rivers, 2010 and "Green Infrastructure Practices and Benefits", National Oceanic and Atmospheric Administration, 2014"

Co-benefits on a scale from 0 (no benefit) to 2 (significant benefit) Energy and Air Quality Impacts includes: energy use reduction, air quality improvements and atmospheric C02 reduction Habitat and Biodiversity includes: increases biodiversity, increases habitat connectivity, and provides pollinator habitat Community and Aesthetic Benefits includes: improved aesthetics, increased recreational opportunities, and increased property values Human health benefits includes health benefits and accident reduction

Appendix L

Herb Parsons Pond Evaluation

## Assessment of Groundwater Conditions Proximal to Herb Parson Pond, Kittery, ME

Streamworks

27 January 2023

**Objective:** Assess whether Herb Parson Pond along Martin Road in Kittery, ME creates high groundwater resulting in nearby basement flooding.

**Result of Assessment**: Herb Parsons Pond acts like a large diameter groundwater well: the pond surface being the groundwater elevation. Because the Pond has very little surface water drainage area, the Pond water surface elevation simply bears witness to local groundwater conditions and does not control that condition.

A site inspection was conducted January 14, 2023. 90% of the Pond perimeter was walked as well as Martin Road ¼ mile either side of the Pond. As evidenced in Figures 1 and 2, the pond water level was low at both times photos were taken as evidenced by the noticeable bank scarp at the high-water line where woody vegetation starts.



Figure 1. Herb Parson Pond Looking North 14 January 2023



Figure 2. Herb Parson Pond Looking East Fall 202s (photo by B&L).

Figure 3 depicts the local topography from a USGS Quadrangle map (Portsmouth, NH 7.5minute Quadrangle 2021 with imagery from 2015-2016). Martin Road runs East-West along a topographic high (possibly an esker or kame deposit) and the pond resides just left of the center of the figure. Elevation drops off quickly to the north (towards Spinney Creek) and gently to the south. Notice that the eastern extension of Happy Avenue does not appear on this map.

Walking the perimeter of the pond displayed no streams or storm drains flowing into or out of the pond. The pond watershed area is not much larger than the pond surface area itself, most-likely due to two factors: setting and excavation. The pond setting is near a local topographic high and the pond watershed appears as though it is a relic gravel pit with the pond in the center where gravel excavation proceeded below the water table. From the USDA soil survey, the pond is identified as a gravel pit (Appendix).

Because of the very small watershed area, which possesses very little impervious cover, the pond water surface is simply a reflection of the local groundwater table. It does appear that at least one basement sump pump delivers water to the pond, but this would be insufficient to
dramatically increase the pond water level, and essentially a zero-sum issue: groundwater is pumped from one spot and moved to another. There are some rooftops that also drain into the pond. This runoff could result in measurable (but very small) increases in the pond level, but such an increase would become undetectable after say one hundred feet away.



Figure 3. USGS Topographic map in the Vicinity of Herb Parson Pond

From the Soils report, the pond is in the Colton gravelly sandy loam soil type which possesses a high infiltration rate. As such any road or rooftop runoff from Martin Road or nearby houses to the pond, that is directed to the pond, would not dramatically effect pond hydrology because in the absence of those impervious covers, precipitation would have infiltrated to groundwater in the high permeability Colton soil.

A very interesting feature of the Soils Report is that south of the pond, and along Happy Avenue, the soil is listed as "water". It did not appear as water at the time of the site visit or from aerial imagery. Earlier USGS maps indicated the wetland symbol at the eastern portion of Happy Avenue.

Many of the houses along Martin Road appear over 50 years old (many appear on the oldest Google Earth image of 1992). Some of these older homes were built with their first floors above grade and have raised bed septic systems, as in Figure 5. Many of these homes appear to have basements. Not all homes inspected during the field visit had rooftop gutters and downspouts to move roof runoff away from the house foundation (a very common cause for basement wetness and flooding).



Figure 5. Evidence of raising local terrain to accommodate infrastructure.

The weight of evidence from the site visit is that Herb Parson Pond does not cause high groundwater levels in its vicinity, but rather witnesses the groundwater condition.

