

Mr. Adam Causey, Director of Planning & Development Town of Kittery, Maine 200 Rogers Road Kittery, Maine 03904 August 4th, 2022 Project No. C277-22

RE: Preliminary Site Plan Review – Town Review Revisions Well Field 44 Cannabis Dispensary (Tax Map 29, Lot 1) 41 Route 236, Kittery, Maine

Dear Mr. Causey:

On behalf of Well Field 44, LLC., I have enclosed for your review and consideration a revised Plan Set and associated attachments for the above-referenced project. Revisions have been made to address comments presented in the July 14th Planning Board meeting.

• After the July 14th Planning Board meeting, the Applicant pursued a more detailed parking calculation for the proposed development that was to include the parking on the abutting Black Bird Business Suites property that this development was proposing to avail itself of for overflow purposes. Attached is a Floor Plan of the abutting Black Bird building, as well as Town Record documents of the Parking Plan and Change of Use approval for said Business Suites use. All of these documents result in the following parking calculation for the abutting property as per §16.7.11.F.(4).(d):

Professional Offices: [2 Parking Spaces per Office Unit]

+ [1 Parking Space per 250 sq. ft. Gross Floor Area]

11 Office Units (all three floors) = 22 Spaces 2,653 sq. ft. GFA (first floor) = 10.61 Spaces 3,377 sq. ft. GFA (second floor) = 13.51 Spaces 700 sq. ft. GFA (third floor) = 2.8 Spaces

[22 + 10.61 + 13.51 + 2.8] = 48.92 => 49 Spaces Required (45 Existing)

As demonstrated by the above calculation, the abutting Black Bird Business Suites has no available overflow parking to dedicate to this proposed development, as has been previously communicated to the Town. As such, this assertion has been abandoned and the Plan Set has been redesigned to reflect this change as described below. This change also removes several other talking points of the Preliminary SPR thus far, including previous TIA recommendations for the closing of an abutting curb cut to facilitate a one-way exit from this property, the construction of a sidewalk along said one-way road for pedestrian access from overflow parking, and the re-sizing of the existing culverted crossing beneath the gravel access drive currently used for the Boat Yard.

- With the abandonment of use of the abutting property as discussed above, the Plan Set has been redesigned to include the following:
 - Additional parking has been provided on-site, with an additional 4-space stall being added on the north side of the building. The proposed parking lot to the

rear of the proposed building has also been expanded by one space, resulting in 13 total spaces being provided, more than double the 6 required by the Town Code of Ordinances. The location of the proposed building has not changed, and all proposed parking still complies with the setbacks outlined in Table 16.5.30 "Minimum Setbacks from Wetlands and Water Bodies".

- The proposed travelway that previously ended in a hammerhead turnaround to the rear of the building has now been extended to provide full two-way circulation around the entirety of the site. Proposed grading has been provided, and the stormwater management areas have been revised to reflect this change.
- The one-way exit onto the southerly-abutting property has been removed, as this site redesign which provides more parking and internal vehicle circulation removes the need.
- A Photometric Plan has been prepared and is included in the Plan Set to depict the proposed lighting around the entire development and demonstrate Dark Sky compliance for all fixtures. Lighting Spec Sheets are also attached for all proposed fixture types.
- With the removal of the proposed one-way exit in the location of the existing gravel access drive for the Boat Yard, the Applicant is now proposing to remove the existing gravel drive in this location, remove the existing 12" culverted crossing, and restore this area to the surrounding wetland grade in addition to the larger restoration proposed in the rear of the property. As requested by the Planning Board, a Wetland Restoration Narrative has been prepared by the project Wetland Scientist Michael Cuomo, CWS#211. This narrative includes both the previously-discussed 10,500 square-foot restoration in the western end of the parcel as well as a 2,500 square-foot section of fill in the area of the gravel access drive for the Boat Yard. Lastly, this narrative also includes a monitoring plan for during-construction and post-construction periods of the proposed development.
- Regarding an update on the proposal for a right-hand turn lane for southbound traffic and/or left-hand turn lane for northbound traffic within the Route 236 corridor, correspondence is attached between Attar and Diane Morabito, P.E. of Sewall, the Transportation Engineer who prepared and revised the TIA for earlier iterations of this development. This correspondence discusses these turning lane requests and speaks to the additional summer traffic counts that Sewall will be performing to determine if the peak volumes would warrant MDOT requiring such turning lanes. The Applicant will keep the Town informed as this information and subsequent report becomes available.
- At the July 14th Planning Board meeting the Board requested the Applicant to determine the level of MDEP permitting that this project would be subject to, given the history of wetland fill on the property to support the Boat Storage Use, the wetland alterations proposed as part of this application, as well as the wetland restoration efforts proposed as part of this application. In response to this request, the Applicant contacted Mr. Darren LaPierre, owner of the abutting Black Bird Business Suites and former owner of the subject parcel for the Boat Storage Yard use. Mr. LaPierre provided the Applicant with records to create a complete history of the site preparations and fills to create the Boat Storage Yard, and also provided a recollection of the construction process. To summarize:
 - May 30th, 2006 Signed Memo prepared by Town of Kittery Planner Jim Noel, approving the Change of Use from Warehousing and Storage to Boat Yard and Marine Storage for the subject parcel.
 - September 29th, 2008 RFI from Town of Kittery Planning Office requesting an updated Wetland Delineation to support the Change of Use
 - January 5th, 2009 Wetlands Sketch Plan prepared by Attar Engineering, Inc., signed and sealed by Kenneth A. Wood, P.E.. Plan stamped as received by Town of Kittery on January 26th, 2009 at 10:36am.

- January 21st, 2009 Signed MDEP Field Determination Form prepared by Chris Coppi, who was present in the field for the above-mentioned Wetland Delineation, stating that the subject parcel qualifies for a one-time wetland alteration exemption of not more than 4,300 square feet.
- February 19th, 2009 Signed Notice of Decision prepared by Town of Kittery Planner Sandra Mowery and Assistant Code Officer Shelly Bishop, approving Business Occupancy Change for the subject parcel to be developed as a Boat Yard and Marine Storage use, citing the above-referenced MDEP Memo and Wetlands Sketch Plan.
- February 24th, 2009 Signed Notice of Decision Amendment prepared by Town of Kittery Planner Sandra Mowery and Code Enforcement Officer Heather Ross, with said amendment extending the buffer area abutting Martin Road residences from 30 feet to 40 feet as agreed upon by phone conversation with Mr. LaPierre.

[Start of Construction]

- Then-owner Darren LaPierre completed the majority of the above-described clearing and installed a timber fenceline at the buffer limits abutting the Martin Road residences.
- Prior to fulfillment of the total fill allowed by the permits and approvals, Mr.
 LaPierre was approached by a Town-contracted Excavating Company working on the sewer line extension beneath a neighboring section of the Route 236 corridor.
- Mr. LaPierre and said Contractor reached an agreement to allow for clean fill from sewer line trench work to be deposited on the subject parcel within the limits and square footages outlined in the permits and approvals.
- Sometime after said agreement was reached and fill from the sewer extension began to be deposited on-site, the Town of Kittery Code Enforcement Officer Bob Marchi was contacted by an abutter to Mr. LaPierre's property with complaints of fill extending within the buffer area and up to the property line.
- o Mr. LaPierre arranged for a site visit with Mr. Marchi and during this visit it was determined that the abutter had incorrectly assumed that the constructed timber fence was the property line, instead of a marker for clearing and fill limits. Mr. Marchi determined that Mr. LaPierre was not in violation of his permits and was allowed to continue his operation.

With the historical summary concluded, the Applicant believes that all previous fills on the subject parcel were deposited responsibly and within the limits of the permits and approvals associated with the Boat Storage Yard. With the newly-proposed removal of the existing gravel access drive, the total wetland impacts for the proposed development are 1,367 square feet, with no impacts occurring within the southerly Wetland of Special Significance. The Applicant continues to contend that this application is subject to a NRPA PBR for Activities adjacent to an (assumed) Significant Vernal Pool, and a NRPA PBR for Activities adjacent to a Protected Natural Resource (Wetland of Special Significance).

Thank you for your review and consideration – we look forward to discussing this project at upcoming Planning Board meetings. Please contact me for any additional information or clarifications required.

Sincerely;

Michael J. Sudak, E.I.

Staff Engineer

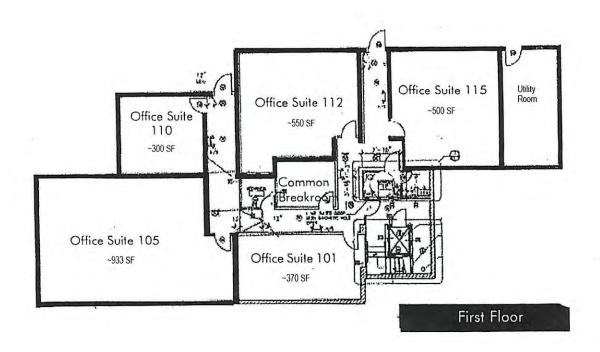
cc: Well Field 44, LLC.

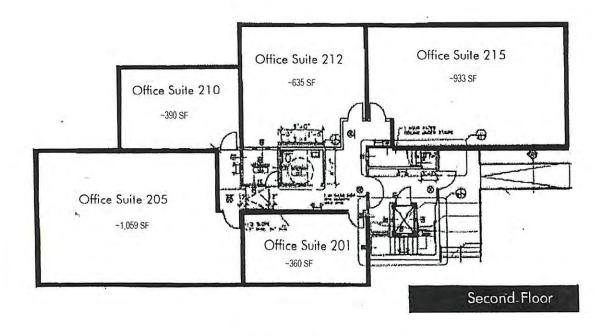
C277-22 Cover Rev 04Aug2022.doc

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37 ROUTE 236, KITTERY, MAINE

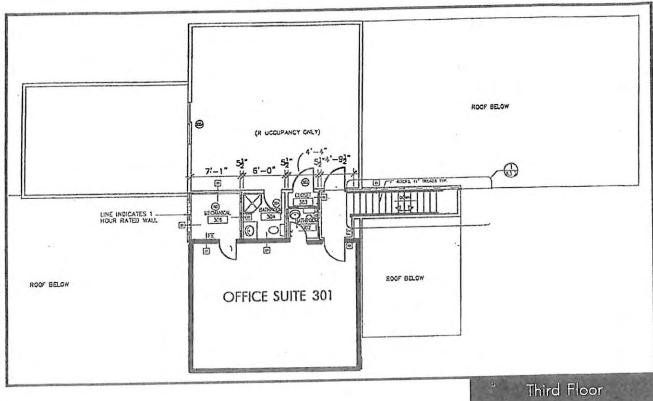
FLOOR PLAN





ATTACHMENT A

Route 236 ♦ Kittery, ME



C'a

PHILIP & VIOLA PARADIS erist. conc. mon. -11 5-19-40 E CONCRETE MONUMENT . NOTES: A) Required Parking 12 units @ 2 = 7941sf/250 = 24 spaces 32 spaces 56 spaces Total B) Parking Available 9 x 19 51 spaces H/C 5 spaces Mail Delv 1 space Total 57 spaces C) Trees
Type a
Type b Sugar Maple Bradford Pear reaist granite manument — ROUTE 236 — 10

Pine Brook Business Suites
SITE PLAN
Kittery, ME

BUILDING KEY

*VID	
DATE	
1	_
-	_
	DATE

1 691
CHO ST:



Town of Kittery, Maine - Development Department

200 Rogers Road, Kittery, ME 03904 Phone: (207) 475-1308 Fax: (207) 439-6806

PROPERTY OWNER & ADDRESS: Lady Slipper LLC, 37 Route 236 Suite 105, Kittery, ME 03904

APPLICANT NAME & ADDRESS Dick Johnson, Pine Brook Business Corp., 37 Route 236 Suite 105, Kittery, ME

03904

PROPERTY LOCATION: 37 Route 236 Suite 205

MAP LOT: Map 20 Lot 17

APPLICATION: Business Use Change - Pine Brook

ZONE(S): Commercial - 2
DATE: January 3, 2018

Title 16.10.3.2 Other Development Review, of the Town of Kittery Land Use and Development Code, does not include business use review, per 16.4.3.5, as requiring Planning Board approval.

16.4.3.5 Business Use Changes.

The Planner and the Code Enforcement Officer are to review and approve, or refer to the Planning Board for action, all business use changes which occur that fall below Planning Board review thresholds as outlined in Sections 16.10.3.2 and 16.10.3.6. Approval must be based on compliance with all requirements of this Code.

The Town Planner and Code Enforcement Officer have reviewed the Business Use Change (BUC) & Building Permit applications and makes the following findings:

Purpose

Review and approve to update the record and required parking for the current businesses located at 37 Route # 236.

Approval

1st Floor

Suite 101 – Emera Energy, 373 square feet

Suite 105 – Pine Brook Corp., 997 square feet

Suite 110 – Dr. Phelps, 304 square feet

Suite 112 – Earthshift Global, 541 square feet

Suite 115 – Vacant, 535 square feet

2nd Floor

Suite 201 – Vacant, 373 square feet

Suite 205 – Atlantic Rehab, 1,089 square feet

Suite 210 – Planet Fitness, 524 square feet

Suite 212 - Vacant, 497 square feet

Suite 215 – Cherokee Construction, 1,054 square feet

3rd Floor

Suite 301 – Vacant, 469 square feet

The total square footage for all offices is 6757.

Required parking is calculated at 1 space per every 250 feet gross floor area plus 2 per each office.

Total gross square footage 6756/250 = 27, plus 11 offices at 2 spaces per office (22), totals 49 spaces required for entire site.

Parking plan reflects 57 spaces are provided.

Your request appears to be in conformance with 16.10.9.3.1 Minor Modifications and 16.10.3.2 Other Development Review, and is approved as submitted.

This Notice of Decision IS NOT a building permit or a sign permit.

duliz

Any proposed field changes, diversion or revisions to the plan and construction documents after approval shall be reviewed and approved by the Code Enforcement Officer prior to proceeding with the proposed changes.

Any site changes not approved in this Notice of Decision will be in violation of State law and Town Codes.

Congratulations, we look forward to working with you.

Sincerely:

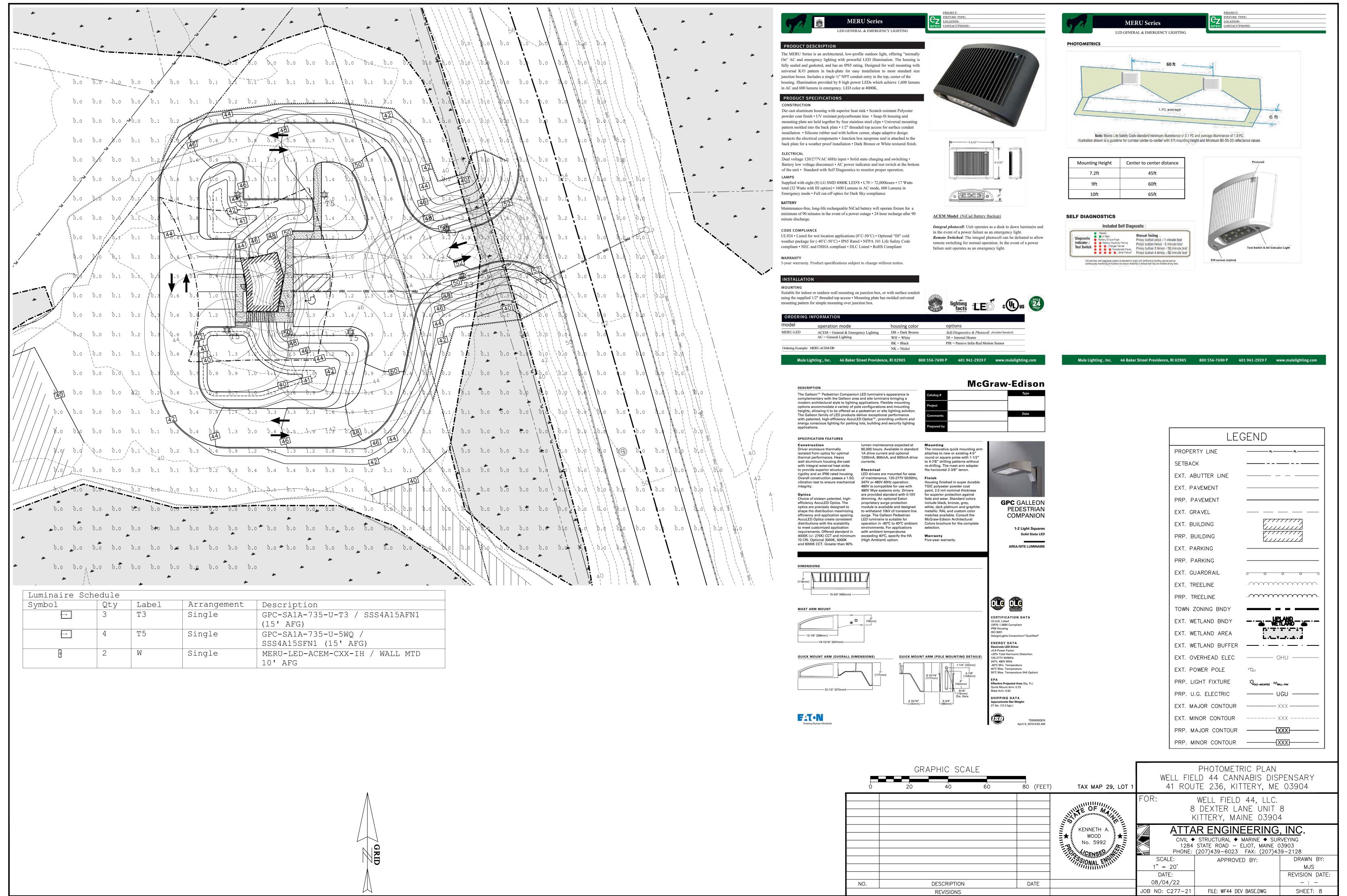
Kendra Amaral

Town Manager

Robert Marchi

Code Enforcement Officer

Cc: Planning Board Chair, File



MAP 29, LOT

DESCRIPTION

The Galleon™ Pedestrian Companion LED luminaire's appearance is complementary with the Galleon area and site luminaire bringing a modern architectural style to lighting applications. Flexible mounting options accommodate a variety of pole configurations and mounting heights, allowing it to be offered as a pedestrian or site lighting solution. The Galleon family of LED products deliver exceptional performance with patented, high-efficiency AccuLED Optics™, providing uniform and energy conscious lighting for parking lots, building and security lighting applications.

