



September 16, 2020

Bart McDonough, Town Planner
Town of Kittery
200 Rogers Road
Kittery, Maine 03904

**RE: Town of Kittery, Planning Board Services
Site Plan Review #2
Rice Public Library (8 Wentworth Street) Tax Map 4, Lot 88
CMA #591.132**

Dear Bart:

CMA Engineers has received the following additional information for the site plan review for the proposed Rice Public Library at 8 Wentworth Street in Kittery, Tax Map 4, Lot 88 (our Assignment #132).

- 1) Site Plan Drawings for Rice Public Library, 8 Wentworth Street, Kittery, Maine 03904, prepared by Sebago Technics of South Portland, ME, revision dated September 10, 2020.
- 2) Revised Final Site Plan Review Application – Rice Public Library, 8 Wentworth Street, prepared by Sebago Technics of South Portland, ME (with Lassel Architects and Scott Simons Architects) dated September 10, 2020.
- 3) Revised Stormwater Management Report for Rice Public Library prepared by Sebago Technics of South Portland, ME dated September 2020 (original dated February 2020).

We had previously provided a review letter dated August 19, 2020 of the application materials dated July 2020 for conformance with the Kittery Land Use and Development Code (LUDC) and general engineering practices and offered comments.

The applicant addressed Town of Kittery staff comments, as well as ours and the Kittery TRC comments, in their revised Final Site Plan Review Application noted above.

The revised application materials satisfactorily address the issues raised in our August 19, 2020 review letter.

In addition to our general comments, we specifically note the following:

Article VIII: Surface Drainage

We noted that there had been several changes to July 2020 site plan since the February 2020 design. The February design was the basis of the stormwater management report dated February 2020. The changes in July were not anticipated to significantly alter the conclusions of the February report, but we suggested the stormwater management report be updated. The applicant prepared a revised stormwater management report that is based on the current final design, dated September 2020. That report demonstrates compliance with Kittery's stormwater performance standards.

Article IX. Parking, Loading and Traffic

The design included upper and lower parking lots but there were no specific minimum off-street parking requirements for library use. The design provided 29 parking spaces on-site. The applicant suggested applicable parking demand based on the size of the facility and equivalent full-time employees. We requested that the applicant provide calculations to support the number of parking spaces.


The September final application includes an analysis of typical parking provided at similar facilities and evaluated the proposed conditions. This analysis concludes that 27 parking spaces are appropriate based on the total proposed publicly accessible square footage based on a 1 parking space per 350 square foot public space criterion. (350 sf is a number at the low end of the reported range). Additional parking of 2 spaces for employees was based on a requirement of 1 pace per two full time equivalent employees. This yields 29 parking spaces. We believe this is a reasonable justification of total parking.

The applicant had not provided any handicap accessible spaces and we had suggested that the deign be modified to provide such spaces. The September 2020 design includes one accessible parking space eac in both the upper and lower parking lots. This is favorable.

Should you have any questions, please do not hesitate to call.

Very truly yours,

CMA ENGINEERS, INC.



William A. Straub, P.E.

Principal/Project Manager

cc: Stephen Doe, P.E., Sebago Technics



Final Site Plan Review Application

To:

Town of Kittery

For:

Rice Public Library Expansion

8 Wentworth Street

Presented by:

Lassel Architects

Scott Simons Architects

Prepared by:

Sebago Technics, Inc.

75 John Roberts Road, Suite 4A

South Portland, Maine 04106

September, 2020



September 10, 2020
18438

Mr. Bart McDonough, Town Planner
Town of Kittery
200 Rogers Rd.
Kittery, ME 03904

Final Site Plan Review Application
Rice Public Library, 8 Wentworth Street

Dear Bart:

On behalf of Rice Public Library (applicant) and in association with Lassel Architects and Scott Simons Architects, I am pleased to submit this Final Site Plan Application for the construction of an addition to the Rice Library and associated site improvements. The Preliminary Site Plan was approved by the Planning Board on August 27th, 2020. This Final Plan is similar to the approved plan with the following minor changes:

- The dumpster has been rotated slightly so it meets the side and rear setbacks.
- A small retaining wall that was previously deleted as part of the value engineering has been added back in to aid in building the stairs in the future.
- Note 15 has been added to the Site Plan which notes the approval for a waiver of Section 16.8.4.13.A Sidewalks along Traip Avenue. This was approved by the Planning Board at their August 27th Planning Board Meeting.

The following are responses to Staff comments from there Plan Review Notes dated August 27th 2020. Staff notes are in *italics* followed by our response in **Bold**.

A. *Site Plan Content*

1. *The Book and Page reference on Map 4 Lot 84 is incorrect. The correct reference should read as follows: 17564 / 144.*

The plans reflect the correct deed reference of 17992/618

2. *It is unclear how the tree and its root system located on the corner of Traip and Wentworth shall be preserved during the re-grading of the site. The Applicant should provide an updated site plan illustrating how this will be accomplished along with other trees slated to be preserved or transplanted.*

Site grading in the vicinity of the large tree at the corner of Wentworth and Traip has been limited so as to not disturb the root system. Stormdrains and site utilities have been placed well outside of the area to avoid disturbance to the roots. Grading is limited to minor fills with limited cuts to again minimize disturbances to the root system. A sizable

area around the tree will be loamed and seeded. In addition, the garden area on the north side of the library has received similar treatment in order to save trees. Utilities are routed around this area and site disturbances are minimized to peripheral areas to limit root damage. Construction fencing will be placed around significant tree saves to provide added protection to root systems.

3. *It is unclear if there will be enough snow storage capacity located along the eastern edge of the upper parking lot given the type of trees proposed (Thuja plicata—spring grove) and whether or not the Thuja plicata are salt tolerant. The Applicant should confirm the salt tolerance of the proposed vegetation and whether there will be adequate space for current and future snow storage. In addition, the Applicant should comment on the proposed spacing of the spring grove plants.*

We have moved some of the snow storage areas away from the arborvitae planting on the north side of the lot. Additional snow storage is shown to the south of the proposed parking area. Thuja plicata has moderate salt tolerance and should not be negatively impacted by the limited salt runoff if may receive. Surface run off is not directed towards the plantings so soils saturation with salts should also be limited. We also anticipate that the use of salt in the parking lot will be limited verses the level of salt used in public streets.

The proposed spacing of the plants is six (6) feet on center which will provide a very effective evergreen screen as the plants mature.

4. *The Applicant should also consider locations of future bicycle racks to accommodate multiple modes of transportation.*

The existing bike rack at the main entrance to the existing library will be maintained.

- B. *PLAN REVIEW NOTES August 27, 2020 8 Wentworth Street M4 L88 Page 3 Preliminary Plan Review ITEM 1*

5. *The Applicant should confirm whether or not the stormwater management plan will need to be revised that reflects the updated plans.*

A revised stormwater management plan has been provided.

6. *The proposed location of the dumpster on the northeast corner of the upper lot appears to encroach on the side yard setback. The Applicant needs to confirm if this is the case and, if so, propose an alternative location that satisfies the requirement, pursuant to §16.3.2..14.D Standards.*

The dumpster has been rotated slightly to meet all setbacks.

7. *As stated in the Applicant's narrative dated July, 23, 2020, a revised floor plan will be submitted to the Fire Chief to confirm the location of the mechanical room and its conformance with Fire Code standards. The Applicant should provide to the Board a timeline for that submission and subsequently submit a letter authored by the Fire Chief to the Board confirming the mechanical room's suitability*

Attached for Staff and Fire Chief review are floor plans and building elevations.

8. *The "Surveyor's Statement" on Sheet 1 of 1 needs endorsement.*

This endorsement is included in the attached plan set.

9. *While an erosion control plan is provided by the Applicant there appears a construction sequencing plan has not been furnished. The Applicant should provide a construction sequencing plan to the Board for review and approval.*

A project schedule is provided which shows project sequencing.

10. *The Applicant appears not to have provided information on construction staging, which should be furnish for review and approval.*

A Construction Logistics/Staging plan is included.

- C. *Peer Review and TRC Comments*

1. *All proposed granite curbing shall be sloped in order to accommodate the fire trucks.*

The plan set includes details for sloped cast-in-place concrete curbing for on-site curbing where a sidewalk does not abut the curbing. Additional details for sloped and vertical granite curbing are provided for curbing within the public Right-of-Way should this be required. This project does not propose curbing within the public ROW with the exception of curb cuts which we propose as sloped cast in place.

2. *The 6" water lines coming off of Traip Avenue must be verified to be adequate to support the proposed sprinkler system. Applicant should contact the Fire Department to confirm adequacy.*

It is our understanding that Staff has received confirmation from the Fire Chief of the adequacy of the line.

3. *Maintenance on the basement floor of the proposed expansion shall be in a location that permits immediate access for fire/rescue staff.*

Building floorplans are provided. It is our understanding that the Technical Review team will do another review of the plans when submitted for final plan review.

4. *Determination letter from Maine Historical should be provided prior to commencement of work.*

Scott Simons Architects and the Town Manager has been in communication with Maine Historical throughout the design process and has received a verbal sign off on the project. A request for a determination letter has been requested and will be forwarded upon receipt.

5. *Revised elevations need to be provided prior to final approval*

Attached are revised building elevations.