Appendix

Soils Report from Web Soil Survey



United States Department of Agriculture

Natural Resources Conservation

Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

# Custom Soil Resource Report for York County, Maine



## Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/? cid=nrcs142p2\_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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## **How Soil Surveys Are Made**

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

## Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.



	MAP LEGEND			MAP INFORMATION
Area of In	terest (AOI) Area of Interest (AOI)	8	Spoil Area Stony Spot	The soil surveys that comprise your AOI were mapped at 1:20,000.
Special	Soil Map Unit Polygons Soil Map Unit Lines Soil Map Unit Points <b>Point Features</b> Blowout	Ø3 ∜ ∽ Water Fea	Wet Spot Other Special Line Features	Warning: Soil Map may not be valid at this scale. Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.
© ₩ ☆ ₩	Borrow Pit Clay Spot Closed Depression Gravel Pit Gravelly Spot	Transport	Streams and Canals tation Rails Interstate Highways US Routes Major Roads	Please rely on the bar scale on each map sheet for map measurements. Source of Map: Natural Resources Conservation Service Web Soil Survey URL: Coordinate System: Web Mercator (EPSG:3857)
0 人 业	Landfill Lava Flow Marsh or swamp Mine or Quarry	Local Roads  Background  Aerial Photography		Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.
◎ ○ + ∵	Miscellaneous Water Perennial Water Rock Outcrop Saline Spot Sandy Spot			This product is generated from the USDA-NRCS certified data as of the version date(s) listed below. Soil Survey Area: York County, Maine Survey Area Data: Version 21, Aug 30, 2022 Soil map units are labeled (as space allows) for map scales
⇔ ♦ ≶	Severely Eroded Spot Sinkhole Slide or Slip Sodic Spot			The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Мар	Unit	Legend
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Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
AgB	Adams-Urban land complex, 0 to 8 percent slopes	6.2	10.8%
СоВ	Colton gravelly sandy loam, 0 to 8 percent slopes	14.1	24.8%
СоD	Colton gravelly sandy loam, 15 to 25 percent slopes	2.9	5.2%
CrB	Croghan loamy fine sand, 0 to 8 percent slopes, wooded	0.6	1.0%
LnB	Lyman loam, 3 to 8 percent slopes, rocky	2.8	5.0%
LnC	Lyman loam, 8 to 15 percent slopes, rocky	2.6	4.6%
Pg	Pits, gravel	7.0	12.3%
SkB	Skerry fine sandy loam, 0 to 8 percent slopes	3.2	5.7%
W	Water bodies	17.5	30.7%
Totals for Area of Interest		56.9	100.0%

## **Map Unit Descriptions**

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas

are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

## York County, Maine

## AgB—Adams-Urban land complex, 0 to 8 percent slopes

#### **Map Unit Setting**

National map unit symbol: 2wqnv Elevation: 490 to 1,310 feet Mean annual precipitation: 36 to 65 inches Mean annual air temperature: 28 to 52 degrees F Frost-free period: 110 to 160 days Farmland classification: Not prime farmland

#### **Map Unit Composition**

Adams and similar soils: 45 percent Urban land: 40 percent Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Adams**

#### Setting

Landform: Outwash deltas Landform position (two-dimensional): Summit, backslope Landform position (three-dimensional): Side slope, crest Down-slope shape: Convex Across-slope shape: Linear Parent material: Sandy glaciofluvial deposits

#### **Typical profile**

*Ap* - 0 to 7 inches: loamy sand *Bs* - 7 to 21 inches: sand *BC* - 21 to 27 inches: sand *C* - 27 to 65 inches: sand

#### Properties and qualities

Slope: 0 to 8 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Somewhat excessively drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (1.42 to 14.17 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Low (about 3.6 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 3s Hydrologic Soil Group: A Ecological site: F144BY601ME - Dry Sand Hydric soil rating: No

#### **Description of Urban Land**

#### Setting

Landform: Outwash deltas

Landform position (two-dimensional): Summit, backslope Landform position (three-dimensional): Side slope, crest Down-slope shape: Linear Across-slope shape: Linear

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 8s Hydric soil rating: Unranked

### CoB—Colton gravelly sandy loam, 0 to 8 percent slopes

#### Map Unit Setting

National map unit symbol: 2ym4k Elevation: 10 to 2,000 feet Mean annual precipitation: 31 to 65 inches Mean annual air temperature: 36 to 52 degrees F Frost-free period: 90 to 160 days Farmland classification: Farmland of statewide importance