Catalog #	Туре
Project	
Comments	Date
Prepared by	

McGraw-Edison

SPECIFICATION FEATURES

Construction

Driver enclosure thermally isolated from optics for optimal thermal performance. Heavy wall aluminum housing die-cast with integral external heat sinks to provide superior structural rigidity and an IP66 rated housing. Overall construction passes a 1.5G vibration test to ensure mechanical integrity.

Optics

Choice of sixteen patented, highefficiency AccuLED Optics. The optics are precisely designed to shape the distribution maximizing efficiency and application spacing. AccuLED Optics create consistent distributions with the scalability to meet customized application requirements. Offered standard in 4000K (+/- 275K) CCT and minimum 70 CRI. Optional 3000K, 5000K and 6000K CCT. Greater than 90%

lumen maintenance expected at 60,000 hours. Available in standard 1A drive current and optional 1200mA, 800mA, and 600mA drive currents.

Electrical

LED drivers are mounted for ease of maintenance. 120-277V 50/60Hz, 347V or 480V 60Hz operation. 480V is compatible for use with 480V Wye systems only. Drivers are provided standard with 0-10V dimming. An optional Eaton proprietary surge protection module is available and designed to withstand 10kV of transient line surge. The Galleon Pedestrian LED luminaire is suitable for operation in -40°C to 40°C ambient environments. For applications with ambient temperatures exceeding 40°C, specify the HA (High Ambient) option.

Mounting

The innovative quick mounting arm attaches to new or existing 4-5' round or square poles with 1-1/2" to 4-7/8" drilling patterns without re-drilling. The mast arm adapter fits horizontal 2-3/8" tenon.

Finish

Housing finished in super durable TGIC polyester powder coat paint, 2.5 mil nominal thickness for superior protection against fade and wear. Standard colors include black, bronze, grey, white, dark platinum and graphite metallic. RAL and custom color matches available. Consult the McGraw-Edison Architectural Colors brochure for the complete selection.

Warranty

Five-year warranty.

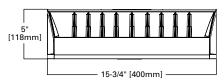


GPC GALLEON **PEDESTRIAN** COMPANION

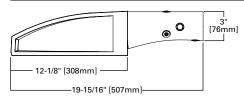
1-2 Light Squares Solid State LED

AREA/SITE LUMINAIRE

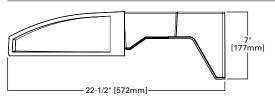
DIMENSIONS



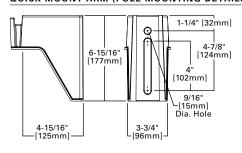
MAST ARM MOUNT



QUICK MOUNT ARM (OVERALL DIMENSIONS)



QUICK MOUNT ARM (POLE MOUNTING DETAILS)







CERTIFICATION DATA UL/cUL Listed

LM79 / LM80 Compliant IP66 Housing ISO 9001 DesignLights Consortium® Qualified*

ENERGY DATA

Electronic LED Driver

>0.9 Power Factor <20% Total Harmonic Distortion 120-277V 50/60Hz 347V, 480V 60Hz

-40°C Min. Temperature 40°C Max. Temperature

50°C Max. Temperature (HA Option)

Effective Projected Area (Sq. Ft.) Quick Mount Arm: 0.73 Mast Arm: 0.62

SHIPPING DATA Approximate Net Weight: 27 lbs. (12.2 kgs.)





POWER AND LUMENS

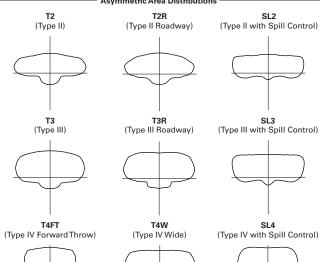
	Light Squares							2	
Drive Curre		600mA	800mA	1.0A	1.2A	600mA	800mA	1.0A	1.2A
	ower (Watts)	34	44	59	67	66	86	113	129
-	ent @ 120V (A)	0.3	0.39	0.51	0.58	0.58	0.77	1.02	1.16
Input Curre	ent @ 208V (A)	0.17	0.22	0.29	0.33	0.34	0.44	0.56	0.63
Input Curre	ent @ 240V (A)	0.15	0.19	0.26	0.29	0.3	0.38	0.48	0.55
Input Curre	ent @ 277V (A)	0.14	0.17	0.23	0.25	0.28	0.36	0.42	0.48
Input Curre	ent @ 347V (mA)	0.11	0.15	0.17	0.2	0.19	0.24	0.32	0.39
Input Curre	ent @ 480V (mA)	0.08	0.11	0.14	0.15	0.15	0.18	0.24	0.3
Optics		,			,	,			
	4000K/5000K Lumens	4,204	5,156	6,381	7,000	8,215	10,075	12,470	13,680
T2	3000K Lumens	3,975	4,874	6,033	6,618	7,767	9,525	11,790	12,934
	BUG Rating	B1-U0-G1	B1-U0-G1	B1-U0-G2	B1-U0-G2	B1-U0-G2	B2-U0-G2	B2-U0-G2	B2-U0-G2
	4000K/5000K Lumens	4,464	5,474	6,775	7,431	8,723	10,696	13,239	14,523
T2R	3000K Lumens	4,221	5,176	6,406	7,025	8,247	10,113	12,517	13,731
	BUG Rating	B1-U0-G1	B1-U0-G1	B1-U0-G1	B1-U0-G1	B1-U0-G2	B1-U0-G2	B2-U0-G2	B2-U0-G2
	4000K/5000K Lumens	4,285	5,256	6,505	7,135	8,375	10,269	12,710	13,943
Т3	3000K Lumens	4,051	4,969	6,150	6,746	7,918	9,710	12,017	13,182
	BUG Rating	B1-U0-G1	B1-U0-G1	B1-U0-G2	B1-U0-G2	B1-U0-G2	B2-U0-G2	B2-U0-G2	B2-U0-G2
	4000K/5000K Lumens	4,380	5,372	6,648	7,294	8,561	10,498	12,993	14,253
T3R	3000K Lumens	4,141	5,078	6,286	6,895	8,094	9,925	12,285	13,475
	BUG Rating	B1-U0-G1	B1-U0-G2	B1-U0-G2	B1-U0-G2	B1-U0-G2	B1-U0-G2	B2-U0-G2	B2-U0-G2
	4000K/5000K Lumens	4,311	5,286	6,542	7,177	8,422	10,329	12,784	14,024
T4FT	3000K Lumens	4,075	4,998	6,185	6,786	7,963	9,766	12,086	13,259
	BUG Rating	B1-U0-G1	B1-U0-G2	B1-U0-G2	B1-U0-G2	B1-U0-G2	B1-U0-G2	B2-U0-G2	B2-U0-G3
	4000K/5000K Lumens	4,254	5,217	6,458	7,084	8,313	10,195	12,619	13,843
T4W	3000K Lumens	4,023	4,933	6,105	6,698	7,860	9,639	11,931	13,088
1444	BUG Rating	B1-U0-G1	#,955 B1-U0-G2	B1-U0-G2	B1-U0-G2	B1-U0-G2	B2-U0-G2	B2-U0-G2	B2-U0-G3
	-								
01.0	4000K/5000K Lumens	4,196	5,147	6,370	6,988	8,202	10,058	12,449	13,656
SL2	3000K Lumens	3,967	4,866	6,022	6,607	7,755	9,509	11,771	12,911
	BUG Rating	B1-U0-G1	B1-U0-G1	B1-U0-G2	B1-U0-G2	B1-U0-G2	B2-U0-G2	B2-U0-G3	B2-U0-G3
	4000K/5000K Lumens	4,284	5,255	6,504	7,134	8,374	10,268	12,709	13,941
SL3	3000K Lumens	3,849	4,720	5,842	6,408	7,520	9,224	11,415	12,523
	BUG Rating	B1-U0-G2	B1-U0-G2	B1-U0-G2	B1-U0-G2	B1-U0-G2	B1-U0-G3	B1-U0-G3	B1-U0-G3
	4000K/5000K Lumens	4,071	4,992	6,179	6,778	7,954	9,756	12,074	13,246
SL4	3000K Lumens	3,849	4,720	5,842	6,408	7,520	9,224	11,415	12,523
	BUG Rating	B1-U0-G2	B1-U0-G2	B1-U0-G2	B1-U0-G2	B1-U0-G2	B1-U0-G3	B1-U0-G3	B1-U0-G3
	4000K/5000K Lumens	4,420	5,420	6,709	7,358	8,637	10,591	13,108	14,380
5NQ	3000K Lumens	4,179	5,124	6,343	6,957	8,166	10,013	12,393	13,595
	BUG Rating	B2-U0-G1	B2-U0-G1	B2-U0-G1	B3-U0-G1	B3-U0-G1	B3-U0-G1	B3-U0-G2	B3-U0-G2
	4000K/5000K Lumens	4,501	5,520	6,831	7,494	8,795	10,786	13,350	14,644
5MQ	3000K Lumens	4,256	5,219	6,458	7,085	8,316	10,198	12,622	13,845
	BUG Rating	B3-U0-G1	B3-U0-G1	B3-U0-G1	B3-U0-G1	B3-U0-G2	B3-U0-G2	B4-U0-G2	B4-U0-G2
	4000K/5000K Lumens	4,513	5,534	6,849	7,514	8,819	10,815	13,385	14,683
5WQ	3000K Lumens	4,268	5,232	6,475	7,104	8,338	10,224	12,656	13,882
	BUG Rating	B3-U0-G1	B3-U0-G1	B3-U0-G2	B3-U0-G2	B3-U0-G2	B4-U0-G2	B4-U0-G2	B4-U0-G2
	4000K/5000K Lumens	3,765	4,619	5,716	6,270	7,358	9,023	11,167	12,251
SLL/SLR	3000K Lumens	3,560	4,367	5,404	5,927	6,957	8,531	10,559	11,583
	BUG Rating	B1-U0-G1	B1-U0-G2	B1-U0-G2	B1-U0-G2	B1-U0-G2	B1-U0-G2	B1-U0-G3	B2-U0-G3
	4000K/5000K Lumens	4,379	5,370	6,647	7,293	8,558	10,494	12,989	14,250
RW	3000K Lumens	4,141	5,077	6,285	6,895	8,092	9,922	12,281	13,473
	BUG Rating	B2-U0-G1	B2-U0-G1	B3-U0-G1	B3-U0-G1	B3-U0-G1	B3-U0-G1	B3-U0-G2	B3-U0-G2
	4000K/5000K Lumens	4,396	5,390	6,672	7,318	8,590	10,533	13,037	14,301
AFL	3000K Lumens	4,156	5,096	6,308	6,919	8,121	9,959	12,326	13,521
	BUG Rating	B1-U0-G1	B1-U0-G1	B1-U0-G1	B1-U0-G1	B1-U0-G1	B1-U0-G1	B2-U0-G2	B2-U0-G2
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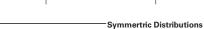
 $^{^\}star$ Nominal lumen data for 70 CRI. BUG rating for 4000K/5000K. Refer to IES files for 3000K BUG ratings.



OPTICAL DISTRIBUTIONS

Asymmetric Area Distributions





5NQ (Type V Square Narrow)

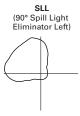




RW (Rectangular Wide Type I)

5WQ

Specialized Distributions SLR (90° Spill Light Eliminator Right)

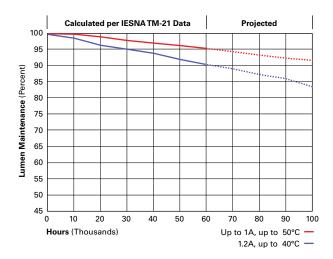


AFL (Automotive Frontline)



LUMEN MAINTENANCE

Drive Current	Ambient Temperature	TM-21 Lumen Maintenance (60,000 Hours)	Projected L70 (Hours)
Up to 1A	Up to 50°C	> 95%	> 416,000
1.2A	Up to 40°C	> 90%	> 205,000



LUMEN MULTIPLIER

Ambient Temperature	Lumen Multiplier
0°C	1.02
10°C	1.01
25°C	1.00
40°C	0.99
50°C	0.97

CONTROL OPTIONS

0-10V (DIM)

This fixture is offered standard with 0-10V dimming driver(s). The DIM option provides 0-10V dimming wire leads for use with a lighting control panel or other control method.

Photocontrol (P. R and PER7)

Optional button-type photocontrol (P) and photocontrol receptacles (R and PER7) provide a flexible solution to enable "dusk-to-dawn" lighting by sensing light levels. Advanced control systems compatible with NEMA 7-pin standards can be utilized with the PER7 receptacle.

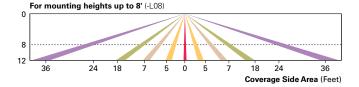
After Hours Dim (AHD)

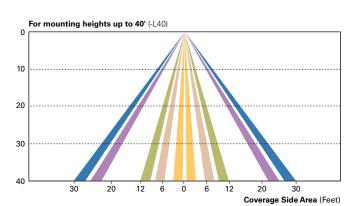
This feature allows photocontrol-enabled luminaires to achieve additional energy savings by dimming during scheduled portions of the night. The dimming profile will automatically take effect after a "dusk-to-dawn" period has been calculated from the photocontrol input. Specify the desired dimming profile for a simple, factory-shipped dimming solution requiring no external control wiring. Reference the After Hours Dim supplemental guide for additional information.

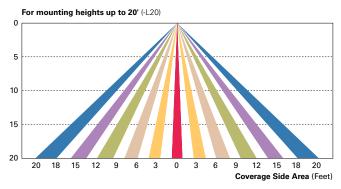
Dimming Occupancy Sensor (MS/DIM-LXX and MS-LXX)

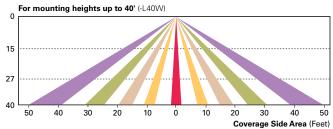
These sensors are factory installed in the luminaire housing. When the MS/DIM-LXX sensor option is selected, the occupancy sensor is connected to a dimming driver and the entire luminaire dims when there is no activity detected. When activity is detected, the luminaire returns to full light output. The MS/DIM sensor is factory preset to dim down to approximately 50 percent power with a time delay of five minutes. The MS-LXX sensor is factory preset to turn the luminaire off after five minutes of no activity. The MS/X-LXX is also preset for five minutes and only controls the specified number of light engines to maintain steady output from the remaining light engines.

These occupancy sensors includes an integral photocell that can be activated with the FSIR-100 accessory for "dusk-to-dawn" control or daylight harvesting - the factory preset is OFF. The FSIR-100 is a wireless tool utilized for changing the dimming level, time delay, sensitivity and other parameters. A variety of sensor lens are available to optimize the coverage pattern for mounting heights from 8'-40'.