6. *Prior to construction, the applicant needs to contact the Department of Public Works to review the construction sequencing plans so as to ensure the proposed utilities that intersect the corner of Traip Avenue and Wentworth Street are compatible.*

We request that this condition be made a condition of Final Plan Approval

In the Site Plan Review letter from CMA Engineers dated August 19, 2020 there was a request for the applicant to provide calculations as it relates to Article IX. Parking, Loading and Traffic with specific focus on the 29 proposed parking spaces. While we believe we have provided information on the proposed parking plan in previous submissions we offer the following information in support of the 29 spaces. Calculations are also attached:

The town of Kittery has no parking requirement for libraries. In the absence of specific requirements the design team has researched recent library projects and available guidelines. The team has applied a standard of 1 space for every 350 sf of occupied public space and 1 spaces for every 2 full time equivalence employee. While the individual rate of parking space allocation varies from location to location, this is a common approach to establishing library parking requirements. The design teams noted a range of 300 to 1000 public sf per parking space with suburban or rural facilities tending toward the lower numbers, and in-town libraries tending towards the higher end of this spectrum.

Submission Items

In accordance with 16.10.7.2 of the Site Plan Review Application requirements of the Land-Use Code we offer the following plans and supportive documents for your review and consideration of this application.

1. Application
2. Final Plan set at a scale of 1" = 20' showing location map, zoning information, proposed name of project, name and address of record owner/applicant, assessor's information, standard boundary survey, right of way lines, metes and bounds of lot lines, parcel summary, net development calculations, proposed improvements including: grading, utilities, exterior mechanical units paved areas, building setbacks, plan dimensions, abutting lot owners, vegetation and landscaping, erosion and sedimentation control, and lighting locations with photometrics.
3. Architectural floor plans and elevations
4. A revised stormwater management plan prepared by a registered professional engineer.
5. A Logistics plan showing construction staging area and a project schedule
6. Parking Calculations

Mr. Bart McDonough
18438

5

September 10, 2020

I trust this submission addresses staff comments and that the final review of this project can continue on the next available Planning Board agenda. If you or Town staff require additional information, please contact me.

Sincerely,

SEBAGO TECHNICS, INC.

A handwritten signature in black ink, appearing to read "Stephen G. Doe". The signature is fluid and cursive, with a long horizontal stroke extending to the right.

Stephen G. Doe, RLA, LEED-AP
Landscape Architect

SGD: llg
Enc.

cc: Kendra Amaral, Town Manager
Ryan Kanteres, Scott Simons Architects

Table of Contents

Final Site Plan Review Application and Cover Letter

EXHIBIT 1 Application Form

EXHIBIT 2 Final Plan Set

EXHIBIT 3 Architectural Floor Plans and Elevations

EXHIBIT 4 Stormwater Management Plan

EXHIBIT 5 Logistics Plan

EXHIBIT 6 Parking Calculations

Exhibit 1

Application Form



TOWN OF KITTERY, MAINE

TOWN PLANNING AND DEVELOPMENT DEPARTMENT

200 Rogers Road, Kittery, Maine 03904
 PHONE: (207) 475-1323 - FAX: (207) 439-6806
www.kittery.org

APPLICATION: SITE PLAN REVIEW

FEE FOR SITE PLAN REVIEW:	<input checked="" type="checkbox"/> \$300.00 PLUS THE GREATER OF:	<input type="checkbox"/> \$50/USE OF UNIT; OR	<input checked="" type="checkbox"/> \$5.00/100 SQ FT OF GROSS FLOOR AREA	Application Fee Paid: \$ _____ Date: _____ ASA Fee Paid: (TITLE 3.3 TOWN CODE) \$ _____ Date: _____		
		<input type="checkbox"/> \$0.50/LINEAR FOOT OF DOCK, SLIP & FLOAT; OR	<input type="checkbox"/> \$20.00/ UNIT INTENDED TO PROVIDE OVERNIGHT SLEEPING ACCOMODATIONS			
PROPERTY DESCRIPTION	Parcel ID	Map	Lot	Zone:	Total Land Area (Square Feet)	
	Physical Address			Base:		34,947
		4	88	Overlay:		
				MS4:	YES NO	
		8 WENTWORTH STREET				
PROPERTY OWNER'S INFORMATION	Name	TOWN OF KITTERY		200 ROGERS ROAD KITTERY, ME 03904		
	Phone					
	Fax					
	Email	KAMARAL@KITTERY.ME.ORG				
APPLICANT'S AGENT INFORMATION <i>* ALSO SEE Below</i>	Name	RYAN KANTERES		Scott Simons Architects 75 YORK ST PORTLAND, ME 04101		
	Phone	772-4645				
	Fax					
	Email	RYAN@SIMONSAARCHITECTS.COM				
PROJECT DESCRIPTION	Existing Use:		Public Library		ALT AGENT FOR SITE PLAN:	
					% STEVE DOE	
					SEBAGO TECHNICS INC	
					SPOE@SEBAGO TECHNICS.COM	
					207-200-2056	
	Project Name:		RICE PUBLIC LIBRARY EXPANSION			
	Proposed Use:		PUBLIC LIBRARY			

WAIVER REQUEST

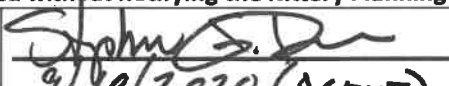
	Ordinance Section	Describe why this request is being made.
DESCRIPTION	***EXAMPLE*** 16.32.560 (B)- OFFSTREET PARKING.	***EXAMPLE*** Requesting a waiver of this ordinance since the proposed professional offices have a written agreement with the abutting Church owned property to share parking.
	N/A	N/A

Related Kittery Land Use Code concerning waivers and modifications:

16.10.8.2.5 Conditions or Waivers.

Conditions required by the Planning Board at the final plan review phase must have been met before the final plan may be given final approval unless so specified in the condition or specifically waived, upon written request by the applicant, by formal Planning Board action wherein the character and extent of such waivers which may have been requested are such that they may be waived without jeopardy to the public health, safety and general welfare.

16.7.4.1 Objectives Met. In granting modifications or waivers, the Planning Board must require such conditions as will, in its judgment, substantially meet the objectives of the requirements so waived or modified.

I certify that, to the best of my knowledge, the information provided in this application is true and correct and will not deviate from the plans submitted without notifying the Kittery Planning Department of any changes.			
Applicant's Signature:		Owner's Signature:	_____
Date:	9/10/2020 (AGENT)	Date:	_____

COMPLETED BY OFFICE STAFF

ASA CHARGE	AMOUNT	ASA CHARGE	AMOUNT
REVIEW		SERVICES	
LEGAL FEES (TBD)		RECORDER	\$35
ENGINEERS REVIEW (TBD)		FACT FINDING (TBD)	
ABUTTER NOTICES		3 RD PARTY INSPECTIONS (TBD)	
POSTAGE	\$20	OTHER PROFESSIONAL SERVICES	\$50
LEGAL NOTICES		PERSONNEL	
ADVERTISING	\$300	SALARY CHARGES IN EXCESS OF 20 HOURS	
SUPPLIES			
OFFICE	\$5		
SUB TOTAL		SUB TOTAL	
		TOTAL ASA REVIEW FEES	

Exhibit 2

Final Plan Set

Exhibit 3

Architectural Floor Plans and Elevations



- 1 - Morin - Academy Smooth Red Range Brick
- 2 - Oko Skin - Fiber Reinforced Concrete Panel - GR04 Silvergrey Textured Finish
For infill panels beside windows
- 3 - Englert - Slate Gray Metal
For Roof, Windows and Metal Siding



25 York Street
 Suite 1000
 Boston, MA 02116
 617.552.8000



370 Main Street
 South Norwalk, CT 06858
 203.784.3000

PROJECT NAME:
RICE PUBLIC LIBRARY

ADDRESS:
 SEAL:

NOT FOR CONSTRUCTION

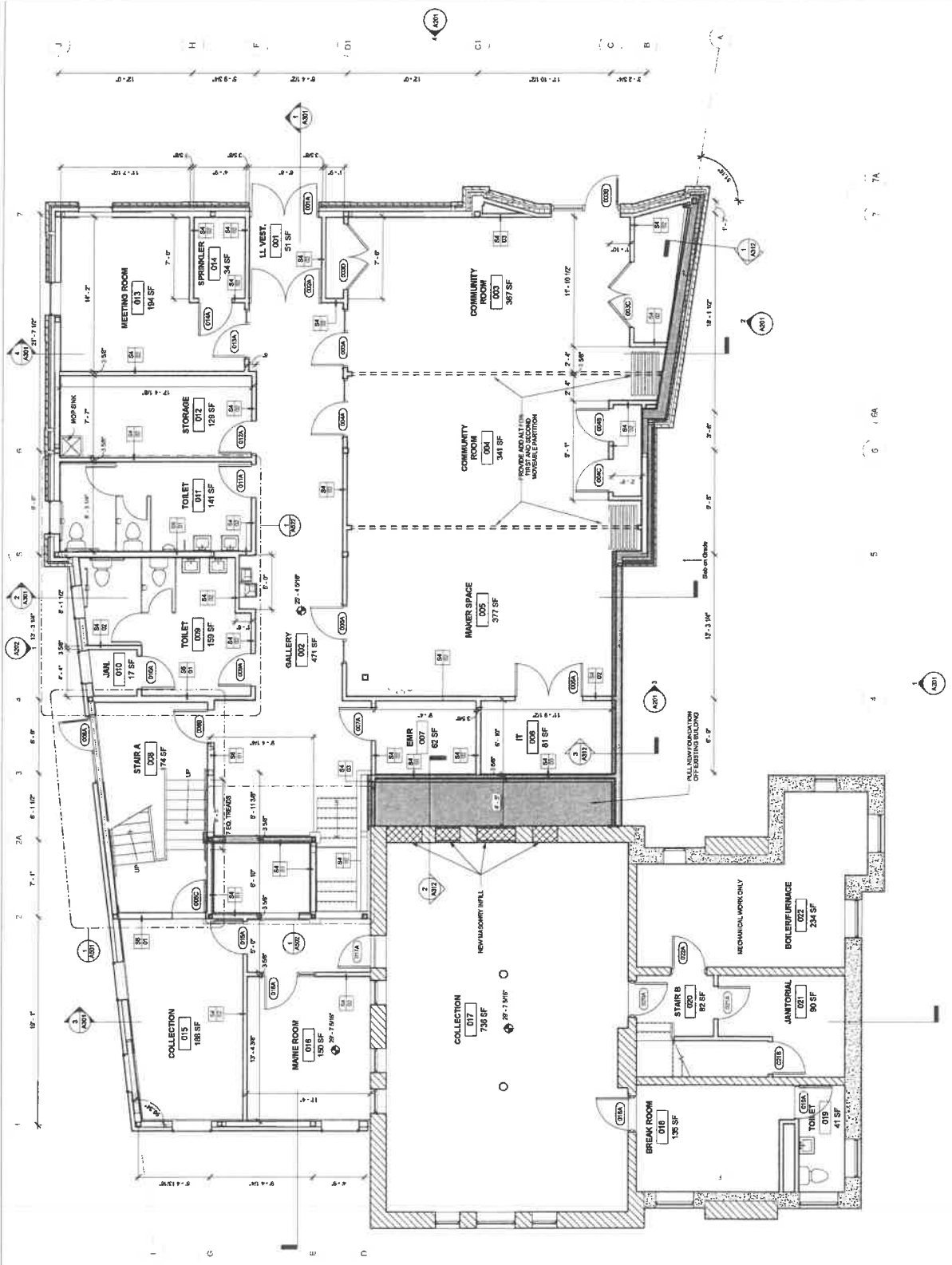
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1	DATE
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3	DATE
4	DATE
5	DATE
6	DATE