#### Map Unit Composition

*Colton and similar soils:* 85 percent *Estimates are based on observations, descriptions, and transects of the mapunit.* 

#### **Description of Colton**

#### Setting

Landform: Kames, eskers Landform position (two-dimensional): Summit, backslope Landform position (three-dimensional): Side slope, crest Down-slope shape: Convex Across-slope shape: Convex Parent material: Sandy-skeletal glaciofluvial deposits

#### **Typical profile**

*Oe - 0 to 4 inches:* moderately decomposed plant material *E - 4 to 6 inches:* gravelly sandy loam *Bs - 6 to 14 inches:* gravelly loamy sand *BC - 14 to 24 inches:* very gravelly coarse sand *C - 24 to 65 inches:* extremely gravelly coarse sand

#### **Properties and qualities**

Slope: 0 to 8 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Excessively drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (1.42 to 14.17 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)

Available water supply, 0 to 60 inches: Very low (about 2.9 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 3s Hydrologic Soil Group: A Ecological site: F144BY601ME - Dry Sand Hydric soil rating: No

## CoD—Colton gravelly sandy loam, 15 to 25 percent slopes

#### Map Unit Setting

National map unit symbol: 2yjfr Elevation: 540 to 2,000 feet Mean annual precipitation: 31 to 95 inches Mean annual air temperature: 36 to 52 degrees F Frost-free period: 90 to 145 days Farmland classification: Not prime farmland

#### Map Unit Composition

*Colton and similar soils:* 85 percent *Estimates are based on observations, descriptions, and transects of the mapunit.* 

#### **Description of Colton**

#### Setting

Landform: Kames, eskers Landform position (two-dimensional): Summit, backslope Landform position (three-dimensional): Side slope, crest Down-slope shape: Convex Across-slope shape: Convex Parent material: Sandy-skeletal glaciofluvial deposits

#### **Typical profile**

*Oe - 0 to 4 inches:* moderately decomposed plant material *E - 4 to 6 inches:* gravelly sandy loam *Bs - 6 to 14 inches:* gravelly loamy sand *BC - 14 to 24 inches:* very gravelly coarse sand *C - 24 to 65 inches:* extremely gravelly coarse sand

#### **Properties and qualities**

Slope: 15 to 25 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Excessively drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (1.42 to 14.17 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Very low (about 2.9 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 6s Hydrologic Soil Group: A Ecological site: F144BY601ME - Dry Sand Hydric soil rating: No

### CrB—Croghan loamy fine sand, 0 to 8 percent slopes, wooded

#### Map Unit Setting

National map unit symbol: 2wqp0 Elevation: 150 to 2,300 feet Mean annual precipitation: 40 to 55 inches Mean annual air temperature: 37 to 46 degrees F Frost-free period: 90 to 135 days Farmland classification: Farmland of statewide importance

#### Map Unit Composition

*Croghan and similar soils:* 85 percent *Estimates are based on observations, descriptions, and transects of the mapunit.* 

#### **Description of Croghan**

#### Setting

Landform: Marine terraces, outwash deltas Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope, base slope Down-slope shape: Linear Across-slope shape: Convex Parent material: Sandy glaciofluvial deposits

#### **Typical profile**

*Oa - 0 to 4 inches:* highly decomposed plant material *E - 4 to 6 inches:* loamy fine sand *Bs - 6 to 17 inches:* loamy fine sand *BC - 17 to 30 inches:* fine sand *C - 30 to 65 inches:* sand

#### **Properties and qualities**

Slope: 0 to 8 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Moderately well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (1.42 to 14.17 in/hr)
Depth to water table: About 18 to 30 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water supply, 0 to 60 inches: Low (about 4.1 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2w Hydrologic Soil Group: A Ecological site: F144BY602ME - Sandy Toeslope Hydric soil rating: No

## LnB—Lyman loam, 3 to 8 percent slopes, rocky

#### Map Unit Setting

National map unit symbol: 2trq7 Elevation: 0 to 520 feet Mean annual precipitation: 36 to 65 inches Mean annual air temperature: 36 to 52 degrees F Frost-free period: 60 to 160 days Farmland classification: Farmland of statewide importance