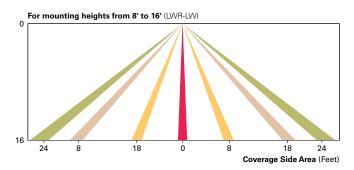


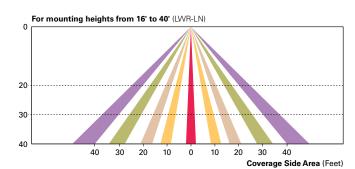




LumaWatt Pro Wireless Control and Monitoring System (LWR-LW and LWR-LN)

The Eaton's LumaWatt Pro powered by Enlighted is a connected lighting solution that combines a broad selection of energy-efficient LED luminaires with a powerful integrated wireless sensor system. The sensor controls the lighting system in compliance with the latest energy codes and collects valuable data about building performance and use. Software applications turn the granular data into information through energy dashboards and specialized apps that make it simple and help optimize the use of building resources, beyond lighting.





WaveLinx Wireless Outdoor Lighting Control Module (WOLC-7P-10A)

The 7-pin wireless outdoor lighting control module enables WaveLinx to control outdoor area, site and flood lighting. WaveLinx controls outdoor lighting using schedules to provide ON, OFF and dimming controls based on astronomic or time schedules based on a 7 day week.



ORDERING INFORMATION

Sample Number: GPC-AF-02-LED-E1-T3-GM

Product Family	Light Engine	Number of Light Squares ¹	Lamp Type	Voltage	Distribution	Color	Mounting Options
GPC=Galleon Pedestrian Companion	AF=1A Drive Current	01=1 02=2	LED=Solid State Light Emitting Diodes	E1=120-277V 347=347V ² 480=480V ^{2,3}	47 V ² T2R= Type II Roadway BZ=Bronze f 80 V ² ³ T3=Type III BK=Black I		QM=Quick Mount Arm for Round or Square Pole ^{5,6} MA=2-3/8" Mast Arm ^{5,7}
Options (Add as S	uffix)	•	•		Accessories (Order Separately)		
R=NEMA Twistloc PER7=NEMA 7-PIN AHD145=After Ho AHD245=After Ho AHD255=After Ho AHD355=After Ho MS-LXX=Motion S MS/DIM-LXX=MO LWR-LW=LumaWa	DK 8 DK 9 DE Factory Set to 6 In Factory Set to 2 In Factory Set to 3 In Factory Set to 3 In Factory Set to 4 In Factory Set to 4 In Factory Set to 4 In Factory Set to 5 In Factory Set to 6 In Factory Set to 6 In Factory Set To 1 In Fac	300mA 1200mA 1200mA Must Specify Volta V. Must Specify Vo 208, 240 or 277V. If Receptacle ocontrol Receptac 14 15 14 15 14 15 14 15 16 17 17 18 19 19 19 19 19 19 19 19 19 19 19 19 19	Itage) Must Specify Voltage) le ¹³ n ^{15, 16, 17} 8' - 16' Mounting Heigh or 16' - 40' Mounting He		OA/RA1013=Photocontrol Shorting Ca OA/RA1016=NEMA Photocontrol - Mu OA/RA1201=NEMA Photocontrol - 347 OA/RA1027=NEMA Photocontrol - 480 MA1252=10kV Circuit Module Replace MA1036-XX=Single Tenon Adapter for MA1197-XX=3@120° Tenon Adapter for MA1188-XX=2@90° Tenon Adapter for MA1189-XX=2@90° Tenon Adapter for MA119-XX=3@90° Tenon Adapter for MA1191-XX=2@120° Tenon Adapter for MA1191-XX=2@120° Tenon Adapter for MA1191-XX=2@120° Tenon Adapter for MA1192-XX=3@120° Tenon Adapter for MA1193-XX=2@90° Tenon Adapter for MA1193-XX=3@90° Tenon Adapter for MA1193-XX=3@90° Tenon Adapter for MA1193-XX=4@90° Tenon Adapter for MA1193-XX=4@90° Tenon Adapter for MA1193-XX=3@90° Tenon Adapter for FSIR-100=Wireless Configuration Tool LS/HSS=Field Installed House Side Sh WOLC-7P-10A=WaveLinx Outdoor Con	ti-Tap 105-285V V V ment 2-3/8" O.D. Tenon r 2-3/8" O.D. Tenon r 2-3/8" O.D. Tenon 2-3/8" O.D. Tenon 2-3/8" O.D. Tenon 2-3/8" O.D. Tenon 3-1/2" O.D. Tenon r 3-1/2" O.D. Tenon 3-1/2" O.D. Tenon 3-1/2" O.D. Tenon 3-1/2" O.D. Tenon 3-1/2" O.D. Tenon 3-1/2" O.D. Tenon 6-1/2" O.D. Tenon 1-1/2" O.D. Tenon	₇ 16

NOTES:

- 1. Standard 4000K CCT and minimum 70 CRI.
- 1. Standard 4000K CCT and minimum 70 CRI.
 2. Requires the use of a step down transformer. Not available in combination with sensor options at 1200mA.
 3. Only for use with 480V Wye systems. Per NEC, not for use with ungrounded systems, impedance grounded systems or corner grounded systems (commonly known as Three Phase Three Wire Delta, Three Phase High Leg Delta and Three Phase Corner Grounded Delta systems).
 4. Custom colors are available. Setup charges apply, Paint chip is amples required. Extended Lead times apply.
 5. Customer is responsible for engineering analysis to confirm pole and fixture compatibility for all applications. Refer to our white paper WP513001EN for additional support information.
 6. Quick mount arm adapter is factory installed. Pole mouting bracked shipped in box. Suitable for 1.5G. Fits square and round pole up to 6" O.D.
 7. Mast arm adapter factory installed (2-3/8" O.D. arm only). Suitable for 3G vibration.
 8. Extended lead times apply. Use dedicated IES files when performing layouts.
 9. Not available with HA option.
 10. Cannot be used with other control options.
 11. Low voltage control lead brought out 18" outside fixture.

- 10. Lamnot be used with other control options.

 11. Low voltage control lead brought out 18" outside fixture.

 12. HA option available for single light square only. Not available with 1200mA drive current.

 13. Compatible with standard 3-PIN photocontrols, 5-PIN or 7-PIN ANSI controls.

 14. Requires the use of P photocontrol or the PER7 or R photocontrol receptacle with photocontrol accessory. See After Hours Dim supplemental guide for additional information.

 15. Replace LXX with mounting height in feet for proper lens selection (e.g., LS=8" mounting height). L8, L20, L40, and L40W are available options.

 16. The FSIR-100 configuration tool is required to adjust parameters including high and low modes, sensitivity, time delay, cutoff and more. Consult your lighting representative at Eaton for more information.

 17. Includes integral photosensor.

 18. Bronze sensor is shipped with Bronze fixtures. White sensor shipped on all other housing color notions.
- 17. Includes integral photosensor.

 18. Bronze sensor is shipped with Bronze fixtures. White sensor shipped on all other housing color options.

 19. LumaWatt wireless sensors are factory installed requiring network components in appropriate quantities. See www.eaton.com/lighting for LumaWatt application information.

 20. Not available with HSS option.

 21. Only for use with BLS, SL3, SL4, and AFL distributions. The light square trim plate is painted black when the HSS option is selected.

 22. CE is not available with the LWR, MS, MS/DIM, P, R or PER7 options. Available in 120-277V only.
- 23. One required for each light square.
- 24. Requires 7-pin NEMA twistlock photocontrol receptacle. The WOLC-7 cannot be used in conjunction with additional sensors or controls.



Steel Poles



SSS SQUARE STRAIGHT STEEL

Catalog #	Туре
Project	
Comments	Date
Prepared by	

FEATURES

- ASTM Grade steel base plate with ASTM A366 base cover
- Hand hole assembly 3" x 5" on 5" and 6" pole; and 2" x 4" on 4" pole
- 10'-39' mounting heights
- Drilled or tenon (specify)

DESIGN CONSIDERATIONS

Wind induced vibrations resulting from steady, unidirectional winds and other aerodynamic forces, as well as vibration and coefficient of height factors for non-grounded mounted installations (e.g., installations on bridges or buildings) are not included in this document. The information contained herein is for general guidance only and is not a replacment for professional judgement. Consult with a professional, and local and federal standards, before ordering to ensure product is appropriate for the intended purpose and installation location. Also, please review Eaton's Light Pole White Paper for risk factors and design considerations. Learn more.

Specifications and dimensions subject to change without notice. Consult your lighting representative at Eaton or visit www.eaton.com/lighting for available options, accessories and ordering information.

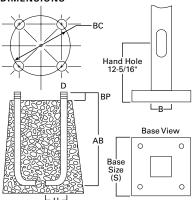
ORDERING INFORMATION

SAMPLE NUMBER: SSA5A20SFM1XG

Product Family	Shaft Size (Inches) ¹	Wall Thickness (Inches)	Mounting Height (Feet)	Base Type	Finish	Mounting Type	Number and Location of Arms	Arm Lengths (Feet)	Options (Add as Suffix)
SSS=Square Straight Steel	4=4" 5=5" 6=6"	A=0.120" M=0.188" X=0.250"	10=10' 15=15' 20=20' 25=25' 30=30' 35=35' 39=39'	S=Square Steel Base	F=Dark Bronze G=Galvanized Steel J=Summit White K=Carbon Bronze L=Dark Platinum R=Hartford Green S=Silver T=Graphite Metallic V=Grey W=White X=Custom Color Y=Black	2=2-3/8" O.D. Tenon (4" Long) 3=3-1/2" O.D. Tenon (5" Long) 4=4" O.D. Tenon (6" Long) 9=3" O.D. Tenon (4" Long) 6=2-3/8" O.D. Tenon (6" Long) 7=4" O.D. Tenon (10" Long) A=Type A Drilling C=Type C Drilling E=Type E Drilling F=Type F Drilling G=Type G Drilling J=Type J Drilling M=Type K Drilling M=Type M Drilling N=Type N Drilling N=Type N Drilling S=Standard Upsweep Arm Z=Type Z Drilling	1=Single 2=2 at 180° 3=Triple ² 4=4 at 90° 5=2 at 90° X=None	X=None 2=2' 3=2.5' 4=4' 6=6' 8=8'	A=1/2" Tapped Hub ³ B=3/4" Tapped Hub ³ C=Convenience Outlet ⁴ E=GFCI Convenience Outlet ⁴ G=Ground Lug H=Additional Hand Hole ⁵ V=Vibration Dampener

NOTES: 1. All shaft sizes nominal. 2. Square poles are 3 at 90°, round poles are 3 at 120°. 3. Tapped Hub is located 5′ below the pole top and on the same side of pole as hand hole, unless specified otherwise. 4. Outlet is located 4′ above base and on same side of pole as hand hole, unless specified otherwise. Receptacle not included, provision only. 5. Additional hand hole is located 12" below pole top and 90° from standard hand hole location, unless otherwise specified.

DIMENSIONS



└─ H ── See technical information.



page 2 SSS SQUARE STRAIGHT STEEL

Effective Projected Area (At Pole Top)

Mounting Height (Feet)	Catalog Number ^{1, 2}	Wall Thickness (Inches)	Base Square ³ (Inches)	Bolt Circle Diameter (Inches)	Anchor Bolt Projection ³ (Inches)	Shaft Size ³ (Inches)	Anchor Bolt Diameter x Length x Hook (Inches)	Net Weight (Pounds)	Maxim		ve Project e Feet) ⁴	ed Area	Max. Fixture Load - Includes Bracket (Pounds)
МН			s	ВС	ВР	В	D x AB x H		80 mph	90 mph	100 mph	110 mph	
10	SSS4A10S	0.120	10-1/2	11	4-1/2	4	3/4 x 25 x 3	85	30.0	22.0	17.0	13.0	100
15	SSS4A15S	0.120	10-1/2	11	4-1/2	4	3/4 x 25 x 3	118	15.0	11.5	8.7	6.5	100
20	SSS4A20S	0.120	10-1/2	11	4-1/2	4	3/4 x 25 x 3	150	8.7	5.9	3.9	2.5	150
20	SSS5A20S	0.120	10-1/2	11	4-1/2	5	3/4 x 25 x 3	183	15.4	11.1	7.9	5.5	150
25	SSS4A25S	0.120	10-1/2	11	4-1/2	4	3/4 x 25 x 3	181	3.7	1.7	0.3		200
25	SSS5A25S	0.120	10-1/2	11	5	5	3/4 x 25 x 3	222	9.3	6.0	3.5	1.6	200
25	SSS6A25S	0.120	12-1/2	12-1/2	5	6	1 x 36 x 4	284	9.9	6.1	3.5	1.2	200
30	SSS5A30S	0.120	10-1/2	11	4-1/2	5	3/4 x 25 x 3	260	4.7	2.1			200
30	SSS5M30S	0.188	10-1/2	11	4-1/2	5	3/4 x 25 x 3	392	10.4	6.4	3.5	1.5	200
30	SSS6A30S	0.120	12-1/2	12-1/2	5	6	1 x 36 x 4	330	4.3	1.4			200
30	SSS6M30S	0.188	12-1/2	12-1/2	5	6	1 x 36 x 4	489	19.0	13.0	8.7	5.6	200
35	SSS5M35S	0.188	10-1/2	11	4-1/2	5	3/4 x 25 x 3	453	5.8	2.8			200
35	SSS6M35S	0.188	12-1/2	12-1/2	5	6	1 x 36 x 4	564	12.8	7.2	3.7	1.0	200
35	SSS6X35S	0.250	12-1/2	12-1/2	5	6	1 x 36 x 4	738	16.5	11.0	6.8	3.5	200
39	SSS6M39S	0.188	12-1/2	12-1/2	5	6	1 x 36 x 4	618	7.3	3.0			300
39	SSS6X39S	0.250	12-1/2	12-1/2	5	6	1 x 36 x 4	816	13.0	7.0	3.7	0.8	300

Fffective Projected Area (Two Feet Above Pole Ton)

Effective Projected Area (1W0 Feet Above Pole 10p)													
Mounting Height (Feet)	Catalog Number ^{1, 2}	Wall Thickness (Inches)	Base Square ³ (Inches)	Bolt Circle Diameter (Inches)	Anchor Bolt Projection ³ (Inches)	Shaft Size ³ (Inches)	Anchor Bolt Diameter x Length x Hook (Inches)	Net Weight (Pounds)	Maxim		ve Project e Feet) ⁴	ed Area	Max. Fixture Load - Includes Bracket (Pounds)
МН			s	ВС	ВР	В	D x AB x H		80 mph	90 mph	100 mph	110 mph	
10	SSS4A10S	0.120	10-1/2	11	4-1/2	4	3/4 x 25 x 3	85	23.0	17.5	14.0	11.0	100
15	SSS4A15S	0.120	10-1/2	11	4-1/2	4	3/4 x 25 x 3	118	13.4	10.0	7.5	5.7	100
20	SSS4A20S	0.120	10-1/2	11	4-1/2	4	3/4 x 25 x 3	150	7.6	5.2	3.4	2.1	150
20	SSS5A20S	0.120	10-1/2	11	4-1/2	5	3/4 x 25 x 3	183	13.8	9.9	7.1	4.9	150
25	SSS4A25S	0.120	10-1/2	11	4-1/2	4	3/4 x 25 x 3	181	3.4	1.6	0.3		200
25	SSS5A25S	0.120	10-1/2	11	5	5	3/4 x 25 x 3	222	8.5	5.5	3.2	1.5	200
25	SSS6A25S	0.120	12-1/2	12-1/2	5	6	1 x 36 x 4	284	9.1	5.6	3.0	1.2	200
30	SSS5A30S	0.120	10-1/2	11	4-1/2	5	3/4 x 25 x 3	260	1.8				200
30	SSS5M30S	0.188	10-1/2	11	4-1/2	5	3/4 x 25 x 3	392	9.6	5.9	1.9	0.2	200
30	SSS6A30S	0.120	12-1/2	12-1/2	5	6	1 x 36 x 4	330	4.1	1.3			200
30	SSS6M30S	0.188	12-1/2	12-1/2	5	6	1 x 36 x 4	489	18.5	12.5	8.4	5.3	200
35	SSS5M35S	0.188	10-1/2	11	4-1/2	5	3/4 x 25 x 3	453	5.5	2.4			200
35	SSS6M35S	0.188	12-1/2	12-1/2	5	6	1 x 36 x 4	564	11.8	7.0	3.5	1.0	200
35	SSS6X35S	0.250	12-1/2	12-1/2	5	6	1 x 36 x 4	738	16.0	10.5	6.4	3.4	200
39	SSS6M39S	0.188	12-1/2	12-1/2	5	6	1 x 36 x 4	618	7.0	2.4			300
39	SSS6X39S	0.250	12-1/2	12-1/2	5	6	1 x 36 x 4	816	12.0	6.7	3.0	0.5	300

NOTES:

- 1. Catalog number includes pole with hardware kit. Anchor bolts not included. Before installing, make sure proper anchor bolts and templates are obtained.