DATE OF ISSUE: 2020.08.14
 PROJECT NUMBER: 2018.0000
 STATUS: DESIGN DEVELOPMENT
 PROPOSED SET

LOWER LEVEL PLAN

A101



1 LOWER LEVEL
 1/4" = 1'-0"



7200 West
 10th Street, Suite 100
 Denver, CO 80202
 303.733.0000



370 Main Street
 South Norwalk, CT 06858
 203.381.3000

PROJECT NAME:
RICE PUBLIC LIBRARY

ARCHITECT:
 SEAL

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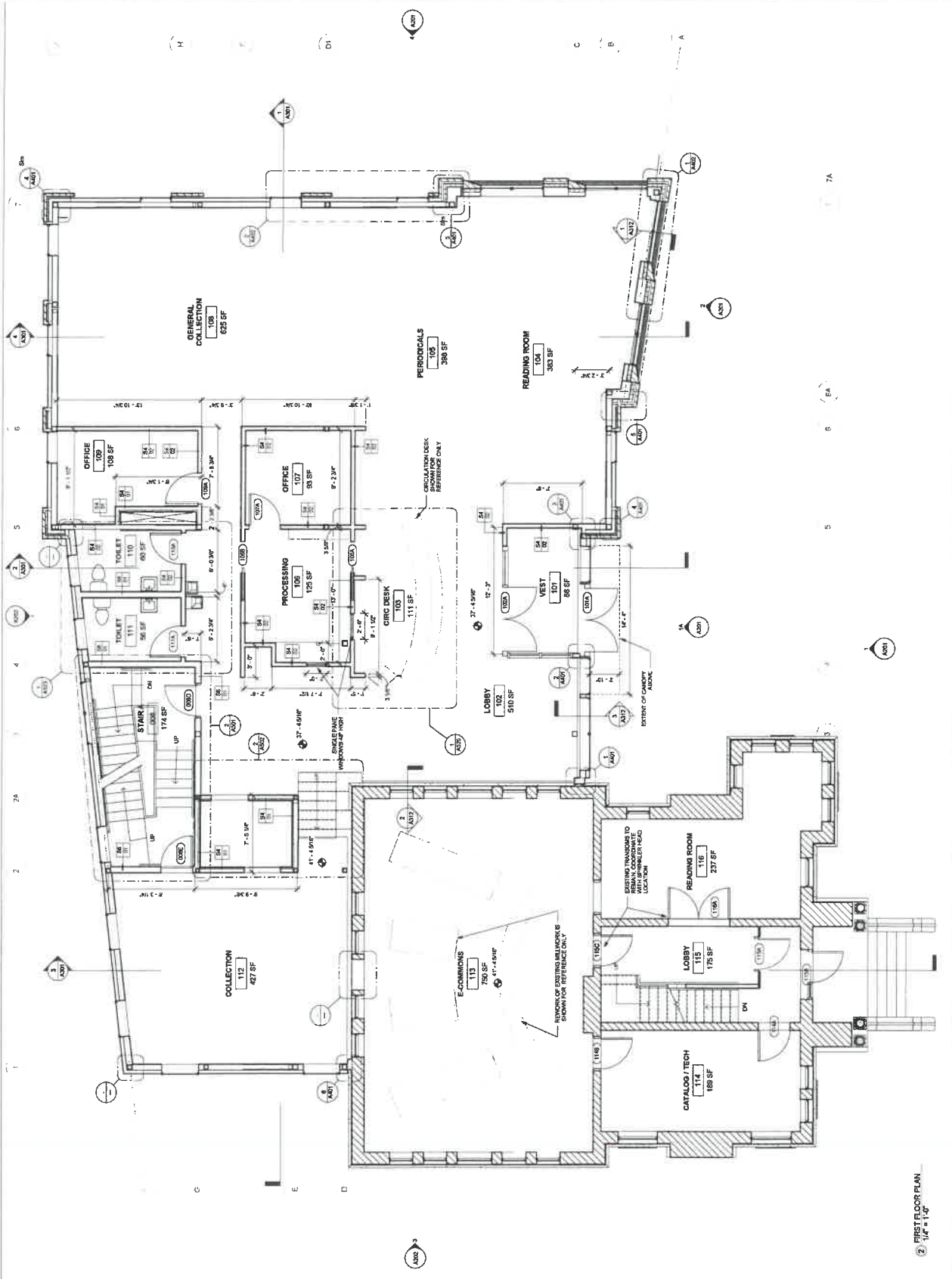
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4	DATE
5	DATE
6	DATE

DATE OF ISSUE: 2020.05.14
 PROJECT NUMBER: 2018-0200
 STATUS: DESIGN DEVELOPMENT
 PROGRESS SET

FIRST FLOOR PLAN

A102



2 FIRST FLOOR PLAN
 1/4" = 1'-0"



75 West Street, Suite 401
 Boston, MA 02108
 scottsimmons.com
 617.552.1000



373 Main Street
 South Berwick, ME 03906
 lasssel.com
 207.368.3589

PROJECT NAME:
RICE PUBLIC LIBRARY

ADDRESS:
 50AL

NOT FOR CONSTRUCTION

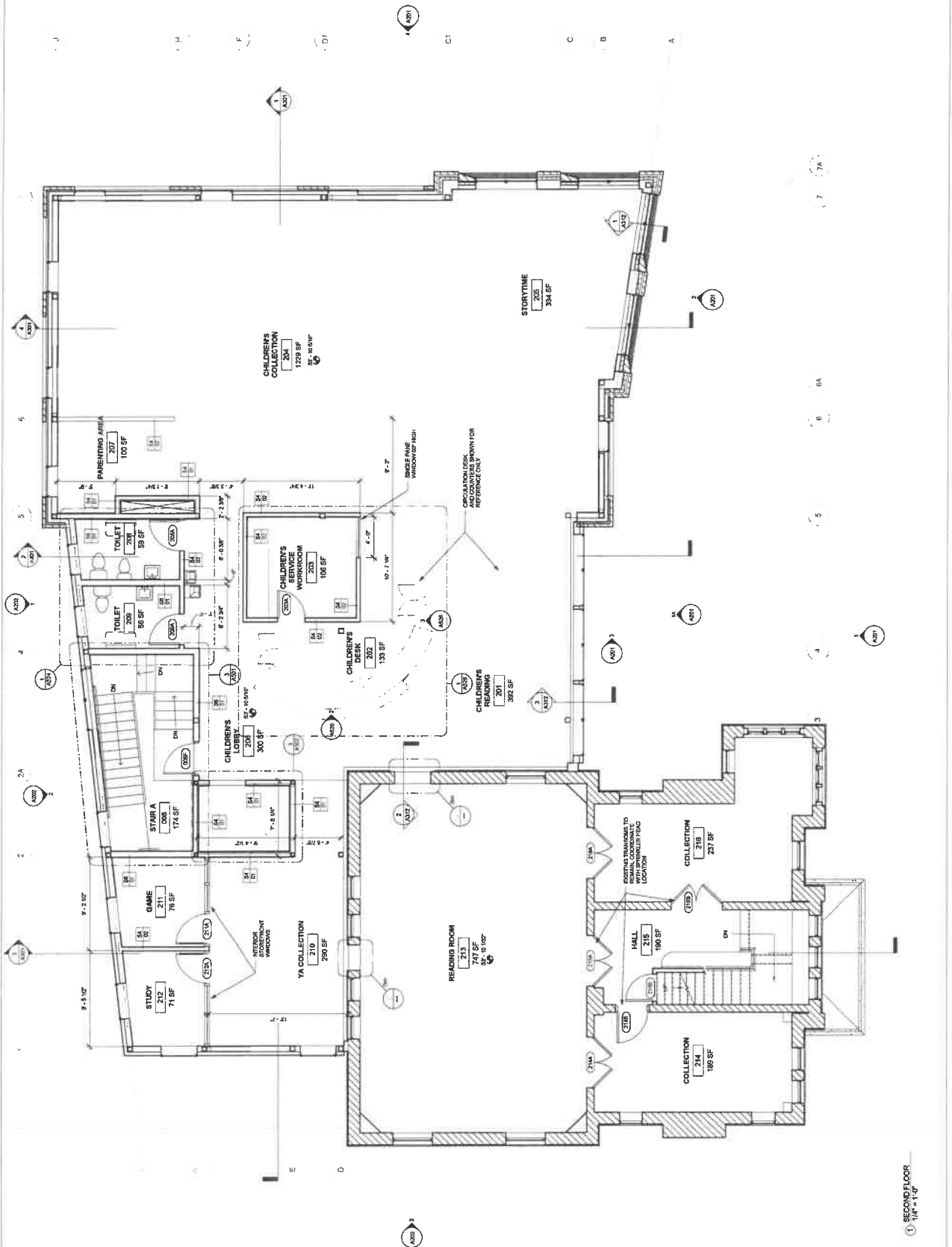
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 WITHOUT THE WRITTEN PERMISSION OF
 SCOTT SIMMONS ARCHITECTS, P.C.
 REVISION:

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8	DATE

DATE OF ISSUE: 2020.08.14
 PROJECT NUMBER: 20184.000
 STATUS: DESIGN DEVELOPMENT
 PROGRESS SET

SECOND FLOOR PLAN

A103



① SECOND FLOOR
 1/8" = 1'-0"



75 York Street
 Suite 2000
 Cambridge, MA 02148
 617.452.3100
 www.scottsimons.com



370 Main Street
 South Norwalk, CT 06858
 203.772.4688

PROJECT NAME:
RICE PUBLIC LIBRARY

ADDRESS:
 507 CENTRAL AVENUE
 DORSET, CT

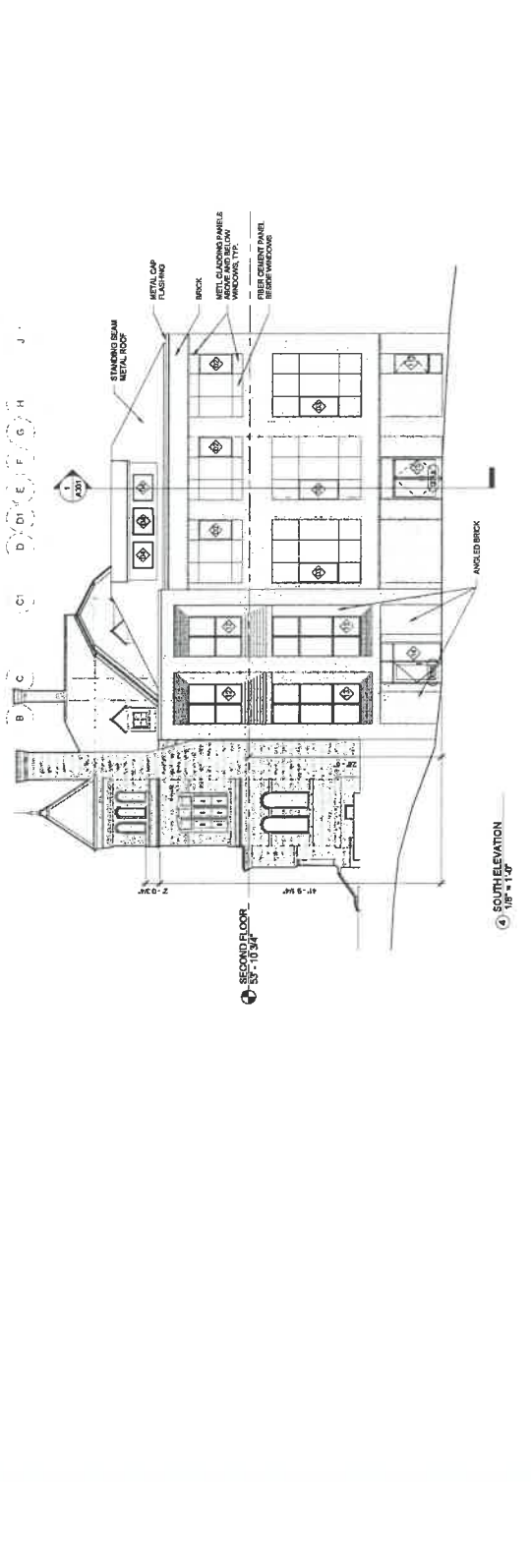
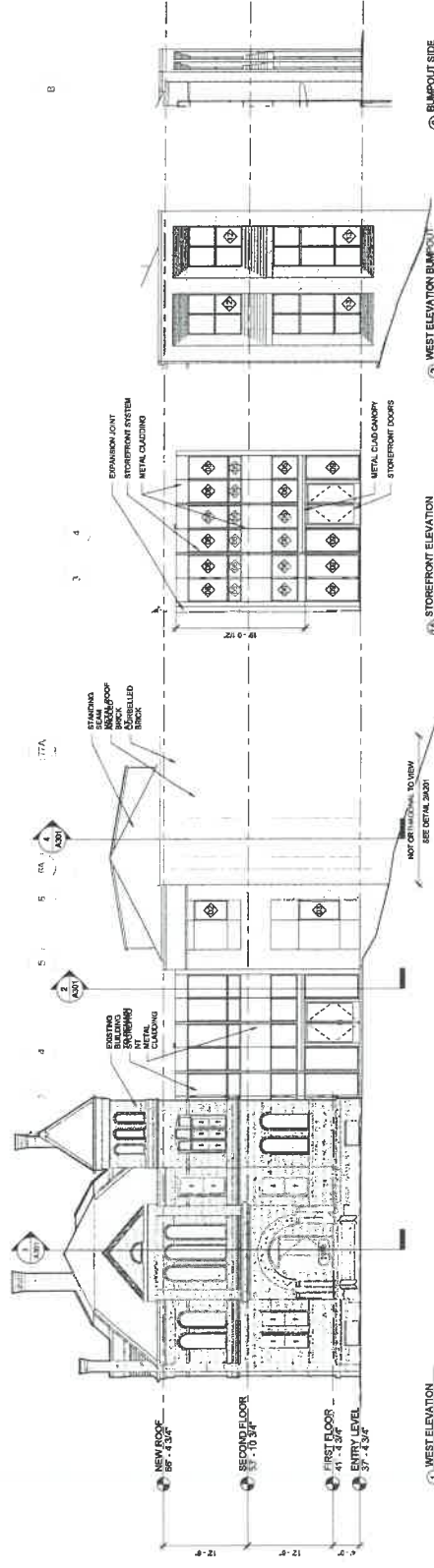
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 MEMPHIS, TN

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6	DATE
7	DATE

DATE OF ISSUE: 2021.08.14
 PROJECT NUMBER: 2016-0200
 STATUS: DESIGN DEVELOPMENT
 PROGRAM: RFP

BUILDING ELEVATIONS

A201



ELEVATION VIEW KEY - 1" = 30'



PROJECT NAME
RICE PUBLIC LIBRARY

ADDRESS
504

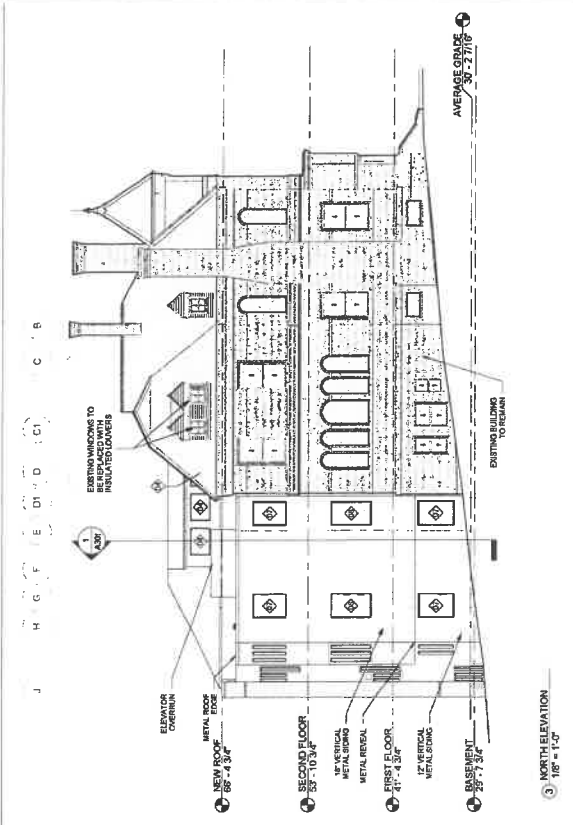
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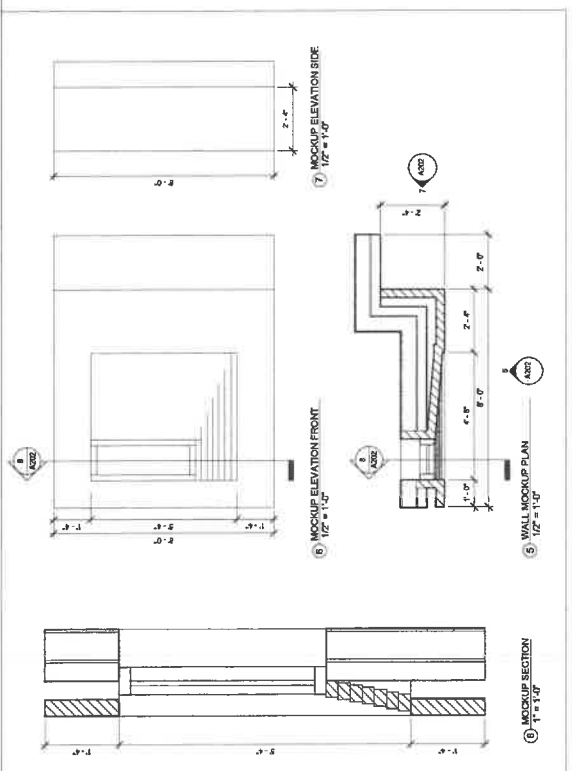
DATE OF ISSUE: 2018.06.14
PROJECT NUMBER: 2018-0000
STATUS: DESIGN DEVELOPMENT
DESIGNER: SCOTT SIMONS ARCHITECTS, P.C.
DESIGNER BY: SCOTT SIMONS
DRAWN BY: JEFFREY BROWN