#### Map Unit Composition

*Lyman, rocky, and similar soils:* 86 percent *Estimates are based on observations, descriptions, and transects of the mapunit.* 

#### **Description of Lyman, Rocky**

#### Setting

Landform: Hills, mountains

Landform position (two-dimensional): Summit, shoulder, backslope Landform position (three-dimensional): Mountaintop, mountainbase, side slope, crest

*Down-slope shape:* Convex

Across-slope shape: Convex

*Parent material:* Loamy supraglacial till derived from granite and gneiss and/or loamy supraglacial till derived from phyllite and/or loamy supraglacial till derived from mica schist

### **Typical profile**

Oe - 0 to 1 inches: moderately decomposed plant material

A - 1 to 3 inches: loam

*E* - 3 to 5 inches: fine sandy loam

Bhs - 5 to 7 inches: loam

Bs1 - 7 to 11 inches: loam

Bs2 - 11 to 18 inches: channery loam

R - 18 to 28 inches: bedrock

### **Properties and qualities**

Slope: 3 to 8 percent
Depth to restrictive feature: 11 to 24 inches to lithic bedrock
Drainage class: Somewhat excessively drained
Capacity of the most limiting layer to transmit water (Ksat): Very low to high (0.00 to 14.03 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Low (about 3.4 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2s Hydrologic Soil Group: D Hydric soil rating: No

## LnC—Lyman loam, 8 to 15 percent slopes, rocky

#### Map Unit Setting

National map unit symbol: 2trq9 Elevation: 0 to 690 feet Mean annual precipitation: 36 to 65 inches Mean annual air temperature: 36 to 52 degrees F Frost-free period: 60 to 160 days Farmland classification: Not prime farmland

#### **Map Unit Composition**

*Lyman, rocky, and similar soils:* 86 percent *Estimates are based on observations, descriptions, and transects of the mapunit.* 

#### Description of Lyman, Rocky

#### Setting

Landform: Hills, mountains Landform position (two-dimensional): Summit, shoulder, backslope Landform position (three-dimensional): Mountaintop, mountainflank,

mountainbase, side slope, crest

Down-slope shape: Convex

Across-slope shape: Convex

*Parent material:* Loamy supraglacial till derived from granite and gneiss and/or loamy supraglacial till derived from phyllite and/or loamy supraglacial till derived from mica schist

#### **Typical profile**

Oe - 0 to 1 inches: moderately decomposed plant material

A - 1 to 3 inches: loam

*E* - 3 to 5 inches: fine sandy loam

Bhs - 5 to 7 inches: loam

Bs1 - 7 to 11 inches: loam

Bs2 - 11 to 18 inches: channery loam

R - 18 to 28 inches: bedrock

#### **Properties and qualities**

Slope: 8 to 15 percent
Depth to restrictive feature: 11 to 24 inches to lithic bedrock
Drainage class: Somewhat excessively drained
Capacity of the most limiting layer to transmit water (Ksat): Very low to high (0.00 to 14.03 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None

Available water supply, 0 to 60 inches: Low (about 3.4 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 3e Hydrologic Soil Group: D Hydric soil rating: No

## Pg—Pits, gravel

#### Map Unit Composition

*Pits:* 88 percent *Estimates are based on observations, descriptions, and transects of the mapunit.* 

#### **Description of Pits**

#### Setting

Landform position (two-dimensional): Summit Landform position (three-dimensional): Tread Down-slope shape: Linear Across-slope shape: Linear

#### **Typical profile**

*H1 - 0 to 6 inches:* extremely gravelly sand *H2 - 6 to 60 inches:* extremely gravelly sand

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 8s Hydric soil rating: No

## SkB—Skerry fine sandy loam, 0 to 8 percent slopes

#### Map Unit Setting

National map unit symbol: 2w9pg Elevation: 160 to 750 feet Mean annual precipitation: 36 to 65 inches Mean annual air temperature: 36 to 52 degrees F Frost-free period: 90 to 160 days Farmland classification: All areas are prime farmland

#### **Map Unit Composition**

*Skerry and similar soils:* 85 percent *Estimates are based on observations, descriptions, and transects of the mapunit.* 