- 2. Tenon size or machining for rectangular arms must be specified. Hand hole position relative to drill location.

 3. Shaft size, base square, anchor bolts and projections may vary slightly. All dimensions nominal.

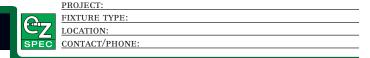
 4. EPAs based on shaft properties with wind normal to flat. EPAs calculated using base wind velocity as indicated plus 30% gust factor.





MERU Series

LED GENERAL & EMERGENCY LIGHTING



PRODUCT DESCRIPTION

The MERU Series is an architectural, low-profile outdoor light, offering "normally On" AC and emergency lighting with powerful LED illumination. The housing is fully sealed and gasketed, and has an IP65 rating. Designed for wall mounting with universal K/O pattern in back-plate for easy installation to most standard size junction boxes. Includes a single ½" NPT conduit entry in the top, center of the housing. Illumination provided by 8 high power LEDs which achieve 1,600 lumens in AC and 600 lumens in emergency. LED color at 4000K.

PRODUCT SPECIFICATIONS

CONSTRUCTION

Die cast aluminum housing with superior heat sink • Scratch resistant Polyester powder coat finish • UV resistant polycarbonate lens • Snap-fit housing and mounting plate are held together by four stainless steel clips • Universal mounting pattern molded into the back plate • 1/2" threaded top access for surface conduit installation • Silicone rubber seal with hollow center, shape adaptive design protects the electrical components • Junction box neoprene seal is attached to the back plate for a weather proof installation • Dark Bronze or White textured finish.

ELECTRICAL

Dual voltage 120/277VAC 60Hz input • Solid state charging and switching • Battery low voltage disconnect • AC power indicator and test switch at the bottom of the unit • Standard with Self Diagnostics to monitor proper operation.

Supplied with eight (8) LG SMD 4000K LED'S • L70 > 72,000hours • 17 Watts total (32 Watts with IH option) • 1600 Lumens in AC mode, 600 Lumens in Emergency mode • Full cut-off optics for Dark Sky compliance

BATTERY

Maintenance-free, long-life rechargeable NiCad battery will operate fixture for a minimum of 90 minutes in the event of a power outage • 24 hour recharge after 90 minute discharge.

CODE COMPLIANCE

UL924 • Listed for wet location applications (0°C-50°C) • Optional "IH" cold weather package for (-40°C-50°C) • IP65 Rated • NFPA 101 Life Safety Code compliant • NEC and OSHA compliant • DLC Listed • RoHS Compliant

WARRANTY

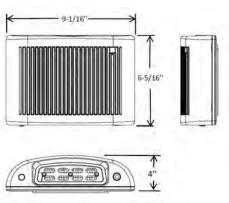
5-year warranty. Product specifications subject to change without notice.

INSTALLATION

MOUNTING

Suitable for indoor or outdoor wall mounting on junction box, or with surface conduit using the supplied 1/2" threaded top access • Mounting plate has molded universal mounting pattern for simple mounting over junction box.





ACEM Model (NiCad Battery Backup)

failure unit operates as an emergency light.

Integral photocell: Unit operates as a dusk to dawn luminaire and in the event of a power failure as an emergency light. **Remote Switched**: The integral photocell can be defeated to allow remote switching for normal operation. In the event of a power











ORDERING INFORMATION											
model	operation mode	housing color	options								
MERU-LED	ACEM = General & Emergency Lighting	DB = Dark Bronze	Self-Diagnostics & Photocell (Included Standard)								
	AC = General Lighting	WH = White	IH = Internal Heater								
		BK = Black	PIR = Passive Infra-Red Motion Sensor								
Ordering Example:	MERU-ACEM-DB	NK = Nickel									

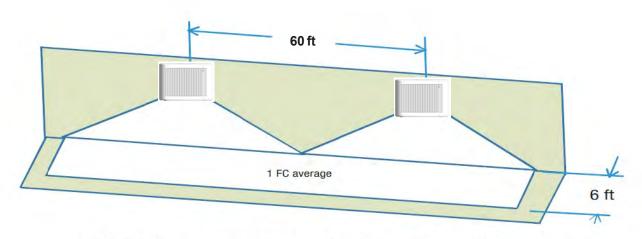
5/1

MERU Series

PROJECT:
FIXTURE TYPE:
LOCATION:
CONTACT/PHONE:

LED GENERAL & EMERGENCY LIGHTING

PHOTOMETRICS

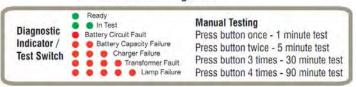


Note: Meets Life Safety Code standard minimum illuminance of 0.1 FC and average illuminance of 1.0 FC. Illustration shown is a guideline for corridor center-to-center with 9 ft mounting height and Minimum 80-50-20 reflectance values.

Mounting Height	Center to center distance
7.2ft	45ft
9ft	60ft
10ft	65ft

SELF DIAGNOSTICS

Included Self Diagnostic



Full self-test, self-diagnostic system is standard in every unit, performs a monthly, test as well as continuously monitoring all functions to ensure reliability, a manual test may be initiated at any time



Michael Cuomo, Soil Scientist

6 York Pond Road, York, Maine 03909 207 363 4532 mcuomosoil@gmail.com

4 August 2022

Wetland Restoration Sequence and Goals

Well Field 44, LLC 41 Route 236, Kittery 2 August 2022

- 1. Two areas are to be restored. The larger is about 10,500 square feet at the westerly side of the project site. The second is about 2,500 square feet at the south easterly corner of the project site.
- 2. The limit of the restoration areas will be marked on the ground with paint. The toe of slope and top of slope will be marked. The slope will be no steeper than 3:1.
- 3. The applicant will secure a capable excavation or landscape contractor to perform the restoration. The contractor will be responsible for DigSafe markout. The wetland scientist will meet with the selected contractor to review the intent and goals of this restoration.
- 4. The contractor will notify the wetland scientist in advance when work will occur. The wetland scientist will make site inspections while the restoration work is taking place.
- 5. The contractor will remove material from filled wetlands to the elevation of the remnant wetland. The work shall be sequenced so the contractor will not enter the wetland with equipment or vehicles. Any buried waste or large trash will be removed from the site and disposed of properly. Any clean fill will be reused on the site or removed from the site and placed legally elsewhere. A slope will be created from the toe of slope to the top of slope painted line.
- 6. Exposed soils within the wetland shall be seeded with one of the following wetland seed mixes at the rate the supplier specifies:

https://newp.com/wp-content/uploads/2018/04/WETMIX2018.pdf

http://www.vermontwetlandplants.com/marsh-swamp-bog-mix/

https://www.stoneyridgeenv.com/environmental-permitting-plants-

- 7. Exposed soils within the wetland shall be mulched with salt marsh hay at the rate of 1 bale of hay per 500 square feet. Estimated wetland area to be seeded and mulched is ± 1.000 square feet.
- 8. 'Erosion Control Mix' or equivalent will be installed as a berm at the re-established wetland boundary toe of slope.

https://www.casella.com/products/earthlife-products/mulch/erosion-control-mix

9. Exposed upland soils on the newly created slope shall be seeded with one of the following and mulched at the rate of 1 bale of hay per 500 square feet.

https://www.tractorsupply.com/tsc/product/barenbrug-k31-plus-clover-40-lb-25694

https://willistonvillagehardware.com/catalog/product/116751/agway-conservation-green-10-lb

- 10. Seed and mulch shall be applied immediately upon finishing of any section of the work. Optimal time of year for seeding is autumn, winter, or spring. Seeding may be delayed depending on when the work is done. Mulching will proceed even if seeding is delayed.
- 11. When the above wetland restoration work is done, the wetland scientist will document with photos and send a report to Attar Engineering and the town.
- 12. A monitoring visit will be made by the wetland scientist about 12 months after finishing the wetland restoration work. If the restored wetlands are dominated by non-invasive species (75% area covered), the restoration will be considered 'complete'. A report with photos will be sent to Attar Engineering and the town.
- 13. Alternately, if the restored wetlands have more than 25% coverage of invasive species, plant-specific treatment by a licensed herbicide applicator will be performed. One month later the wetland scientist will revisit the site and document invasive plant mortality. A report with photos will be sent to Attar Engineering and the town, and the restoration will be considered 'complete'.

 From:
 Mike Sudak

 To:
 "Diane Morabito"

 Cc:
 Ken Wood; Wyatt

Subject: RE: 41 Route 236, Kittery - Additional TIA Request Date: Wednesday, August 3, 2022 12:53:00 PM

Attachments: <u>image001.png</u>

Hi Diane,

We are in agreement on your recommendation and I just cleared it with my Client, so you are welcome to proceed as you have described.

Please let me know if there is anything Attar can do to facilitate this counts study.

Thanks, -Mike

From: Diane Morabito <mordi@sewall.com> **Sent:** Wednesday, August 3, 2022 12:11 PM **To:** Mike Sudak <mike@attarengineering.com>

Subject: RE: 41 Route 236, Kittery - Additional TIA Request

Hi Mike,

The TIA was performed for a larger facility, which would have greater trips. The volumes had high seasonal factors given the time of year the counts were done. I am 99 % certain a right turn lane would not be warranted by the volumes. I recommend that we do new summer counts to obtain actual peak volumes (rather than factoring the previous December volumes) and that we then reassess the need for a left-turn lane (if it meets the warrant) since I think it will be marginal with the lower volumes. It is unlikely MaineDOT would allow it to be built if warrants aren't met. I estimate the cost to do new summer PM counts, re-evaluate LOS and turn lane warrants

This does not include a formal write up for later submission to the town.

I think this is the best approach to take. Let me know if we should proceed.

Diane

Diane W. Morabito, PE, PTOE

Vice President Traffic Engineering
T: +1. 207.817.5440 | F: +1. 207.827.3641 | E: diane.morabito@sewall.com
77 Exchange Street | Suite 401 | Bangor, Maine 04401



From: Mike Sudak < <u>mike@attarengineering.com</u>>

Sent: Wednesday, August 3, 2022 11:28 AM **To:** Diane Morabito <mordi@sewall.com>

Cc: Ken Wood <<u>Ken@attarengineering.com</u>>; Wyatt <<u>wyatt@attarengineering.com</u>>

Subject: 41 Route 236, Kittery - Additional TIA Request

Good Morning Diane,

Thanks again for helping me out with the Eliot Planning Board last week, I'm very pleased to finally have that project moving forward. This is Eliot's first Subdivision Application in years (at least the duration of Planner Brubaker's tenure), so there have certainly been some hurdles to overcome while everyone gets comfortable.

At any rate, I'd like to change subjects to the project in Kittery that you helped Brian with during this past winter/spring that I've now taken over with his departure – the marijuana dispensary on Route 236 near Martin Road. I was before the Planning Board the end of last month discussing this application and have made quite a few changes to the site since you've likely last had eyes on it. To briefly provide overview:

- The one-way exit that was proposed onto the southerly-abutting property for existing trips and overflow parking has been abandoned (after further research, the abutting property has no surplus of parking that we could avail ourselves of).
- We are proposing to remove the existing gravel road entirely, restoring the wetland to its original grade.
- Some additional parking has been added on-site, and the travelway has been designed to allow full circulation back to the proposed entrance onto Route 236.

While this topic has been discussed by the Planning Board throughout the approvals process, now that it is our sole means of egress to and from the site, the proposed entrance is going to be heavily scrutinized by the Town. I have attached an excerpt from your original TIA in December of last year, specifically the section related to turn lanes in the Route 236 corridor.

The Planning Board is of the opinion that they would like to see both a right-hand turn lane for southbound traffic and a left-hand turn lane for northbound traffic to be constructed, if possible. You're welcome to use the attached Grading & Utility Plan as reference, but I am skeptical of the Route 236 corridor in this area having the capacity for such a buildout of even one turning lane, let alone two. This section is guardrailed and has steep slopes in the range of 10'-12' high.

What I'm looking for (maybe someday I'll learn to be less long-winded) is if you think it appropriate to provide an opinion on the constructability of such turning lanes (if one or both are possible/practical). I have engaged the MDOT as this is a State-maintained corridor but have not heard back yet, and in the meantime I wanted to reach out to you and get your opinion.

Take care, talk soon.

-Mike

Michael J. Sudak, E.I.

Civil Engineer

Attar Engineering, Inc.

1284 State Road

Eliot, Maine 03903

Ph: (207) 439-6023

Fax: (207) 439-2128 Cell: (978) 317-3398



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TOWN OF KITTERY, MAINE PLANNING OFFICE

PO Box 808, Kittery, Maine 03904

(207) 439-6807 Ext. 307

Mr. Darren LaPierre Island Marine Service 32 Rt. 236 Kittery, Me. 03904

May 30, 2006

Re: Operation of Boat Yard, Map 29 Lot 44 zoned C-2

Mr. LaPierre.

I have reviewed your revised site plan prepared by Civil Consultants, Inc. of So. Berwick, Me. for your boat yard. As you are probably aware, a boat yard in the C-2 (commercial – 2 zone) is a permitted use.

The narrative you have provided with your application makes the description of use for the property much more clear. Be aware that that the addition of any exterior lighting on the property will have to be compliant with section 16.08.020 Standards for Exterior Lighting.

You may remember our discussion some time ago regarding storage of any type within the flood plane. Storage within the flood plane was prohibited per the approved plan for Interstate Moving and Storage (1994). Storage in this area is to continue to be prohibited.

It does appear that on a seasonal average, this particular use does not exceed the intensity of the previous storage and warehousing business that existed prior to your purchase and use of the property.

Per section 16.28.260 Site Review Threshold, the CEO and I concur that the current use as a boat yard, being a permitted use, and no greater intensity of use than the previous warehousing and storage use, is approved. However, be advised that that this section does require that both the planner and CEO are responsible to assure that this use, (boat yard) is to maintain compliance at all times with the Kittery Land Use and **Development Code/Zoning Ordinance.**

It is your responsibility, as I'm sure you are aware, to maintain all safeguards to prevent any pollutants from your business activities to enter

the adjacent wetlands or allowed to be spilled into the soils. We are therefore requesting a copy of all necessary permits that you have received from the Maine Department of Environmental Protection or any other State agency that is responsible to monitor the use and or disposal of products used in your business such as coolants, chemicals, and petroleum products etc.

Be advised that the only use that you are approved for is that which falls within the definition of boat yard as defined within the Kittery Land Use and Development Code. Any other uses without appropriate approval will be considered a violation of the LUDC. Any additional uses will likely need to be approved by the planning board, zoning board of appeals or both.

Contact me with any questions you have on this matter.

Sincerely

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Jim Noel.

Town Planner

Heather Ross, CEO



TOWN OF KITTERY

Planning Office 200 Rogers Rd. Kittery, Maine 03904 (207) 475-1323

Request For Additional Information

Property Owner:

Lapierre Propertise, LLC

Applicant:

Island Marine Service c/o Darren Lapierre

Mailing Address:

32 Route 236 Kittery, ME 03904

Property Location:

41 Route 236 Kittery, ME 03904

Map Lot:

Map 29 Lot 1

Proposed.

Activity/Project:

Boat Yard and Storage

Date:

. Š

September 29, 2008

Re: Business Occupancy Change

The Planning and Code Enforcement offices have reviewed your application for a boat yard and marine storage located at 41 Route 236.

Please be advised; that the Town Planner and Code Enforcement Officer are in need of additional information for the project cited above in the Commercial 2 District (C2). Specifically, a wetlands delineation performed by a state licensed soil scientist will be needed for the property at 41 Route 236, in order to determine the project is appropriate for this site.

Please provide the requested information to the Planning Office located at the address provided above in order to complete the administrative review for the business occupancy change.

Sincerely,

Michael Asciola Planning Clerk Town of Kittery

Phone: 207-475-1323 Fax: 207-439-6806



DEPARTMENT OF ENVIRONMENTAL PROTECTION BUREAU OF LAND AND WATER OUALITY

1/21/200

7506

CONTACT ID

FIELD DETERMINATION FORM

CONTACT

LAPIERRE

ISLAND MARINE SERVICE

32 ROUTE 236

KITTERY 2074393810

DARREN

ME

03904

PROPERTY OWNER

LAPIERRE, DARREN

ISLAND MARINE SERVICE

KITTERY

ME

03904

. 3

STAFF

COPPI, CHRIS

RESOURCE FW

DIRECTIONS

Rt 95 south to Rt 236 exit in Kittery. Follow Rt 236 north for less than 1/4 of a mile from the turnpike to Island Marine Service on the right.