BUILDING ELEVATIONS

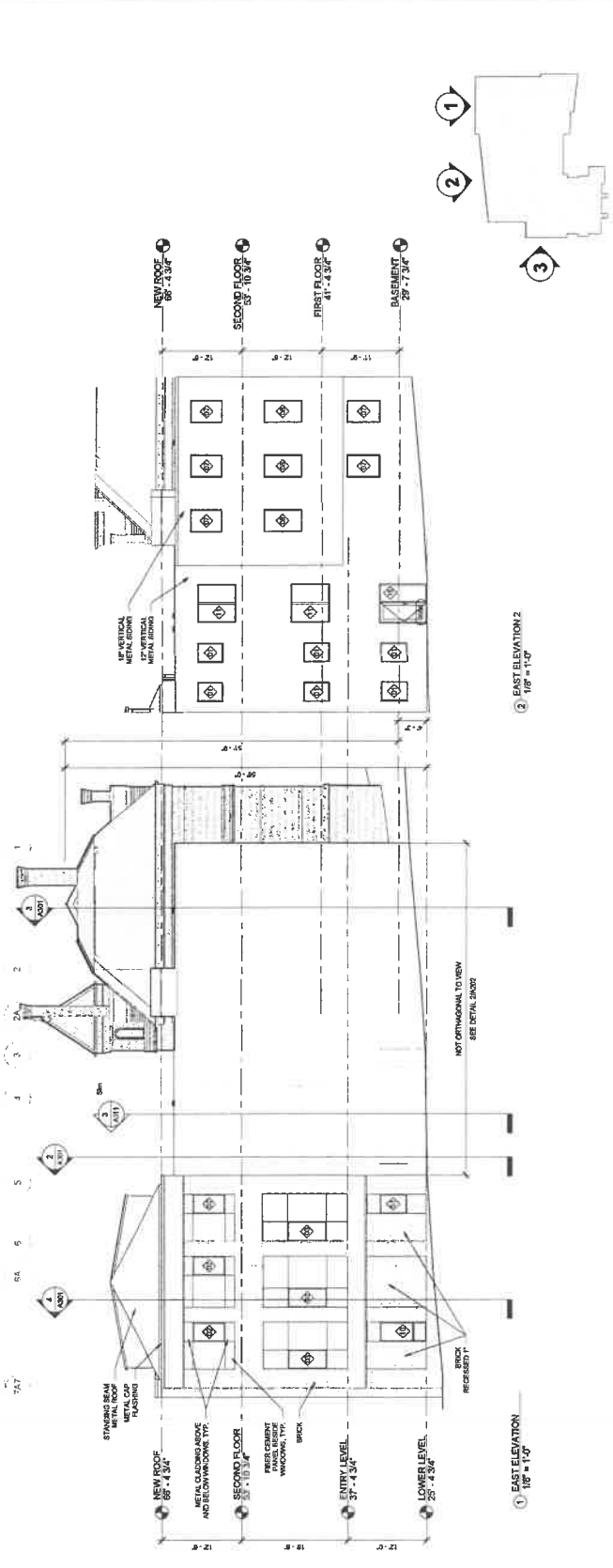
A202



3 NORTH ELEVATION
1/8" = 1'-0"



5 WALL MOCKUP PLAN
1/2" = 1'-0"



1 EAST ELEVATION
1/8" = 1'-0"

2 EAST ELEVATION 2
1/8" = 1'-0"

ELEVATION VIEW KEY - 1" = 30'

Exhibit 4

Stormwater Management Plan



CIVIL ENGINEERING • SURVEYING • LANDSCAPE ARCHITECTURE

STORMWATER MANAGEMENT REPORT

For

**RICE PUBLIC LIBRARY
KITTERY, MAINE**

Prepared for

Rice Public Library
8 Wentworth Street
Kittery, Maine 03904

February, 2020

Revised: September, 2020

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- Appendix 1B: Hydrologic Modeling – Proposed Conditions (HydroCAD) Summary
- Appendix 2: Inspection, Maintenance and Housekeeping Plan
- Appendix 3: Subsurface Investigations
- Appendix 4: Stormwater Management Plans

**STORMWATER MANAGEMENT REPORT
RICE PUBLIC LIBRARY
KITTERY, MAINE**

I. Introduction

This Stormwater Management Plan Report has been prepared to present analyses performed to address the potential impacts associated with the project due to proposed modification in stormwater runoff characteristics and land cover changes. The stormwater management controls that are outlined in this report have been designed to suit the proposed development and to comply with applicable regulatory requirements.

II. Existing Conditions

The project site consists of developed land located at 8 Wentworth Street in downtown Kittery. The site is approximately 0.8 acres in total consisting of the existing Rice Public Library building, gravel parking on the south end of the lot and paved parking on the north end. The site is bounded by Wentworth Street to the west and Traip Avenue on the southern and eastern sides of the site. The surrounding properties located on Traip Avenue consist of residential units while the buildings located on Wentworth Street are primarily for commercial use.

Slopes on the site range from approximately 3% across the paved parking lot to 45% in the landscape area between the southwestern corner of the library and the gravel lot. The majority of the site consists of slopes of 9-10% across the gravel parking lot south of the library and the surrounding landscaped areas.

The site is tributary to a system of storm sewers which outlet into the Piscataqua River. The river is not listed in Chapter 502 of the Maine Department of Environmental Protection (MDEP) regulations on *Direct Watersheds of Lakes Most at Risk From New Development, and Urban Impaired Streams*.

The proposed development area of the site is not located in an identified flood zone per the FEMA Flood Insurance Rate Map for the Kittery Community Panel 728, revised April 14, 2017.

III. Soils

Soil characteristics were obtained from the Class D: Medium Intensity Soil Survey completed by the United States Department of Agriculture Natural Resources Conservation Service Custom Web Soil Survey. The Hydrologic Groups (HSG) of the soils are classified by Technical Release TR-55 of the Soil Conservation Service as follows:

Soil Map Symbol	Soil Name	Slope (%)	HSG
Ur	Urban Land	0-8	D

Hydrologic Soil Group boundaries are delineated on the Watershed Map. A copy of the Class D Medium Intensity Soil Survey is included as Appendix 3.

IV. Proposed Site Improvements

The proposed development will consist of an approximately 3,660 square-foot addition to the existing Rice Public Library. The existing gravel parking lot on site will be transformed into a paved parking area with a new driveway entrance. The existing paved parking lot in the northern most area of the site will be redesigned as part of the project. Other site improvements include the redesign of pedestrian walkways on site as well as various landscape features.

V. Existing Conditions Model

The existing conditions watershed plan consists of one subcatchment labeled 1.0S in the HydroCAD model. One location was identified as the Study Point (SP) for comparing peak runoff rates for the site.

SP1 represents the drainage structure located south of the southeasterly corner of the site within Traip Avenue. Subcatchment 1.0S contributes runoff to this study point with an overall runoff area of approximately 0.84 acres. Runoff from the site enters a catch basin (1.0P) at the southeast corner of the site and then flows to SP1 via a corrugated metal pipe. SP1 and the associated drainage area are tributary to the Piscataqua River.

VI. Proposed Conditions Model

The proposed conditions watershed area consists of the same overall area as the existing conditions plan, however, the existing conditions subcatchment has been broken into smaller subcatchments as a result of the proposed development.

Post-development subcatchment 1.1S represents the southern half of the analyzed watershed. Runoff from this subcatchment area primarily consists of roof collection and paved parking, along with landscaped area. Subcatchment 1.2S represents the northern half of the watershed which is a majority landscaped area and more paved parking. All drainage in the proposed conditions flow to the existing municipal storm sewers located within Traip Avenue, represented by SP1, via a proposed catch basin and pipe network.

VII. Stormwater Management

Basic Standard - Chapter 500, Section 4(B)

The project will not disturb more than one (1) acre of land area, where MDEP Basic Standards apply, requiring that grading or other construction activities on the site do not impede or otherwise alter drainage ways to have an unreasonable adverse impact. However, we have avoided adverse impacts by providing an Erosion & Sedimentation Control Plan, and an Inspection, Maintenance and Housekeeping Plan (Appendix 2) to be implemented during construction and post-construction stabilization of the site. These construction requirements have been developed following Best Management Practice guidelines, latest edition.