#### **Description of Skerry**

#### Setting

Landform: Mountains, hills Landform position (two-dimensional): Backslope, footslope Landform position (three-dimensional): Mountainbase, interfluve Down-slope shape: Convex Across-slope shape: Linear

*Parent material:* Loamy lodgment till derived from granite and gneiss and/or schist over sandy lodgment till derived from granite and gneiss and/or schist

#### **Typical profile**

Ap - 0 to 6 inches: fine sandy loam Bs1 - 6 to 20 inches: gravelly fine sandy loam Bs2 - 20 to 25 inches: gravelly fine sandy loam Cd1 - 25 to 34 inches: gravelly loamy sand Cd2 - 34 to 65 inches: gravelly loamy sand

#### **Properties and qualities**

Slope: 0 to 8 percent
Depth to restrictive feature: 21 to 43 inches to densic material
Drainage class: Moderately well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.01 to 1.42 in/hr)
Depth to water table: About 18 to 30 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Low (about 3.9 inches)

### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2w Hydrologic Soil Group: C/D Ecological site: F144BY501ME - Loamy Slope (Northern Hardwoods) Hydric soil rating: No

## W—Water bodies

#### Map Unit Composition

*Water:* 100 percent *Estimates are based on observations, descriptions, and transects of the mapunit.* 

#### **Description of Water**

Setting

Landform: Hills

## References

American Association of State Highway and Transportation Officials (AASHTO). 2004. Standard specifications for transportation materials and methods of sampling and testing. 24th edition.

American Society for Testing and Materials (ASTM). 2005. Standard classification of soils for engineering purposes. ASTM Standard D2487-00.

Cowardin, L.M., V. Carter, F.C. Golet, and E.T. LaRoe. 1979. Classification of wetlands and deep-water habitats of the United States. U.S. Fish and Wildlife Service FWS/OBS-79/31.

Federal Register. July 13, 1994. Changes in hydric soils of the United States.

Federal Register. September 18, 2002. Hydric soils of the United States.

Hurt, G.W., and L.M. Vasilas, editors. Version 6.0, 2006. Field indicators of hydric soils in the United States.

National Research Council. 1995. Wetlands: Characteristics and boundaries.

Soil Survey Division Staff. 1993. Soil survey manual. Soil Conservation Service. U.S. Department of Agriculture Handbook 18. http://www.nrcs.usda.gov/wps/portal/ nrcs/detail/national/soils/?cid=nrcs142p2\_054262

Soil Survey Staff. 1999. Soil taxonomy: A basic system of soil classification for making and interpreting soil surveys. 2nd edition. Natural Resources Conservation Service, U.S. Department of Agriculture Handbook 436. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2\_053577

Soil Survey Staff. 2010. Keys to soil taxonomy. 11th edition. U.S. Department of Agriculture, Natural Resources Conservation Service. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2 053580

Tiner, R.W., Jr. 1985. Wetlands of Delaware. U.S. Fish and Wildlife Service and Delaware Department of Natural Resources and Environmental Control, Wetlands Section.

United States Army Corps of Engineers, Environmental Laboratory. 1987. Corps of Engineers wetlands delineation manual. Waterways Experiment Station Technical Report Y-87-1.

United States Department of Agriculture, Natural Resources Conservation Service. National forestry manual. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/ home/?cid=nrcs142p2 053374

United States Department of Agriculture, Natural Resources Conservation Service. National range and pasture handbook. http://www.nrcs.usda.gov/wps/portal/nrcs/ detail/national/landuse/rangepasture/?cid=stelprdb1043084

United States Department of Agriculture, Natural Resources Conservation Service. National soil survey handbook, title 430-VI. http://www.nrcs.usda.gov/wps/portal/ nrcs/detail/soils/scientists/?cid=nrcs142p2\_054242

United States Department of Agriculture, Natural Resources Conservation Service. 2006. Land resource regions and major land resource areas of the United States, the Caribbean, and the Pacific Basin. U.S. Department of Agriculture Handbook 296. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/? cid=nrcs142p2\_053624

United States Department of Agriculture, Soil Conservation Service. 1961. Land capability classification. U.S. Department of Agriculture Handbook 210. http://www.nrcs.usda.gov/Internet/FSE\_DOCUMENTS/nrcs142p2\_052290.pdf

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