SITE TOWN KITTERY

1

MAP LOT

29

MEMO

On January 20, 2009, Darren Lapierre and Chris Coppi met on site at 32 Route 236 for a wetland setback determination and to discuss permitting requirements for a proposed boat storage area. The property contains a freshwater wetland with less than 20,000 square feet of aquatic vegetation, emergent marsh vegetation or open water as defined by the Natural Resources Protection Act (NRPA).

Island Marine Service is proposing to clear, strip, and grade 2 acres of upland area including areas to the wetland edge. The entire 2 acre area will be revegetated with grass. Less than 1 acre of this area will be used for the access road, and associated turnaround areas. The remaining area will be used for seasonal boat storage. A stormwater permit by rule is required for this activity which includes submitting a erosion control and site plan to scale indicating the location of the access road, turnaround area, and storage area.

Under the NRPA, the Department regulates activities in the wetland but not within 75 feet of the edge of the wetland. The property qualifies for a one-time only wetland alteration exemption of no more than 4,300 square feet of alteration which can include removing vegetation and placing fill material in the wetland. A NRPA permit will be required for wetland alterations exceeding this amount.

NAME:

RECEIVED

1/15/2009

SITE VISIT

Mrs Com 1/21/09

1/20/2009

COMPLETED

1/21/2009



TOWN OF KITTERY

Planning Office 200 Rogers Rd. Kittery, Maine 03904 (207) 475-1323

NOTICE OF DECISION

Property Owner:

Lapierre Properties, LLC

Applicant:

Island Marine Service c/o Darren Lapierre

Mailing Address:

32 Route 236 Kittery, ME 03904

Property Location:

41 Route 236 Kittery, ME 03904

Map Lot:

Map 29 Lot 1

Proposed

Activity/Project:

Boat Yard and Storage

Date:

. Š

February 19, 2009

Re: Business Occupancy Change

The Planning and Code Enforcement offices have reviewed your latest application for Business Occupancy Change for the property located at 41 Route 236, as an expansion of your existing business, Island Marine Services, by developing this property as a boat yard and marine storage.

Your latest submission received on February 17, 2009 included a site plan with wetlands sketch prepared by Attar Engineering, Inc. and correspondences from the State of Maine, Department of Environmental Protection (DEP). A DEP Field Determination Form prepared by Chris Coppi, describes a site visit on January 20, 2009, and a one-time approval to alter a 4,300 square foot wetland. The report also details project requirements set forth by DEP.

Please be advised; that the Town Planner and Code Enforcement Officer approve your Business Occupancy Change for the project cited above in the Commercial 2 District (C2) based upon the following findings:

- Your proposed project is in compliance with zoning regulations for the Commercial 2 District (C2).
- You are working with DEP develop your project in compliance with state and local wetlands and environmental law.

Review Threshold - In accordance with Section 16.16.020(E) the Land Use and development Code, the Town Planner and Code Enforcement Officer shall review and approve or refer the applicant to the Planning Board.

This approval of Change of Business Occupancy IS NOT a building permit or a sign permit.

Any proposed field changes, diversion from or revisions to the plans and construction documents after approval shall be reported to the Code Enforcement Officer prior to proceeding with the proposed changes. Any site changes not approved in this Notice of Decision will be in violation of State law and Town ordinances.

Sincerely.

Sandra Movery, P.E.

Town Planner

Shelly Bishop, L.P.I

Assistant Code Officer



TOWN OF KITTERY

Planning Office 200 Rogers Rd. Kittery, Maine 03904 (207) 475-1323

NOTICE OF DECISION AMENDMENT

Property Owner:

Lapierre Properties, LLC

Applicant:

Island Marine Service c/o Darren Lapierre

Mailing Address:

32 Route 236 Kittery, ME 03904

Property Location:

41 Route 236 Kittery, ME 03904

Map Lot:

Map 29 Lot 1

Proposed

Activity/Project:

Boat Yard and Storage

Date:

. 5

February 24, 2009

Re: Business Occupancy Change

This letter is in follow-up to our earlier phone conversation. Upon further review of your site plan submitted February 17, 2009, the Planning and Development Office discovered that the proposed 30 foot buffer area abutting the residential lots on Martin Rd. will need to be expanded to 40 feet, in order to comply with the town ordinance. The following ordinance sections related to buffering standards are applicable to your project.

"16.32.430 Buffer areas.

Any nonresidential yard setback space abutting an existing or potential residential area shall be maintained as a buffer strip by the developer. Such buffer area shall be for the purpose of eliminating any adverse effects upon the environmental or aesthetic qualities of abutting properties or any type of nuisance affecting the health, safety, welfare and property values of the residents of Kittery. (Land use and dev. code § 8.10.3, 1994)"

"16.12.110 Commercial—C.

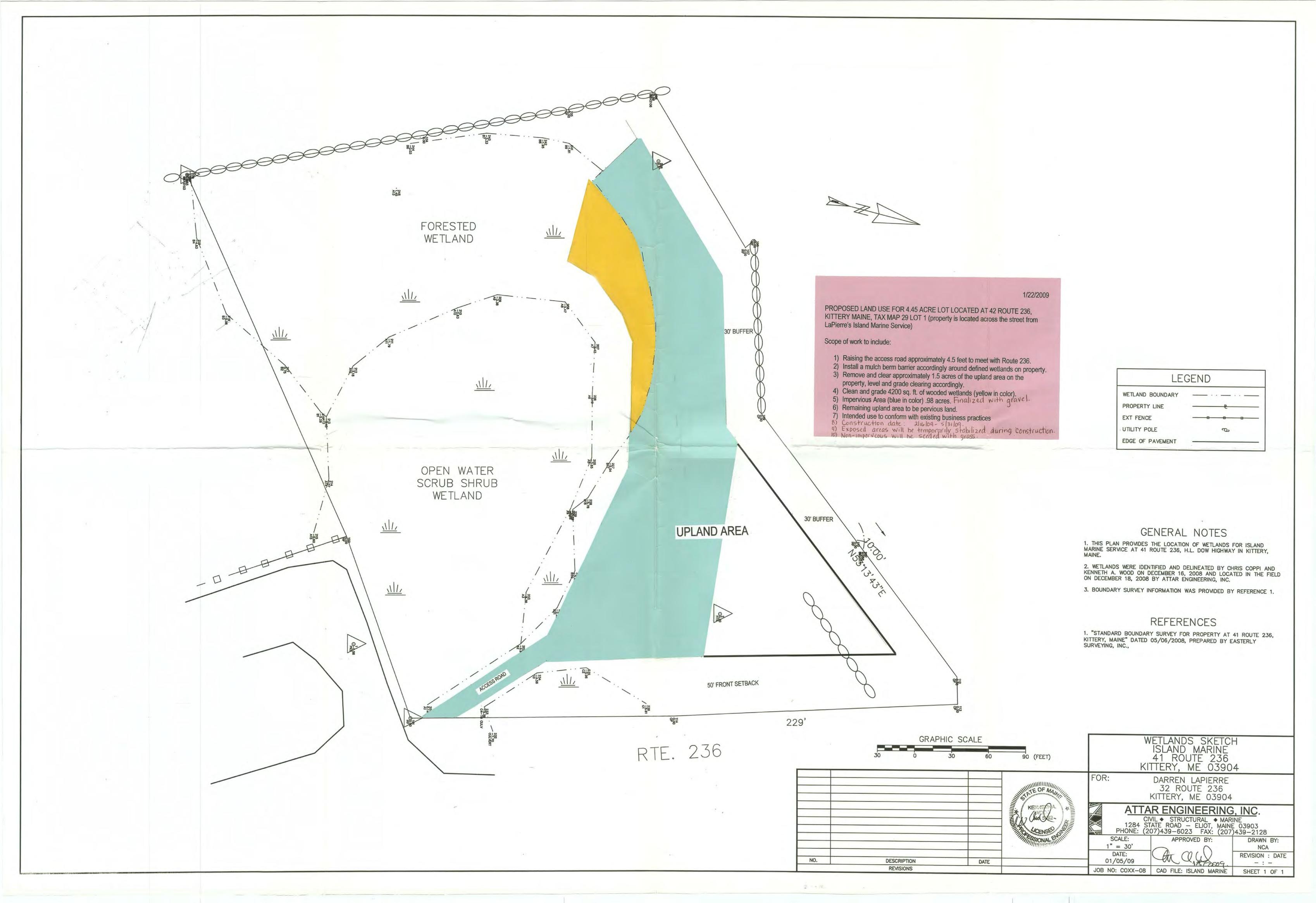
In our phone conversation this afternoon you were informed of this information and acknowledged you would extend the buffer area from 30 feet to 40 feet. We are not requesting that you submit a revised site plan to the office for review for this minor adjustment at this time. However, if this adjustment causes you to further alter your site plan we will need to be informed and may require you to submit a revised site plan. Any site changes not approved in this Notice of Decision will be in violation of state law and town ordinances.

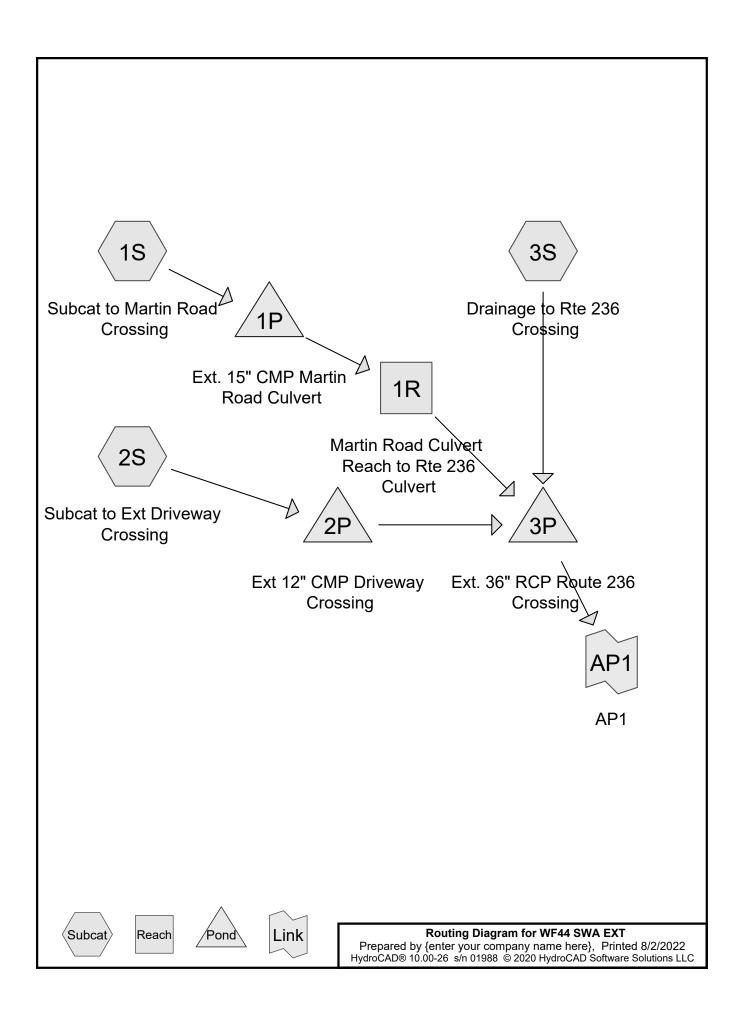
Sincerely,

May I Myry Bandra Mowery, F.E.

Heather Ross

Code Enforcement Officer





Printed 8/2/2022 Page 2

Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
2.306	46	2 acre lots, 12% imp, HSG A (1S, 2S)
4.258	77	2 acre lots, 12% imp, HSG C (1S, 2S)
54.930	82	2 acre lots, 12% imp, HSG D (1S, 2S)
4.883	77	Brush, Fair, HSG D (1S, 2S)
0.955	96	Gravel surface, HSG D (2S, 3S)
0.533	98	Paved parking, HSG C (1S, 2S)
4.384	98	Paved parking, HSG D (1S, 2S, 3S)
0.344	36	Woods, Fair, HSG A (1S)
0.689	73	Woods, Fair, HSG C (1S)
17.356	79	Woods, Fair, HSG D (1S, 2S, 3S)
4.517	82	Woods/grass comb., Fair, HSG D (3S)
95.156	81	TOTAL AREA

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Page 3

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Subcat to Martin Runoff Area=1,905,022 sf 12.81% Impervious Runoff Depth>1.37"

Flow Length=2,053' Tc=38.0 min CN=80 Runoff=38.51 cfs 5.002 af

Subcatchment 2S: Subcat to Ext Runoff Area=1,929,172 sf 12.87% Impervious Runoff Depth>1.43"

Flow Length=3,194' Tc=60.4 min CN=81 Runoff=31.70 cfs 5.262 af

Subcatchment 3S: Drainage to Rte 236 Runoff Area=310,792 sf 13.95% Impervious Runoff Depth>1.66"

Flow Length=1,095' Tc=17.3 min CN=84 Runoff=10.66 cfs 0.988 af

Reach 1R: Martin Road Culvert Reach to Avg. Flow Depth=0.07' Max Vel=0.39 fps Inflow=1.31 cfs 0.677 af

n=0.070 L=744.0' S=0.0121'/' Capacity=1,793.41 cfs Outflow=1.30 cfs 0.564 af

Pond 1P: Ext. 15" CMP Martin Road Peak Elev=49.63' Storage=307,858 cf Inflow=38.51 cfs 5.002 af

15.0" Round Culvert n=0.013 L=50.0' S=0.0100 '/' Outflow=1.31 cfs 0.677 af

Pond 2P: Ext 12" CMP Driveway Crossing Peak Elev=40.81' Storage=251,020 cf Inflow=31.70 cfs 5.262 af

12.0" Round Culvert n=0.013 L=30.0' S=0.0167 '/' Outflow=1.65 cfs 0.834 af

Pond 3P: Ext. 36" RCP Route 236 Crossing Peak Elev=40.57' Storage=11,849 cf Inflow=10.69 cfs 2.387 af

36.0" Round Culvert n=0.011 L=100.0' S=0.0050 '/' Outflow=6.79 cfs 2.270 af

Link AP1: AP1 Inflow=6.79 cfs 2.270 af

Primary=6.79 cfs 2.270 af

Total Runoff Area = 95.156 ac Runoff Volume = 11.252 af Average Runoff Depth = 1.42" 87.08% Pervious = 82.859 ac 12.92% Impervious = 12.297 ac Prepared by {enter your company name here}

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Page 4

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Subcat to Martin Runoff Area=1,905,022 sf 12.81% Impervious Runoff Depth>2.95"

Flow Length=2,053' Tc=38.0 min CN=80 Runoff=82.60 cfs 10.765 af

Subcatchment 2S: Subcat to Ext Runoff Area=1,929,172 sf 12.87% Impervious Runoff Depth>3.02"

Flow Length=3,194' Tc=60.4 min CN=81 Runoff=66.98 cfs 11.159 af

Subcatchment 3S: Drainage to Rte 236 Runoff Area=310,792 sf 13.95% Impervious Runoff Depth>3.36" Flow Length=1,095' Tc=17.3 min CN=84 Runoff=21.18 cfs 1.995 af

Reach 1R: Martin Road Culvert Reach to Avg. Flow Depth=0.12' Max Vel=0.55 fps Inflow=3.23 cfs 1.848 af n=0.070 L=744.0' S=0.0121 '/' Capacity=1,793.41 cfs Outflow=3.23 cfs 1.645 af

Pond 1P: Ext. 15" CMP Martin Road Peak Elev=50.10' Storage=508,857 cf Inflow=82.60 cfs 10.765 af 15.0" Round Culvert n=0.013 L=50.0' S=0.0100 '/' Outflow=3.23 cfs 1.848 af

Pond 2P: Ext 12" CMP Driveway Crossing Peak Elev=41.55' Storage=468,716 cf Inflow=66.98 cfs 11.159 af 12.0" Round Culvert n=0.013 L=30.0' S=0.0167 '/' Outflow=3.06 cfs 1.733 af

Pond 3P: Ext. 36" RCP Route 236 Crossing Peak Elev=41.07' Storage=21,205 cf Inflow=21.46 cfs 5.374 af 36.0" Round Culvert n=0.011 L=100.0' S=0.0050 '/' Outflow=13.31 cfs 5.153 af

Link AP1: AP1Inflow=13.31 cfs 5.153 af
Primary=13.31 cfs 5.153 af

Total Runoff Area = 95.156 ac Runoff Volume = 23.919 af Average Runoff Depth = 3.02" 87.08% Pervious = 82.859 ac 12.92% Impervious = 12.297 ac Prepared by {enter your company name here}
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Page 1