Flooding Standard - Chapter 500, Section 4(F)

The planned project will not create more than three (3) acres of impervious surface, where MDEP Flooding Standards must be met. However, the Town of Kittery code for stormwater management requires a project’s stormwater management system detain, retain, or result in the infiltration of stormwater from 24-hour storms of the 2 and 25-year frequencies such that the peak flows of stormwater from the project site do not exceed the peak flows of stormwater prior to undertaking the project. As such, a runoff evaluation was performed using the methodology outlined in the USDA Soil Conservation Service’s “Urban Hydrology for Small Watersheds - Technical Release #55 (TR-55)”. HydroCAD computer software was utilized to perform the calculations using Portsmouth, NH rainfall data, as required by Town of Kittery codes.

Runoff curve numbers were determined for each of the watersheds by measuring the area of each hydrologic soil group within each type of land cover. The type of land cover was determined based on survey data, field reconnaissance and aerial photography. Times of concentration were determined from site topographic maps in accordance with SCS procedures.

The 24-hour rainfall values utilized in the hydrologic model were obtained from Appendix A of the *New Hampshire Stormwater Manual, Volume 2: Post-Construction Best Management Practices Selection & Design* (latest edition). Rainfall values for Portsmouth, NH are listed in the table below.

Storm Frequency Precipitation (in./24 hr)	
Portsmouth, NH	
2-year	3.1
25-year	5.2

The following table presents the results of the peak runoff calculations at the study point for the existing and proposed conditions.

Peak Runoff Rate Summary Table			
Analysis Point	Storm Event	Existing Conditions (cfs)	Proposed Conditions (cfs)
SP-1	2-year	2.0	2.0
	25-year	3.8	3.8

The HydroCAD Data output sheets from this analysis are appended to this report (Appendix 1) along with the Stormwater Management Plans (Appendix 4). The model predicts that the peak runoff rates in the proposed condition at Study Point 1 are equal to existing condition runoff rates for the 2 and 25-year storm events with implementation of the proposed stormwater management practices.

VIII. Summary

The proposed development has been designed to manage stormwater runoff through Best Management Practices approved by MDEP. Runoff discharging from the site will be the same as existing conditions for the 2 and 25-year storm events at the chosen study point. Additionally, erosion and sedimentation controls along with associated maintenance and housekeeping procedures have been outlined to prevent unreasonable impacts on the site and to the surrounding environment.

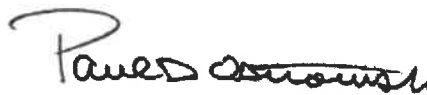
Prepared by:

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Mathew K. Orr, EI
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MKO



Paul D. Ostrowski, P.E.
Senior Project Engineer

PDO

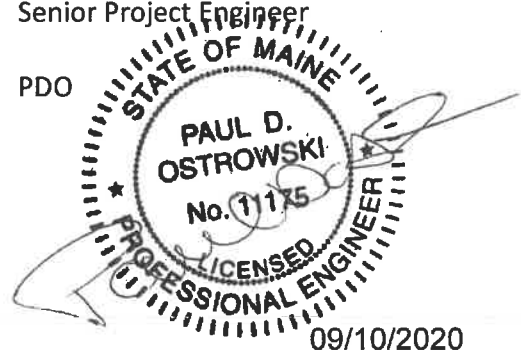


Exhibit 5

Logistics Plan

RICE LIBRARY LOGISTICS PLAN 9/11/2020

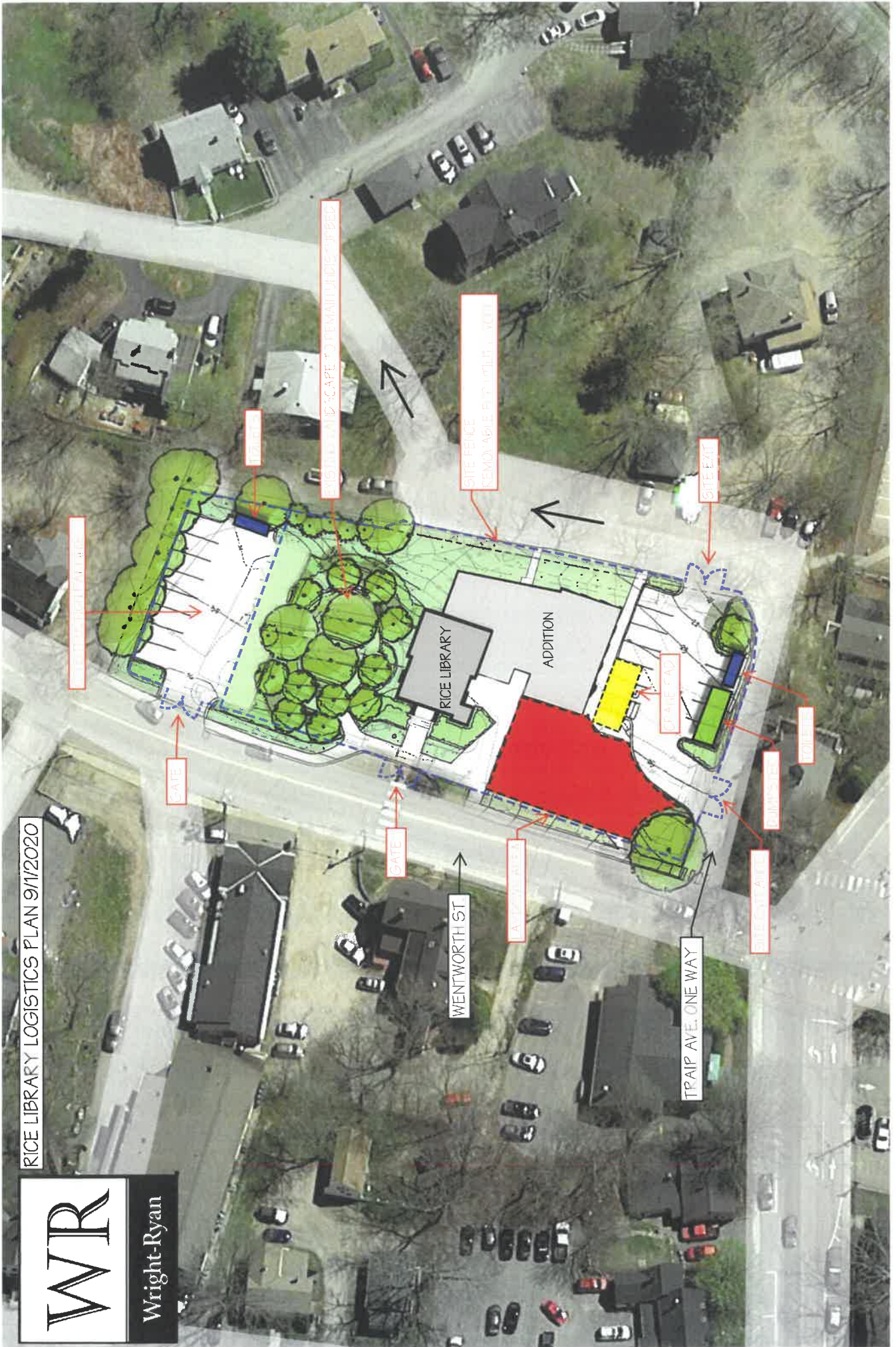


Exhibit 6

Parking Calculations

Room Areas by Floor			
	1st	2nd	3rd
	736	237	189
	145	740	740
	135	377	237
	281	575	200
Support Spaces in Yellow	489	325	71
1692.0	360	93	75
	344	132	500
	360	109	208
	84	624	154
	84	407	423
		189	1040
		150	147
Spaces			
	15500.0		
26	9268.0	2361	3622
	0.4		
	350.0	Sqft per Space *	
3	6		
	2.0	FEE Per Space	
29			

29

Total spaces provided

* Range based on recent project experience, ASCE guidelines, and survey of similar projects a variation of 1 space per 300 - 1000 spft was found for intown library projects



CIVIL ENGINEERING • SURVEYING • LANDSCAPE ARCHITECTURE

STORMWATER MANAGEMENT REPORT

For

**RICE PUBLIC LIBRARY
KITTERY, MAINE**

Prepared for

Rice Public Library
8 Wentworth Street
Kittery, Maine 03904

February, 2020

Revised: September, 2020

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Flooding Standard - Chapter 500, Section 4(F).....	3
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Appendices

- Appendix 1A: Hydrologic Modeling– Existing Conditions (HydroCAD)Summary
- Appendix 1B: Hydrologic Modeling – Proposed Conditions (HydroCAD) Summary
- Appendix 2: Inspection, Maintenance and Housekeeping Plan
- Appendix 3: Subsurface Investigations
- Appendix 4: Stormwater Management Plans

**STORMWATER MANAGEMENT REPORT
RICE PUBLIC LIBRARY
KITTERY, MAINE**

I. Introduction

This Stormwater Management Plan Report has been prepared to present analyses performed to address the potential impacts associated with the project due to proposed modification in stormwater runoff characteristics and land cover changes. The stormwater management controls that are outlined in this report have been designed to suit the proposed development and to comply with applicable regulatory requirements.

II. Existing Conditions

The project site consists of developed land located at 8 Wentworth Street in downtown Kittery. The site is approximately 0.8 acres in total consisting of the existing Rice Public Library building, gravel parking on the south end of the lot and paved parking on the north end. The site is bounded by Wentworth Street to the west and Traip Avenue on the southern and eastern sides of the site. The surrounding properties located on Traip Avenue consist of residential units while the buildings located on Wentworth Street are primarily for commercial use.

Slopes on the site range from approximately 3% across the paved parking lot to 45% in the landscape area between the southwestern corner of the library and the gravel lot. The majority of the site consists of slopes of 9-10% across the gravel parking lot south of the library and the surrounding landscaped areas.