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Subcat to Martin Runoff Area=1,905,022 sf 12.81% Impervious Runoff Depth>4.02" Flow Length=2,053' Tc=38.0 min CN=80 Runoff=111.47 cfs 14.658 af

Subcatchment 2S: Subcat to Ext

Runoff Area=1,929,172 sf 12.87% Impervious Runoff Depth>4.10"

Flow Length=3,194' Tc=60.4 min CN=81 Runoff=89.98 cfs 15.120 af

Subcatchment 3S: Drainage to Rte 236 Runoff Area=310,792 sf 13.95% Impervious Runoff Depth>4.47" Flow Length=1,095' Tc=17.3 min CN=84 Runoff=27.89 cfs 2.660 af

Reach 1R: Martin Road Culvert Reach to Avg. Flow Depth=0.13' Max Vel=0.60 fps Inflow=4.14 cfs 2.454 af n=0.070 L=744.0' S=0.0121 '/' Capacity=1,793.41 cfs Outflow=4.14 cfs 2.219 af

Pond 1P: Ext. 15" CMP Martin Road Peak Elev=50.41' Storage=652,211 cf Inflow=111.47 cfs 14.658 af 15.0" Round Culvert n=0.013 L=50.0' S=0.0100 '/' Outflow=4.14 cfs 2.454 af

Pond 2P: Ext 12" CMP Driveway Crossing Peak Elev=42.01' Storage=623,596 cf Inflow=89.98 cfs 15.120 af 12.0" Round Culvert n=0.013 L=30.0' S=0.0167 '/' Outflow=3.67 cfs 2.138 af

Pond 3P: Ext. 36" RCP Route 236 Crossing Peak Elev=41.34' Storage=27,795 cf Inflow=28.55 cfs 7.017 af 36.0" Round Culvert n=0.011 L=100.0' S=0.0050 '/' Outflow=17.45 cfs 6.745 af

Link AP1: AP1Inflow=17.45 cfs 6.745 af
Primary=17.45 cfs 6.745 af

Total Runoff Area = 95.156 ac Runoff Volume = 32.438 af Average Runoff Depth = 4.09" 87.08% Pervious = 82.859 ac 12.92% Impervious = 12.297 ac

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Summary for Subcatchment 1S: Subcat to Martin Road Crossing

Runoff = 111.47 cfs @ 12.52 hrs, Volume= 14.658 af, Depth> 4.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 YEAR STORM Rainfall=6.60"

A	rea (sf)	CN E	Description			
	13,186	98 F	Paved parking, HSG C			
	84,109	98 F	Paved parking, HSG D			
	24,295	46 2	2 acre lots, 12% imp, HSG A			
	14,995	36 V	Woods, Fair, HSG A			
1	57,513		2 acre lots, 12% imp, HSG C			
	30,028	73 V	Woods, Fair, HSG C			
	77,718		Brush, Fair, HSG D			
	62,281		Voods, Fai			
	40,897		acre lots,	12% imp, F	HSG D	
,	05,022		Veighted A			
,	61,002	_	-	vious Area		
2	244,020	1	2.81% Imp	pervious Ar	ea	
_		٥.				
Tc	Length	Slope			Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
7.1	50	0.0750	0.12		Sheet Flow, SF 1	
					Woods: Light underbrush n= 0.400 P2= 3.33"	
18.4	875	0.0251	0.79		Shallow Concentrated Flow, SCF 1	
					Woodland Kv= 5.0 fps	
12.5	1,128	0.0022	1.50	1,953.86		
					Area= 1,300.0 sf Perim= 701.0' r= 1.85'	
					n= 0.070 Sluggish weedy reaches w/pools	
38.0	2,053	Total				

2,000 Total

Summary for Subcatchment 2S: Subcat to Ext Driveway Crossing

Runoff = 89.98 cfs @ 12.80 hrs, Volume= 15.120 af, Depth> 4.10"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 YEAR STORM Rainfall=6.60"

Area (sf)	CN	Description
76,151	46	2 acre lots, 12% imp, HSG A
27,952	77	2 acre lots, 12% imp, HSG C
10,053	98	Paved parking, HSG C
31,291	96	Gravel surface, HSG D
63,490	98	Paved parking, HSG D
34,997	77	Brush, Fair, HSG D
333,371	79	Woods, Fair, HSG D
1,351,867	82	2 acre lots, 12% imp, HSG D
1,929,172	81	Weighted Average
1,680,913		87.13% Pervious Area
248,259		12.87% Impervious Area

Prepared by {enter your company name here}

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	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	12.1	50	0.0200	0.07		Sheet Flow, SF 1
						Woods: Light underbrush n= 0.400 P2= 3.33"
	27.7	1,393	0.0281	0.84		Shallow Concentrated Flow, SCF 1
						Woodland Kv= 5.0 fps
	20.6	1,751	0.0057	1.42	708.86	Channel Flow, CF 1
						Area= 500.0 sf Perim= 601.0' r= 0.83'
_						n= 0.070 Sluggish weedy reaches w/pools
	60.4	3 194	Total			

Summary for Subcatchment 3S: Drainage to Rte 236 Crossing

Runoff = 27.89 cfs @ 12.23 hrs, Volume= 2.660 af, Depth> 4.47"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 YEAR STORM Rainfall=6.60"

A	rea (sf)	CN D	escription		
	10,297	96 G	Gravel surfa)	
	43,366	98 P	aved park	ing, HSG D	
	60,382		Voods, Fai		
1	96,747	82 V	Voods/gras	ss comb., F	air, HSG D
3	10,792		Veighted A		
	67,426	_		vious Area	
	43,366	1	3.95% lmp	pervious Are	ea
_		01			
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
7.8	50	0.0600	0.11		Sheet Flow, SF 1
					Woods: Light underbrush n= 0.400 P2= 3.33"
4.9	351	0.0569	1.19		Shallow Concentrated Flow, SCF 1
					Woodland Kv= 5.0 fps
4.6	694	0.0094	2.49	995.12	
					Area= 400.0 sf Perim= 301.0' r= 1.33'
					n= 0.070 Sluggish weedy reaches w/pools
17.3	1,095	Total			

Summary for Reach 1R: Martin Road Culvert Reach to Rte 236 Culvert

[79] Warning: Submerged Pond 1P Primary device # 1 OUTLET by 0.13'

Inflow Area = 43.733 ac, 12.81% Impervious, Inflow Depth > 0.67" for 25 YEAR STORM event

Inflow = 4.14 cfs @ 18.87 hrs, Volume= 2.454 af

Outflow = 4.14 cfs @ 19.45 hrs, Volume= 2.219 af, Atten= 0%, Lag= 34.9 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity = 0.60 fps, Min. Travel Time = 20.6 min Avg. Velocity = 0.45 fps, Avg. Travel Time = 27.7 min

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Peak Storage= 5,114 cf @ 19.11 hrs

Average Depth at Peak Storage= 0.13'

Pank Full Depth= 4.00', Flow Area = 4.00 of Cana

Bank-Full Depth= 4.00' Flow Area= 400.0 sf, Capacity= 1,793.41 cfs

50.00' x 4.00' deep channel, n= 0.070 Sluggish weedy reaches w/pools

Side Slope Z-value= 12.5 '/' Top Width= 150.00'

Length= 744.0' Slope= 0.0121 '/'

Inlet Invert= 48.50', Outlet Invert= 39.50'



Summary for Pond 1P: Ext. 15" CMP Martin Road Culvert

Inflow Area = 43.733 ac, 12.81% Impervious, Inflow Depth > 4.02" for 25 YEAR STORM event

Inflow = 111.47 cfs @ 12.52 hrs, Volume= 14.658 af

Outflow = 4.14 cfs @ 18.87 hrs, Volume= 2.454 af, Atten= 96%, Lag= 381.1 min

Primary = 4.14 cfs @ 18.87 hrs, Volume= 2.454 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Starting Elev= 49.00' Surf.Area= 231,086 sf Storage= 119,579 cf

Peak Elev= 50.41' @ 18.87 hrs Surf.Area= 469,035 sf Storage= 652,211 cf (532,632 cf above start)

Flood Elev= 52.50' Surf.Area= 556,200 sf Storage= 1,713.635 cf (1,594,056 cf above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= 178.3 min (981.3 - 803.0)

Volume	Inve	<u>ert Avail.Sto</u>	rage Storage	Description	
#1	48.0	00' 1,999,1	2 cf Custom Stage Data (Pr		rismatic)Listed below (Recalc)
Elevation (fee			Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
48.00 8, 50.00 454, 52.00 526,		8,072 454,100 526,650 585,750	0 462,172 980,750 556,200	462,172 1,442,922 1,999,122	
Device	Routing	Invert	Outlet Devices	S	
#1 Primary 49.00'		15.0" Round CMP_Round 15" L= 50.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 49.00' / 48.50' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf			

Primary OutFlow Max=4.14 cfs @ 18.87 hrs HW=50.41' (Free Discharge) 1=CMP_Round 15" (Inlet Controls 4.14 cfs @ 3.37 fps)

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Summary for Pond 2P: Ext 12" CMP Driveway Crossing

Inflow Area = 44.288 ac, 12.87% Impervious, Inflow Depth > 4.10" for 25 YEAR STORM event

Inflow = 89.98 cfs @ 12.80 hrs, Volume= 15.120 af

Outflow = 3.67 cfs @ 20.00 hrs, Volume= 2.138 af, Atten= 96%, Lag= 431.7 min

Primary = 3.67 cfs @ 20.00 hrs, Volume= 2.138 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Starting Elev= 40.00' Surf.Area= 210,051 sf Storage= 58,153 cf

Peak Elev= 42.01' @ 20.00 hrs Surf.Area= 351,062 sf Storage= 623,596 cf (565,443 cf above start)

Flood Elev= 43.00' Surf.Area= 406,736 sf Storage= 996,917 cf (938,764 cf above start)

Plug-Flow detention time= 526.8 min calculated for 0.801 af (5% of inflow)

Center-of-Mass det. time= 165.7 min (984.4 - 818.8)

Volume	Invert	Avail.Storage	Storage Description
#1	39.50'	1,431,905 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
39.50	22,560	0	0
40.00	210,051	58,153	58,153
42.00	350,230	560,281	618,434
44.00	463,241	813,471	1,431,905

Device	Routing	Invert	Outlet Devices
#1	Primary	40.00'	12.0" Round CMP_Round 12"

L= 30.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 40.00' / 39.50' S= 0.0167 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=3.67 cfs @ 20.00 hrs HW=42.01' (Free Discharge) 1=CMP_Round 12" (Inlet Controls 3.67 cfs @ 4.68 fps)

Summary for Pond 3P: Ext. 36" RCP Route 236 Crossing

[62] Hint: Exceeded Reach 1R OUTLET depth by 1.81' @ 12.45 hrs

[81] Warning: Exceeded Pond 2P by 0.80' @ 12.40 hrs

Inflow Area = 95.156 ac, 12.92% Impervious, Inflow Depth > 0.88" for 25 YEAR STORM event

Inflow = 28.55 cfs @ 12.24 hrs, Volume= 7.017 af

Outflow = 17.45 cfs @ 12.49 hrs, Volume= 6.745 af, Atten= 39%, Lag= 15.4 min

Primary = 17.45 cfs @ 12.49 hrs, Volume= 6.745 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Starting Elev= 39.50' Surf.Area= 5,400 sf Storage= 2,210 cf

Peak Elev= 41.34' @ 12.49 hrs Surf.Area= 25,988 sf Storage= 27,795 cf (25,585 cf above start)

Flood Elev= 49.50' Surf.Area= 70,300 sf Storage= 153,260 cf (151,050 cf above start)

Plug-Flow detention time= 29.2 min calculated for 6.672 af (95% of inflow)

Center-of-Mass det. time= 12.7 min (924.3 - 911.6)

Type III 24-hr 25 YEAR STORM Rainfall=6.60"

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Volume Invert Avail.Sto		orage Storage	Description				
#1 39.00' 153,260		60 cf Custom	0 cf Custom Stage Data (Prismatic)Listed below (Recalc)				
		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)			
39.0	00	3,440	0	0			
40.0	00	7,360	5,400	5,400			
42.0	00	35,100	42,460	47,860			
44.0	00	70,300	105,400	153,260			
Device	Routing	Invert	Outlet Devices	3			
#1 Primary		39.50'	36.0" Round	36.0" Round RCP_Round 36"			
	•		L= 100.0' RC	P, sq.cut end p	projecting, Ke= 0.500		
			Inlet / Outlet Ir	nvert= 39.50' / 3	39.00' S= 0.0050 '/' Cc= 0.900		
			n= 0.011 Con	crete pipe, stra	ight & clean, Flow Area= 7.07 sf		

Primary OutFlow Max=17.43 cfs @ 12.49 hrs HW=41.34' (Free Discharge) 1=RCP_Round 36" (Barrel Controls 17.43 cfs @ 5.48 fps)

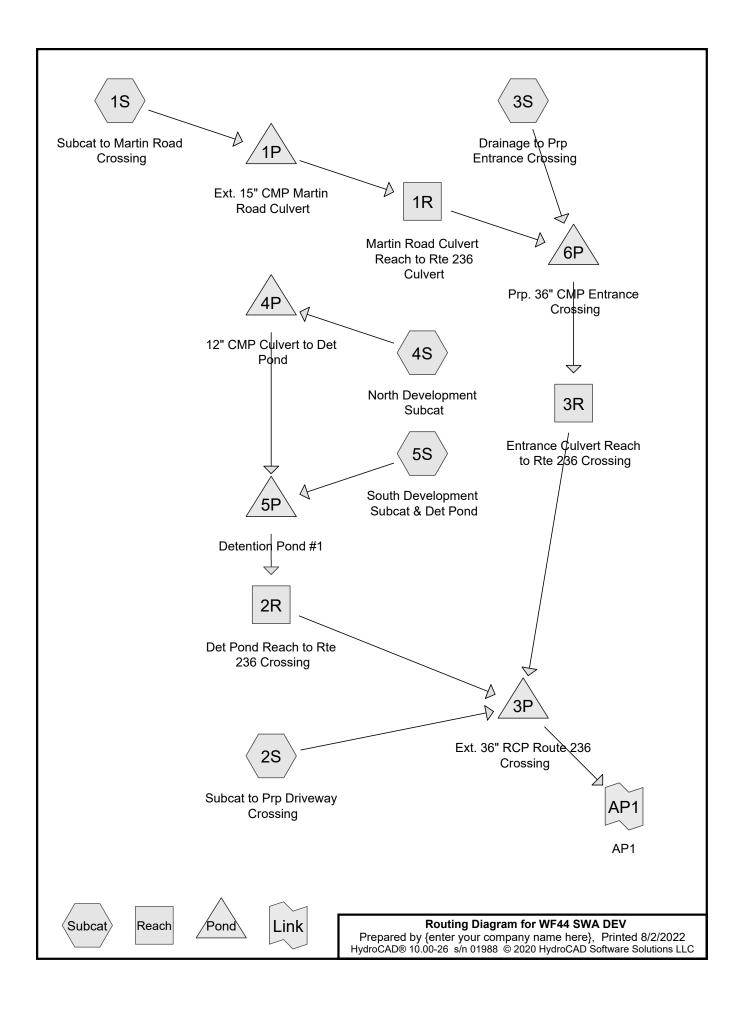
Summary for Link AP1: AP1

Inflow Area = 95.156 ac, 12.92% Impervious, Inflow Depth > 0.85" for 25 YEAR STORM event

Inflow = 17.45 cfs @ 12.49 hrs, Volume= 6.745 af

Primary = 17.45 cfs @ 12.49 hrs, Volume= 6.745 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Area Listing (all nodes)

Area	CN	Description	
(acres)		(subcatchment-numbers)	
2.306	46	2 acre lots, 12% imp, HSG A (1S, 2S)	
4.258	77	2 acre lots, 12% imp, HSG C (1S, 2S)	
55.016	82	2 acre lots, 12% imp, HSG D (1S, 2S)	
0.540	80	>75% Grass cover, Good, HSG D (2S, 3S, 4S, 5S)	
4.883	77	Brush, Fair, HSG D (1S, 2S)	
0.018	96	Gravel surface, HSG D (3S)	
0.533	98	Paved parking, HSG C (1S, 2S)	
4.719	98	Paved parking, HSG D (1S, 2S, 3S, 4S, 5S)	
0.012	98	Roofs, HSG D (4S, 5S)	
0.344	36	Woods, Fair, HSG A (1S)	
0.689	73	Woods, Fair, HSG C (1S)	
17.667	79	Woods, Fair, HSG D (1S, 2S, 3S)	
4.171	82	Woods/grass comb., Fair, HSG D (3S)	
95.156	81	TOTAL AREA	

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Subcat to Martin

Runoff Area=1,905,022 sf 12.81% Impervious Runoff Depth>1.37"

Flow Length=2,053' Tc=33.8 min CN=80 Runoff=40.72 cfs 5.010 af

Subcatchment 2S: Subcat to Prp

Runoff Area=1,946,168 sf 13.28% Impervious Runoff Depth>1.43"
Flow Length=3,232' Tc=61.6 min CN=81 Runoff=31.53 cfs 5.306 af

Subcatchment 3S: Drainage to Prp

Runoff Area=278,095 sf 14.10% Impervious Runoff Depth>1.66"
Flow Length=880' Tc=15.9 min CN=84 Runoff=9.88 cfs 0.885 af

Subcatchment 4S: North DevelopmentRunoff Area=7,250 sf 64.19% Impervious Runoff Depth>2.34"
Flow Length=48' Slope=0.0950 '/' Tc=2.9 min CN=92 Runoff=0.51 cfs 0.032 af

Subcatchment 5S: South Development Runoff Area=8,451 sf 57.92% Impervious Runoff Depth>2.16" Flow Length=44' Slope=0.1000 '/' Tc=2.6 min CN=90 Runoff=0.56 cfs 0.035 af

Reach 1R: Martin Road Culvert Reach to Avg. Flow Depth=0.06' Max Vel=0.43 fps Inflow=1.31 cfs 0.685 af n=0.070 L=529.0' S=0.0161 '/' Capacity=2,066.93 cfs Outflow=1.30 cfs 0.611 af

Reach 2R: Det Pond Reach to Rte 236 Avg. Flow Depth=0.02' Max Vel=0.08 fps Inflow=0.67 cfs 0.052 af n=0.070 L=226.0' S=0.0011 '/' Capacity=1,761.91 cfs Outflow=0.25 cfs 0.050 af

Reach 3R: Entrance Culvert Reach to Avg. Flow Depth=0.24' Max Vel=0.46 fps Inflow=5.90 cfs 1.417 af n=0.070 L=147.0' S=0.0034 '/' Capacity=950.98 cfs Outflow=5.81 cfs 1.387 af

Pond 1P: Ext. 15" CMP Martin Road Peak Elev=49.63' Storage=307,905 cf Inflow=40.72 cfs 5.010 af 15.0" Round Culvert n=0.013 L=50.0' S=0.0100 '/' Outflow=1.31 cfs 0.685 af

Pond 3P: Ext. 36" RCP Route 236 CrossingPeak Elev=40.34' Storage=242,182 cf Inflow=36.80 cfs 6.743 af 36.0" Round Culvert n=0.013 L=100.0' S=0.0050 '/' Outflow=4.03 cfs 2.120 af

Pond 4P: 12" CMP Culvert to Det Pond Peak Elev=41.38' Storage=87 cf Inflow=0.51 cfs 0.032 af 12.0" Round Culvert n=0.013 L=50.0' S=0.0100 '/' Outflow=0.46 cfs 0.032 af

Pond 5P: Detention Pond #1 Peak Elev=40.98' Storage=1,021 cf Inflow=1.00 cfs 0.067 af Primary=0.67 cfs 0.052 af Secondary=0.00 cfs 0.000 af Outflow=0.67 cfs 0.052 af

Pond 6P: Prp. 36" CMP Entrance Crossing Peak Elev=41.43' Storage=10,742 cf Inflow=9.89 cfs 1.496 af 36.0" Round Culvert n=0.013 L=40.0' S=0.0125 '/' Outflow=5.90 cfs 1.417 af

Link AP1: AP1Inflow=4.03 cfs 2.120 af

Primary=4.03 cfs 2.120 af

Total Runoff Area = 95.156 ac Runoff Volume = 11.268 af Average Runoff Depth = 1.42" 86.70% Pervious = 82.502 ac 13.30% Impervious = 12.653 ac HydroCAD® 10.00-26 s/n 01988 © 2020 HydroCAD Software Solutions LLC

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Subcat to Martin Runoff Area=1,905,022 sf 12.81% Impervious Runoff Depth>2.96"

Flow Length=2,053' Tc=33.8 min CN=80 Runoff=87.41 cfs 10.780 af

Subcatchment 2S: Subcat to Prp Runoff Area=1,946,168 sf 13.28% Impervious Runoff Depth>3.02"

Flow Length=3,232' Tc=61.6 min CN=81 Runoff=66.73 cfs 11.252 af

Subcatchment 3S: Drainage to Prp Runoff Area=278,095 sf 14.10% Impervious Runoff Depth>3.36" Flow Length=880' Tc=15.9 min CN=84 Runoff=19.64 cfs 1.786 af

Subcatchment 4S: North Development Runoff Area=7,250 sf 64.19% Impervious Runoff Depth>4.18"

Flow Length=48' Slope=0.0950 '/' Tc=2.9 min CN=92 Runoff=0.88 cfs 0.058 af

Subcatchment 5S: South Development Runoff Area=8,451 sf 57.92% Impervious Runoff Depth>3.98"

Flow Length=44' Slope=0.1000 '/' Tc=2.6 min CN=90 Runoff=1.00 cfs 0.064 af

Reach 1R: Martin Road Culvert Reach to Avg. Flow Depth=0.10' Max Vel=0.60 fps Inflow=3.23 cfs 1.865 af n=0.070 L=529.0' S=0.0161 '/' Capacity=2,066.93 cfs Outflow=3.23 cfs 1.734 af

Reach 2R: Det Pond Reach to Rte 236 Avg. Flow Depth=0.03' Max Vel=0.08 fps Inflow=1.16 cfs 0.107 af n=0.070 L=226.0' S=0.0011 '/' Capacity=1,761.91 cfs Outflow=0.55 cfs 0.103 af

Reach 3R: Entrance Culvert Reach to Avg. Flow Depth=0.36' Max Vel=0.60 fps Inflow=11.98 cfs 3.374 af n=0.070 L=147.0' S=0.0034 '/' Capacity=950.98 cfs Outflow=11.87 cfs 3.322 af

Pond 1P: Ext. 15" CMP Martin Road Peak Elev=50.10' Storage=508,855 cf Inflow=87.41 cfs 10.780 af 15.0" Round Culvert n=0.013 L=50.0' S=0.0100 '/' Outflow=3.23 cfs 1.865 af

Pond 3P: Ext. 36" RCP Route 236 Peak Elev=40.96' Storage=441,068 cf Inflow=77.19 cfs 14.677 af 36.0" Round Culvert n=0.013 L=100.0' S=0.0050 '/' Outflow=11.10 cfs 6.121 af

Pond 4P: 12" CMP Culvert to Det Pond Peak Elev=41.52' Storage=128 cf Inflow=0.88 cfs 0.058 af 12.0" Round Culvert n=0.013 L=50.0' S=0.0100 '/' Outflow=0.80 cfs 0.058 af

Pond 5P: Detention Pond #1 Peak Elev=41.23' Storage=1,331 cf Inflow=1.77 cfs 0.122 af Primary=1.16 cfs 0.107 af Secondary=0.00 cfs 0.000 af Outflow=1.16 cfs 0.107 af

Pond 6P: Prp. 36" CMP Entrance Crossing Peak Elev=41.91' Storage=18,997 cf Inflow=19.73 cfs 3.521 af 36.0" Round Culvert n=0.013 L=40.0' S=0.0125 '/' Outflow=11.98 cfs 3.374 af

Link AP1: AP1Inflow=11.10 cfs 6.121 af
Primary=11.10 cfs 6.121 af

Total Runoff Area = 95.156 ac Runoff Volume = 23.941 af Average Runoff Depth = 3.02" 86.70% Pervious = 82.502 ac 13.30% Impervious = 12.653 ac HydroCAD® 10.00-26 s/n 01988 © 2020 HydroCAD Software Solutions LLC

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Subcat to Martin Runoff Area=1,905,022 sf 12.81% Impervious Runoff Depth>4.03" Flow Length=2,053' Tc=33.8 min CN=80 Runoff=118.01 cfs 14.678 af

Subcatchment 2S: Subcat to Prp

Runoff Area=1,946,168 sf 13.28% Impervious Runoff Depth>4.10"

Flow Length=3,232' Tc=61.6 min CN=81 Runoff=89.68 cfs 15.247 af

Subcatchment 3S: Drainage to PrpRunoff Area=278,095 sf 14.10% Impervious Runoff Depth>4.48"
Flow Length=880' Tc=15.9 min CN=84 Runoff=25.86 cfs 2.381 af

Subcatchment 4S: North DevelopmentRunoff Area=7,250 sf 64.19% Impervious Runoff Depth>5.34"
Flow Length=48' Slope=0.0950 '/' Tc=2.9 min CN=92 Runoff=1.12 cfs 0.074 af

Subcatchment 5S: South DevelopmentRunoff Area=8,451 sf 57.92% Impervious Runoff Depth>5.14"
Flow Length=44' Slope=0.1000 '/' Tc=2.6 min CN=90 Runoff=1.27 cfs 0.083 af

Reach 1R: Martin Road Culvert Reach to Avg. Flow Depth=0.12' Max Vel=0.65 fps Inflow=4.14 cfs 2.476 af n=0.070 L=529.0' S=0.0161 '/' Capacity=2,066.93 cfs Outflow=4.14 cfs 2.323 af

Reach 2R: Det Pond Reach to Rte 236 Avg. Flow Depth=0.04' Max Vel=0.09 fps Inflow=1.39 cfs 0.142 af n=0.070 L=226.0' S=0.0011 '/' Capacity=1,761.91 cfs Outflow=0.75 cfs 0.137 af

Reach 3R: Entrance Culvert Reach to Avg. Flow Depth=0.43' Max Vel=0.66 fps Inflow=15.81 cfs 4.525 af n=0.070 L=147.0' S=0.0034 '/' Capacity=950.98 cfs Outflow=15.70 cfs 4.464 af

Pond 1P: Ext. 15" CMP Martin Road Peak Elev=50.41' Storage=652,236 cf Inflow=118.01 cfs 14.678 af 15.0" Round Culvert n=0.013 L=50.0' S=0.0100 '/' Outflow=4.14 cfs 2.476 af

Pond 3P: Ext. 36" RCP Route 236 Peak Elev=41.33' Storage=568,843 cf Inflow=103.57 cfs 19.848 af 36.0" Round Culvert n=0.013 L=100.0' S=0.0050 '/' Outflow=16.31 cfs 9.066 af

Pond 4P: 12" CMP Culvert to Det Pond Peak Elev=41.59' Storage=153 cf Inflow=1.12 cfs 0.074 af 12.0" Round Culvert n=0.013 L=50.0' S=0.0100 '/' Outflow=1.01 cfs 0.074 af

Pond 5P: Detention Pond #1 Peak Elev=41.39' Storage=1,548 cf Inflow=2.25 cfs 0.157 af Primary=1.39 cfs 0.142 af Secondary=0.00 cfs 0.000 af Outflow=1.39 cfs 0.142 af

Pond 6P: Prp. 36" CMP Entrance Crossing Peak Elev=42.17' Storage=24,440 cf Inflow=26.09 cfs 4.703 af 36.0" Round Culvert n=0.013 L=40.0' S=0.0125 '/' Outflow=15.81 cfs 4.525 af

Link AP1: AP1Inflow=16.31 cfs 9.066 af
Primary=16.31 cfs 9.066 af

Total Runoff Area = 95.156 ac Runoff Volume = 32.463 af Average Runoff Depth = 4.09" 86.70% Pervious = 82.502 ac 13.30% Impervious = 12.653 ac

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Summary for Subcatchment 1S: Subcat to Martin Road Crossing

Runoff = 118.01 cfs @ 12.46 hrs, Volume= 14.678 af, Depth> 4.03"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 YEAR STORM Rainfall=6.60"

A	rea (sf)	CN E	escription						
	13,186	98 F	98 Paved parking, HSG C						
	84,109			ing, HSG D					
	24,295	46 2	acre lots,	12% imp, H	HSG A				
	14,995		Voods, Fai	•					
	57,513			12% imp, F	HSG C				
	30,028		Voods, Fai						
	77,718		Brush, Fair,						
	62,281		Voods, Fai						
	40,897		acre lots,	12% imp, F	HSG D				
	05,022		Veighted A						
	61,002	_	_	vious Area					
2	44,020	1	2.81% lmp	pervious Ar	ea				
_									
Tc	Length	Slope	Velocity		Description				
(min)_	(feet)	(ft/ft)	(ft/sec)	(cfs)					
2.9	50	0.0750	0.29		Sheet Flow, SF 1				
					Range n= 0.130 P2= 3.33"				
18.4	875	0.0251	0.79		Shallow Concentrated Flow, SCF 1				
					Woodland Kv= 5.0 fps				
12.5	1,128	0.0022	1.50	1,953.86					
					Area= 1,300.0 sf Perim= 701.0' r= 1.85'				
					n= 0.070 Sluggish weedy reaches w/pools				
33.8	2,053	Total							

Summary for Subcatchment 2S: Subcat to Prp Driveway Crossing

Runoff = 89.68 cfs @ 12.81 hrs, Volume= 15.247 af, Depth> 4.10"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 YEAR STORM Rainfall=6.60"

Area (sf)	CN	Description			
76,151	46	2 acre lots, 12% imp, HSG A			
27,952	77	2 acre lots, 12% imp, HSG C			
10,053	98	Paved parking, HSG C			
34,997	77	Brush, Fair, HSG D			
73,197	98	Paved parking, HSG D			
13,850	80	>75% Grass cover, Good, HSG D			
354,389	79	Woods, Fair, HSG D			
1,355,579	82	2 acre lots, 12% imp, HSG D			
1,946,168	81	Weighted Average			
1,687,756		86.72% Pervious Area			
258,412		13.28% Impervious Area			

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	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	12.1	50	0.0200	0.07		Sheet Flow, SF 1
						Woods: Light underbrush n= 0.400 P2= 3.33"
	27.7	1,393	0.0281	0.84		Shallow Concentrated Flow, SCF 1
						Woodland Kv= 5.0 fps
	21.8	1,789	0.0053	1.37	683.53	Channel Flow, CF 1
						Area= 500.0 sf Perim= 601.0' r= 0.83'
						n= 0.070 Sluggish weedy reaches w/pools
_	61.6	3.232	Total	•	•	

Summary for Subcatchment 3S: Drainage to Prp Entrance Crossing

Runoff = 25.86 cfs @ 12.21 hrs, Volume= 2.381 af, Depth> 4.48"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 YEAR STORM Rainfall=6.60"

_	Α	rea (sf)	CN [Description						
		767	96 (Gravel surface, HSG D						
		39,205			ing, HSG D					
		3,521	80 >	75% Gras	s cover, Go	ood, HSG D				
		52,906		Voods, Fai	•					
_	1	81,696	82 V	Voods/gras	ss comb., F	air, HSG D				
		78,095		Veighted A						
		38,890	_		vious Area					
		39,205	1	4.10% lmp	pervious Ar	ea				
	_	1 41.	01	V/-124	0	December				
	Tc	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	7.8	50	0.0600	0.11		Sheet Flow, SF 1				
	4.0	254	0.0500	4.40		Woods: Light underbrush n= 0.400 P2= 3.33"				
	4.9	351	0.0569	1.19		Shallow Concentrated Flow, SCF 1				
	2.2	470	0.0004	2.40	005.40	Woodland Kv= 5.0 fps				
	3.2	479	0.0094	2.49	995.12	Channel Flow, CF 1 Area= 400.0 sf Perim= 301.0' r= 1.33'				
						n= 0.070 Sluggish weedy reaches w/pools				
_	45.0		-			11- 0.070 Sluggisti weedy reaches w/pools				
	15.9	880	Total							

Summary for Subcatchment 4S: North Development Subcat

[49] Hint: Tc<2dt may require smaller dt

Runoff = 1.12 cfs @ 12.05 hrs, Volume= 0.074 af, Depth> 5.34"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 YEAR STORM Rainfall=6.60"

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A	rea (sf)	CN I	Description			
	308	98 F	Roofs, HSC	D D		
	4,346	98 I	Paved park	ing, HSG D		
	2,596	80 >	>75% Gras	s cover, Go	ood, HSG D	
	7,250	92 \	Neighted A	verage		
	2,596	(35.81% Per	vious Area		
	4,654	(64.19% Impervious Area			
Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	,	(cfs)		
2.9	48	0.0950	0.28		Sheet Flow, SF 1	
					Grass: Short n= 0.150 P2= 3.33"	