The site is tributary to a system of storm sewers which outlet into the Piscataqua River. The river is not listed in Chapter 502 of the Maine Department of Environmental Protection (MDEP) regulations on *Direct Watersheds of Lakes Most at Risk From New Development, and Urban Impaired Streams*.

The proposed development area of the site is not located in an identified flood zone per the FEMA Flood Insurance Rate Map for the Kittery Community Panel 728, revised April 14, 2017.

III. Soils

Soil characteristics were obtained from the Class D: Medium Intensity Soil Survey completed by the United States Department of Agriculture Natural Resources Conservation Service Custom Web Soil Survey. The Hydrologic Groups (HSG) of the soils are classified by Technical Release TR-55 of the Soil Conservation Service as follows:

Soil Map Symbol	Soil Name	Slope (%)	HSG
Ur	Urban Land	0-8	D

Hydrologic Soil Group boundaries are delineated on the Watershed Map. A copy of the Class D Medium Intensity Soil Survey is included as Appendix 3.

IV. Proposed Site Improvements

The proposed development will consist of an approximately 3,660 square-foot addition to the existing Rice Public Library. The existing gravel parking lot on site will be transformed into a paved parking area with a new driveway entrance. The existing paved parking lot in the northern most area of the site will be redesigned as part of the project. Other site improvements include the redesign of pedestrian walkways on site as well as various landscape features.

V. Existing Conditions Model

The existing conditions watershed plan consists of one subcatchment labeled 1.0S in the HydroCAD model. One location was identified as the Study Point (SP) for comparing peak runoff rates for the site.

SP1 represents the drainage structure located south of the southeasterly corner of the site within Traip Avenue. Subcatchment 1.0S contributes runoff to this study point with an overall runoff area of approximately 0.84 acres. Runoff from the site enters a catch basin (1.0P) at the southeast corner of the site and then flows to SP1 via a corrugated metal pipe. SP1 and the associated drainage area are tributary to the Piscataqua River.

VI. Proposed Conditions Model

The proposed conditions watershed area consists of the same overall area as the existing conditions plan, however, the existing conditions subcatchment has been broken into smaller subcatchments as a result of the proposed development.

Post-development subcatchment 1.1S represents the southern half of the analyzed watershed. Runoff from this subcatchment area primarily consists of roof collection and paved parking, along with landscaped area. Subcatchment 1.2S represents the northern half of the watershed which is a majority landscaped area and more paved parking. All drainage in the proposed conditions flow to the existing municipal storm sewers located within Traip Avenue, represented by SP1, via a proposed catch basin and pipe network.

VII. Stormwater Management

Basic Standard - Chapter 500, Section 4(B)

The project will not disturb more than one (1) acre of land area, where MDEP Basic Standards apply, requiring that grading or other construction activities on the site do not impede or otherwise alter drainage ways to have an unreasonable adverse impact. However, we have avoided adverse impacts by providing an Erosion & Sedimentation Control Plan, and an Inspection, Maintenance and Housekeeping Plan (Appendix 2) to be implemented during construction and post-construction stabilization of the site. These construction requirements have been developed following Best Management Practice guidelines, latest edition.

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Runoff curve numbers were determined for each of the watersheds by measuring the area of each hydrologic soil group within each type of land cover. The type of land cover was determined based on survey data, field reconnaissance and aerial photography. Times of concentration were determined from site topographic maps in accordance with SCS procedures.

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The following table presents the results of the peak runoff calculations at the study point for the existing and proposed conditions.

Peak Runoff Rate Summary Table			
Analysis Point	Storm Event	Existing Conditions (cfs)	Proposed Conditions (cfs)
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	25-year	3.8	3.8

The HydroCAD Data output sheets from this analysis are appended to this report (Appendix 1) along with the Stormwater Management Plans (Appendix 4). The model predicts that the peak runoff rates in the proposed condition at Study Point 1 are equal to existing condition runoff rates for the 2 and 25-year storm events with implementation of the proposed stormwater management practices.

VIII. Summary

The proposed development has been designed to manage stormwater runoff through Best Management Practices approved by MDEP. Runoff discharging from the site will be the same as existing conditions for the 2 and 25-year storm events at the chosen study point. Additionally, erosion and sedimentation controls along with associated maintenance and housekeeping procedures have been outlined to prevent unreasonable impacts on the site and to the surrounding environment.

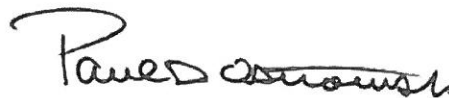
Prepared by:

SEBAGO TECHNICS, INC.



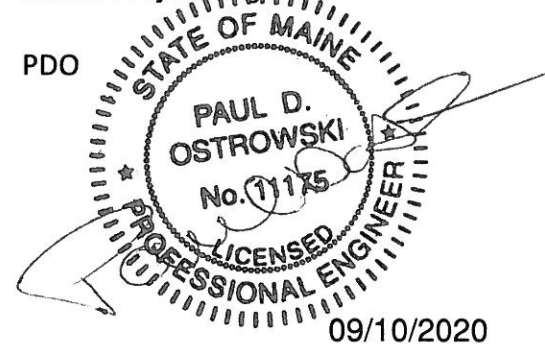
Mathew K. Orr, EI
Civil Engineer

MKO



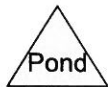
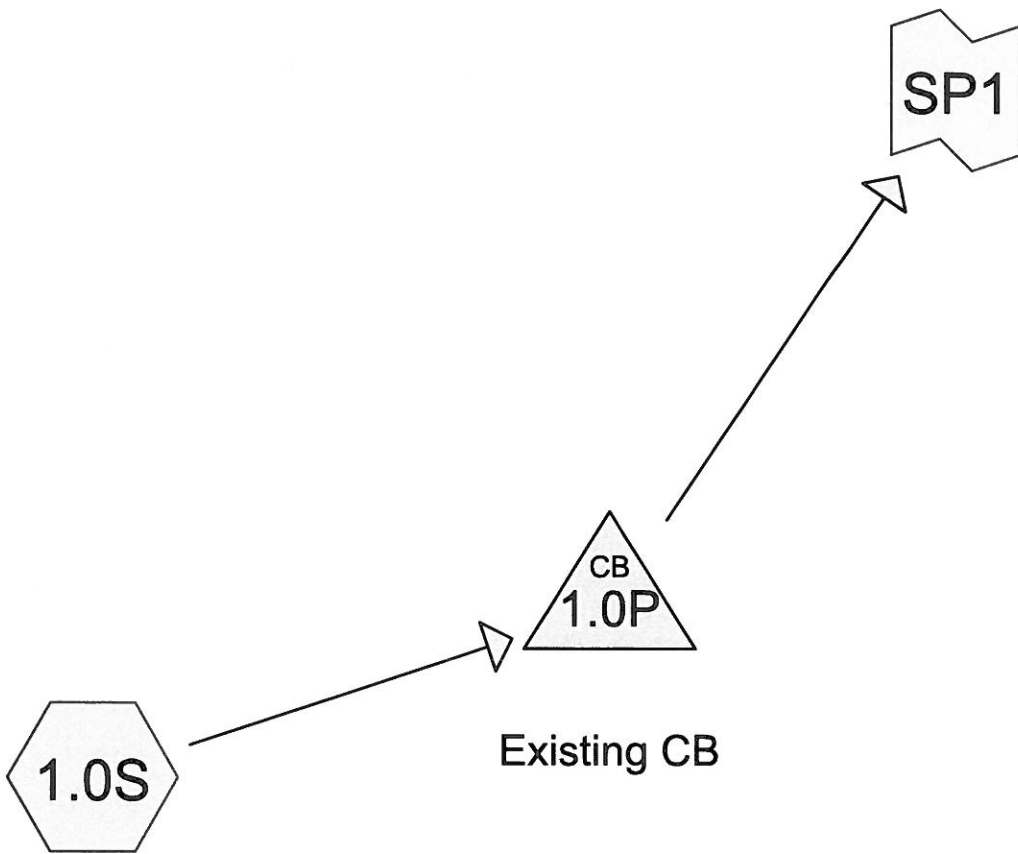
Paul D. Ostrowski, P.E.
Senior Project Engineer

PDO



Appendix 1A

**Hydrologic Modeling
Existing Conditions
HydroCAD Summary**



Area Listing (all nodes)

Area (sq-ft)	CN	Description (subcatchment-numbers)
16,845	80	>75% Grass cover, Good, HSG D (1.0S)
11,550	98	Existing Gravel (1.0S)
5,050	98	Existing Pavement (1.0S)
1,770	98	Existing Roof (1.0S)
1,240	98	Existing Walkways (1.0S)
36,455	90	TOTAL AREA

18438PRE

Type III 24-hr 2-YR Rainfall=3.10"

Prepared by Sebago Technics, Inc.

Printed 9/9/2020

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Page 3

Time span=0.00-50.00 hrs, dt=0.01 hrs, 5001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1.0S:

Runoff Area=36,455 sf 53.79% Impervious Runoff Depth=2.08"
Flow Length=242' Tc=6.0 min CN=90 Runoff=2.0 cfs 6,307 cf

Pond 1.0P: Existing CB

Peak Elev=17.04' Inflow=2.0 cfs 6,307 cf
12.0" Round Culvert n=0.025 L=38.0' S=0.0329 '/ Outflow=2.0 cfs 6,307 cf

Link SP1:

Inflow=2.0 cfs 6,307 cf
Primary=2.0 cfs 6,307 cf

Total Runoff Area = 36,455 sf Runoff Volume = 6,307 cf Average Runoff Depth = 2.08"
46.21% Pervious = 16,845 sf 53.79% Impervious = 19,610 sf

18438PRE

Type III 24-hr 25-YR Rainfall=5.20"

Prepared by Sebago Technics, Inc.