Summary for Subcatchment 5S: South Development Subcat & Det Pond

[49] Hint: Tc<2dt may require smaller dt

Runoff = 1.27 cfs @ 12.04 hrs, Volume= 0.0

0.083 af, Depth> 5.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 YEAR STORM Rainfall=6.60"

_	<u> </u>	rea (sf)	CN	Description						
		210	98	Roofs, HSG D						
		4,685	98	Paved parking, HSG D						
_		3,556	80	>75% Gras	s cover, Go	ood, HSG D				
_		8,451 3,556 4,895	90	Weighted Average 42.08% Pervious Area 57.92% Impervious Area						
	Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	Description				
	2.6	44	0.1000	0.28		Sheet Flow, SF 1				
				O 450 DO 000						

Grass: Short n= 0.150 P2= 3.33"

Summary for Reach 1R: Martin Road Culvert Reach to Rte 236 Culvert

[79] Warning: Submerged Pond 1P Primary device # 1 OUTLET by 0.12'

Inflow Area = 43.733 ac, 12.81% Impervious, Inflow Depth > 0.68" for 25 YEAR STORM event

Inflow = 4.14 cfs @ 18.79 hrs, Volume= 2.476 af

Outflow = 4.14 cfs @ 19.16 hrs, Volume= 2.323 af, Atten= 0%, Lag= 21.9 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.65 fps, Min. Travel Time= 13.5 min Avg. Velocity = 0.50 fps, Avg. Travel Time= 17.7 min

Peak Storage= 3,347 cf @ 18.93 hrs Average Depth at Peak Storage= 0.12'

Bank-Full Depth= 4.00' Flow Area= 400.0 sf, Capacity= 2,066.93 cfs

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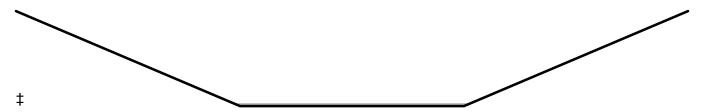
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50.00' x 4.00' deep channel, n= 0.070 Sluggish weedy reaches w/pools Side Slope Z-value= 12.5 '/' Top Width= 150.00' Length= 529.0' Slope= 0.0161 '/'

Inlet Invert= 48.50', Outlet Invert= 40.00'



Summary for Reach 2R: Det Pond Reach to Rte 236 Crossing

[79] Warning: Submerged Pond 5P Primary device # 1 OUTLET by 0.04'

Inflow Area = 0.360 ac, 60.82% Impervious, Inflow Depth > 4.71" for 25 YEAR STORM event

Inflow = 1.39 cfs @ 12.15 hrs, Volume= 0.142 af

Outflow = 0.75 cfs @ 13.23 hrs, Volume= 0.137 af, Atten= 46%, Lag= 64.2 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.09 fps, Min. Travel Time= 42.7 min Avg. Velocity = 0.08 fps, Avg. Travel Time= 45.7 min

Peak Storage= 1,938 cf @ 12.51 hrs Average Depth at Peak Storage= 0.04'

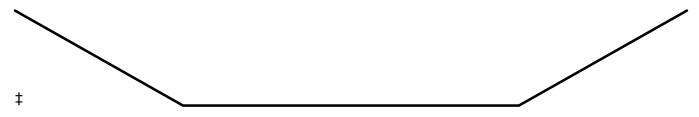
Bank-Full Depth= 4.00' Flow Area= 1,200.0 sf, Capacity= 1,761.91 cfs

200.00' x 4.00' deep channel, n= 0.070 Sluggish weedy reaches w/pools

Side Slope Z-value= 25.0 '/' Top Width= 400.00'

Length= 226.0' Slope= 0.0011 '/'

Inlet Invert= 39.75', Outlet Invert= 39.50'



Summary for Reach 3R: Entrance Culvert Reach to Rte 236 Crossing

[79] Warning: Submerged Pond 6P Primary device # 1 OUTLET by 0.43'

Inflow Area = 50.117 ac, 12.97% Impervious, Inflow Depth > 1.08" for 25 YEAR STORM event

Inflow = 15.81 cfs @ 12.45 hrs, Volume= 4.525 af

Outflow = 15.70 cfs @ 12.55 hrs, Volume= 4.464 af, Atten= 1%, Lag= 6.4 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.66 fps, Min. Travel Time= 3.7 min

Avg. Velocity = 0.34 fps, Avg. Travel Time= 7.2 min

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Peak Storage= 3,489 cf @ 12.49 hrs Average Depth at Peak Storage= 0.43' Bank-Full Depth= 4.00' Flow Area= 400.0 sf. Capacity= 950.98 cfs

50.00' x 4.00' deep channel, n= 0.070 Sluggish weedy reaches w/pools

Side Slope Z-value= 12.5 '/' Top Width= 150.00'

Length= 147.0' Slope= 0.0034 '/'

Inlet Invert= 40.00', Outlet Invert= 39.50'



Summary for Pond 1P: Ext. 15" CMP Martin Road Culvert

Inflow Area = 43.733 ac, 12.81% Impervious, Inflow Depth > 4.03" for 25 YEAR STORM event

Inflow = 118.01 cfs @ 12.46 hrs, Volume= 14.678 af

Outflow = 4.14 cfs @ 18.79 hrs, Volume= 2.476 af, Atten= 96%, Lag= 380.0 min

Primary = 4.14 cfs @ 18.79 hrs, Volume= 2.476 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Starting Elev= 49.00' Surf.Area= 231,086 sf Storage= 119,579 cf

Peak Elev= 50.41' @ 18.79 hrs Surf.Area= 469,037 sf Storage= 652,236 cf (532,657 cf above start)

Flood Elev= 52.50' Surf.Area= 556,200 sf Storage= 1,713.635 cf (1,594,056 cf above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= 179.8 min (979.5 - 799.6)

Volume	Inve	<u>ert Avail.Sto</u>	rage Storage	Description		
#1	48.0	00' 1,999,1	22 cf Custom	Stage Data (P	rismatic)Listed below (Recalc)	
Elevation (fee		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)		
48.0 50.0 52.0 53.0	00 00	8,072 454,100 526,650 585,750	0 462,172 980,750 556,200	462,172 1,442,922 1,999,122		
Device	Routing	Invert	Outlet Devices	S		
#1	Primary	49.00'	15.0" Round CMP_Round 15" L= 50.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 49.00' / 48.50' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf			

Primary OutFlow Max=4.14 cfs @ 18.79 hrs HW=50.41' (Free Discharge) 1=CMP_Round 15" (Inlet Controls 4.14 cfs @ 3.37 fps)

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Summary for Pond 3P: Ext. 36" RCP Route 236 Crossing

[63] Warning: Exceeded Reach 2R INLET depth by 1.57' @ 15.80 hrs [63] Warning: Exceeded Reach 3R INLET depth by 1.10' @ 15.95 hrs

Inflow Area = 95.156 ac, 13.30% Impervious, Inflow Depth > 2.50" for 25 YEAR STORM event

Inflow = 103.57 cfs @ 12.79 hrs, Volume= 19.848 af

Outflow = 16.31 cfs @ 15.74 hrs, Volume= 9.066 af, Atten= 84%, Lag= 177.0 min

Primary = 16.31 cfs @ 15.74 hrs, Volume= 9.066 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Starting Elev= 39.50' Surf.Area= 143,350 sf Storage= 37,675 cf

Peak Elev= 41.33' @ 15.74 hrs Surf.Area= 362,652 sf Storage= 568,843 cf (531,168 cf above start)

Flood Elev= 49.50' Surf.Area= 452,670 sf Storage= 1,685,450 cf (1,647,775 cf above start)

Plug-Flow detention time= 274.0 min calculated for 8.201 af (41% of inflow)

Center-of-Mass det. time= 144.4 min (983.2 - 838.8)

<u>Volume</u>	Inv	<u>ert Avail.Sto</u>	rage Stora	ge Description	
#1	39.0	00' 1,685,4	50 cf Cust	om Stage Data (Pr	ismatic)Listed below (Recalc)
Elevation (fee		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	• • • • • • • • • • • • • • • • • • • •	
39.0	00	7,350	0	0	
40.0	00	279,350	143,350	143,350	
42.0	00	405,040	684,390	827,740	
44.0	00	452,670	857,710	1,685,450	
Device	Routing	Invert	Outlet Dev	ices	
#1	Primary	39.50'	36.0" Rou	ind RCP_Round 3	6"
	-		L= 100.0'	RCP, sq.cut end pr	rojecting, Ke= 0.500
			Inlet / Outle	et Invert= 39.50' / 39	9.00' S= 0.0050 '/' Cc= 0.900

Primary OutFlow Max=16.31 cfs @ 15.74 hrs HW=41.33' (Free Discharge)
1=RCP_Round 36" (Barrel Controls 16.31 cfs @ 5.19 fps)

Summary for Pond 4P: 12" CMP Culvert to Det Pond

n= 0.013 Corrugated PE, smooth interior, Flow Area= 7.07 sf

[82] Warning: Early inflow requires earlier time span

Inflow Area = 0.166 ac, 64.19% Impervious, Inflow Depth > 5.34" for 25 YEAR STORM event Inflow = 0.074 af

Outflow = 1.01 cfs @ 12.08 hrs, Volume= 0.074 af, Atten= 10%, Lag= 1.8 min

Primary = 1.01 cfs @ 12.08 hrs, Volume= 0.074 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 41.59' @ 12.08 hrs Surf.Area= 339 sf Storage= 153 cf

Plug-Flow detention time= 6.1 min calculated for 0.074 af (99% of inflow) Center-of-Mass det. time= 4.4 min (750.8 - 746.4)

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Volume	Inv	vert Avail.Sto	orage Storag	e Description	
#1	41.	.00' 1,7	43 cf Custo	m Stage Data (P	rismatic)Listed below (Recalc)
Elevation (fee		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
41.0	00	175	0	0	
42.0	00	450	313	313	
44.0	00	980	1,430	1,743	
Device	Routing	lnvert	Outlet Device	es	
#1	Primary	41.00'	12.0" Roun	nd CMP_Round	12"
			L= 50.0' CI	MP, projecting, no	headwall, Ke= 0.900
			Inlet / Outlet	t Invert= 41.00' / 4	10.50' S= 0.0100 '/' Cc= 0.900
n= 0.0			n= 0.013 Co	orrugated PE, sm	ooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.98 cfs @ 12.08 hrs HW=41.58' (Free Discharge) 1=CMP_Round 12" (Inlet Controls 0.98 cfs @ 2.05 fps)

Summary for Pond 5P: Detention Pond #1

[82] Warning: Early inflow requires earlier time span

[92] Warning: Device #4 is above defined storage

[79] Warning: Submerged Pond 4P Primary device # 1 INLET by 0.39'

Inflow Area = 0.360 ac, 60.82% Impervious, Inflow Depth > 5.22" for 25 YEAR STORM event
Inflow = 2.25 cfs @ 12.05 hrs, Volume= 0.157 af
Outflow = 1.39 cfs @ 12.15 hrs, Volume= 0.142 af, Atten= 38%, Lag= 6.1 min
Primary = 1.39 cfs @ 12.15 hrs, Volume= 0.142 af
Secondary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 41.39' @ 12.15 hrs Surf.Area= 1,376 sf Storage= 1,548 cf

Plug-Flow detention time= 68.8 min calculated for 0.142 af (90% of inflow) Center-of-Mass det. time= 35.7 min (786.7 - 751.0)

Volume	Invert	Avail.Sto	rage Storag	e Description	
#1	40.00'	4,3	60 cf Custor	m Stage Data (P	rismatic)Listed below (Recalc)
Elevation (feet)	S	urf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
40.00		890	0	0	
41.00		1,200	1,045	1,045	
42.00		1,650	1,425	2,470	
43.00		2,130	1,890	4,360	
Device R	Routing	Invert	Outlet Devic	es	

#1 Primary 40.00' **12.0" Round CMP_Round 12"**

L= 40.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 40.00' / 39.75' S= 0.0063 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

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#2	Device 1	40.60'	6.0" Vert. Orifice/Grate X 2.00 C= 0.600
#3	Device 1	42.00'	2.0" x 2.0" Horiz. Orifice/Grate
			C= 0.600 in 24.0" Grate (1% open area)
			Limited to weir flow at low heads
#4	Secondary	43.00'	20.0' long x 4.0' breadth Broad-Crested Rectangular Weir
	•		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50 4.00 4.50 5.00 5.50
			Coef. (English) 2.38 2.54 2.69 2.68 2.67 2.67 2.65 2.66 2.66
			2.68 2.72 2.73 2.76 2.79 2.88 3.07 3.32

Primary OutFlow Max=1.39 cfs @ 12.15 hrs HW=41.39' (Free Discharge)

_1=CMP_Round 12" (Passes 1.39 cfs of 2.81 cfs potential flow)

-2=Orifice/Grate (Orifice Controls 1.39 cfs @ 3.53 fps)

-3=Orifice/Grate (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=40.00' (Free Discharge)
4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond 6P: Prp. 36" CMP Entrance Crossing

[62] Hint: Exceeded Reach 1R OUTLET depth by 2.13' @ 12.45 hrs

Inflow Area = 50.117 ac, 12.97% Impervious, Inflow Depth > 1.13" for 25 YEAR STORM event

Inflow = 26.09 cfs @ 12.21 hrs, Volume= 4.703 af

Outflow = 15.81 cfs @ 12.45 hrs, Volume= 4.525 af, Atten= 39%, Lag= 14.0 min

Primary = 15.81 cfs @ 12.45 hrs, Volume= 4.525 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Starting Elev= 40.50' Surf.Area= 5,263 sf Storage= 1,351 cf

Peak Elev= 42.17' @ 12.45 hrs Surf.Area= 22,976 sf Storage= 24,440 cf (23,090 cf above start)

Flood Elev= 46.00' Surf.Area= 48,500 sf Storage= 89,900 cf (88,549 cf above start)

Plug-Flow detention time= 31.9 min calculated for 4.494 af (96% of inflow)

Center-of-Mass det. time= 15.0 min (898.5 - 883.4)

Volume	In	vert A	ail.Storaر	ge Storaç	ge Description	
#1	40	.00'	89,900	cf Custo	om Stage Data (P	rismatic)Listed below (Recalc)
Elevatio (fee 40.0 42.0 44.0	et) 00 00	Surf.Area (sq-ft 14(20,63) 48,500) (c))	Inc.Store ubic-feet) 0 20,770 69,130	Cum.Store (cubic-feet) 0 20,770 89,900	
Device #1	Routing Primary	•	40.50' 3	= 40.0' C	nd CMP_Round 3 MP, square edge	36" headwall, Ke= 0.500 -0.00' S= 0.0125 '/' Cc= 0.900

n= 0.013 Corrugated PE, smooth interior, Flow Area= 7.07 sf

Primary OutFlow Max=15.80 cfs @ 12.45 hrs HW=42.17' (Free Discharge) 1=CMP Round 36" (Barrel Controls 15.80 cfs @ 5.66 fps)

Type III 24-hr 25 YEAR STORM Rainfall=6.60"

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Summary for Link AP1: AP1

Inflow Area = 95.156 ac, 13.30% Impervious, Inflow Depth > 1.14" for 25 YEAR STORM event

Inflow = 16.31 cfs @ 15.74 hrs, Volume= 9.066 af

Primary = 16.31 cfs @ 15.74 hrs, Volume= 9.066 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Well Field 44 Cannabis Dispensary - Existing Condition Peak Flows

TTOIL LOIG TT Gal	mable Bioponeal	y Exioting Cont	aitioir i oak i iomo
Analysis Point 2 Year Storn		10 Year Storm	25 Year Storm
	(cfs)	(cfs)	(cfs)
AP1	6.79	13.31	17.45

Rainfall Event Totals (in.)			
2-Year	3.33		
10-Year	5.34		
25-Year	6.60		

Well Field 44 Cannabis Dispensary - Developed Condition Peak Flows

		/	
Analysis Point	2 Year Storm	10 Year Storm	25 Year Storm
	(cfs)	(cfs)	(cfs)
AP1	4.03	11.10	16.31

Well Field 44 Cannabis Dispensary - Change in Peak Flows

		<i>j</i>	
Analysis Point	2 Year Storm	10 Year Storm	25 Year Storm
	(cfs)	(cfs)	(cfs)
AP1	-2.76	-2.21	-1.14

Headwater Elevations: 25-Year Rainfall Event

Analysis Point	Existing Elev.	Developed Elev.	Location in Analysis
	(ft)	(ft)	
1P	50.41	50.41	Martin Road Crossing (15" CMP)
2P	42.01	N/A	Ext. On-Site Driveway: 12" Culvert Removed
3P	41.34	40.34	Route 236 Crossing (36" RCP)