Printed 9/9/2020

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Page 4

Time span=0.00-50.00 hrs, dt=0.01 hrs, 5001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1.0S:

Runoff Area=36,455 sf 53.79% Impervious Runoff Depth=4.07"
Flow Length=242' Tc=6.0 min CN=90 Runoff=3.8 cfs 12,363 cf

Pond 1.0P: Existing CB

Peak Elev=18.20' Inflow=3.8 cfs 12,363 cf
12.0" Round Culvert n=0.025 L=38.0' S=0.0329 '/ Outflow=3.8 cfs 12,363 cf

Link SP1:

Inflow=3.8 cfs 12,363 cf
Primary=3.8 cfs 12,363 cf

Total Runoff Area = 36,455 sf Runoff Volume = 12,363 cf Average Runoff Depth = 4.07"
46.21% Pervious = 16,845 sf 53.79% Impervious = 19,610 sf

Summary for Subcatchment 1.0S:

Runoff = 3.8 cfs @ 12.08 hrs, Volume= 12,363 cf, Depth= 4.07"

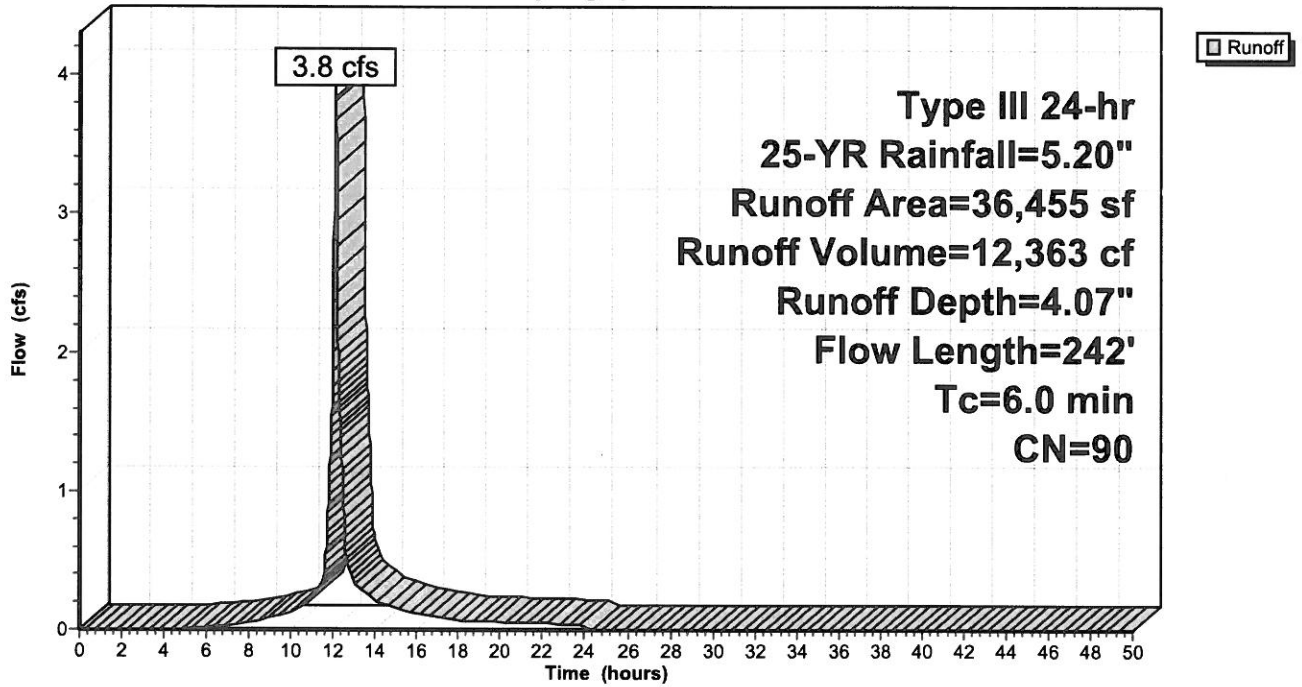
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-50.00 hrs, dt= 0.01 hrs
 Type III 24-hr 25-YR Rainfall=5.20"

	Area (sf)	CN	Description
*	1,770	98	Existing Roof
*	1,240	98	Existing Walkways
*	11,550	98	Existing Gravel
*	5,050	98	Existing Pavement
	16,845	80	>75% Grass cover, Good, HSG D
	36,455	90	Weighted Average
	16,845		46.21% Pervious Area
	19,610		53.79% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.4	39	0.0769	0.19		Sheet Flow, A-B Grass: Short n= 0.150 P2= 2.00"
0.3	52	0.1346	2.57		Shallow Concentrated Flow, B-C Short Grass Pasture Kv= 7.0 fps
0.4	92	0.0489	3.56		Shallow Concentrated Flow, C-D Unpaved Kv= 16.1 fps
0.6	59	0.0508	1.58		Shallow Concentrated Flow, D-E Short Grass Pasture Kv= 7.0 fps
1.3					Direct Entry,
6.0	242	Total			

Subcatchment 1.0S:

Hydrograph



Summary for Pond 1.0P: Existing CB

Inflow Area = 36,455 sf, 53.79% Impervious, Inflow Depth = 4.07" for 25-YR event
 Inflow = 3.8 cfs @ 12.08 hrs, Volume= 12,363 cf
 Outflow = 3.8 cfs @ 12.08 hrs, Volume= 12,363 cf, Atten= 0%, Lag= 0.0 min
 Primary = 3.8 cfs @ 12.08 hrs, Volume= 12,363 cf

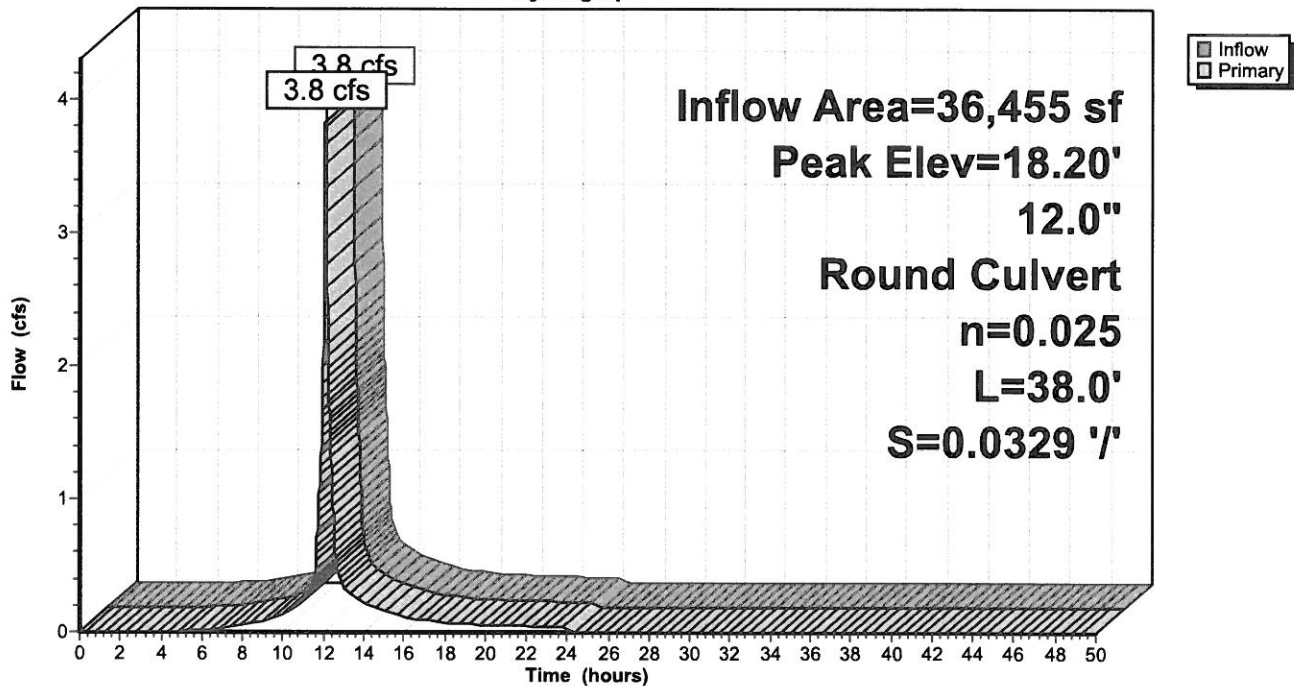
Routing by Dyn-Stor-Ind method, Time Span= 0.00-50.00 hrs, dt= 0.01 hrs
 Peak Elev= 18.20' @ 12.08 hrs
 Flood Elev= 18.75'

Device	Routing	Invert	Outlet Devices
#1	Primary	16.25'	12.0" Round CMP_Round 12" L= 38.0' CMP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 16.25' / 15.00' S= 0.0329 '/' Cc= 0.900 n= 0.025 Corrugated metal, Flow Area= 0.79 sf

Primary OutFlow Max=3.8 cfs @ 12.08 hrs HW=18.19' TW=0.00' (Dynamic Tailwater)
 ↳=CMP_Round 12" (Barrel Controls 3.8 cfs @ 4.88 fps)

Pond 1.0P: Existing CB

Hydrograph



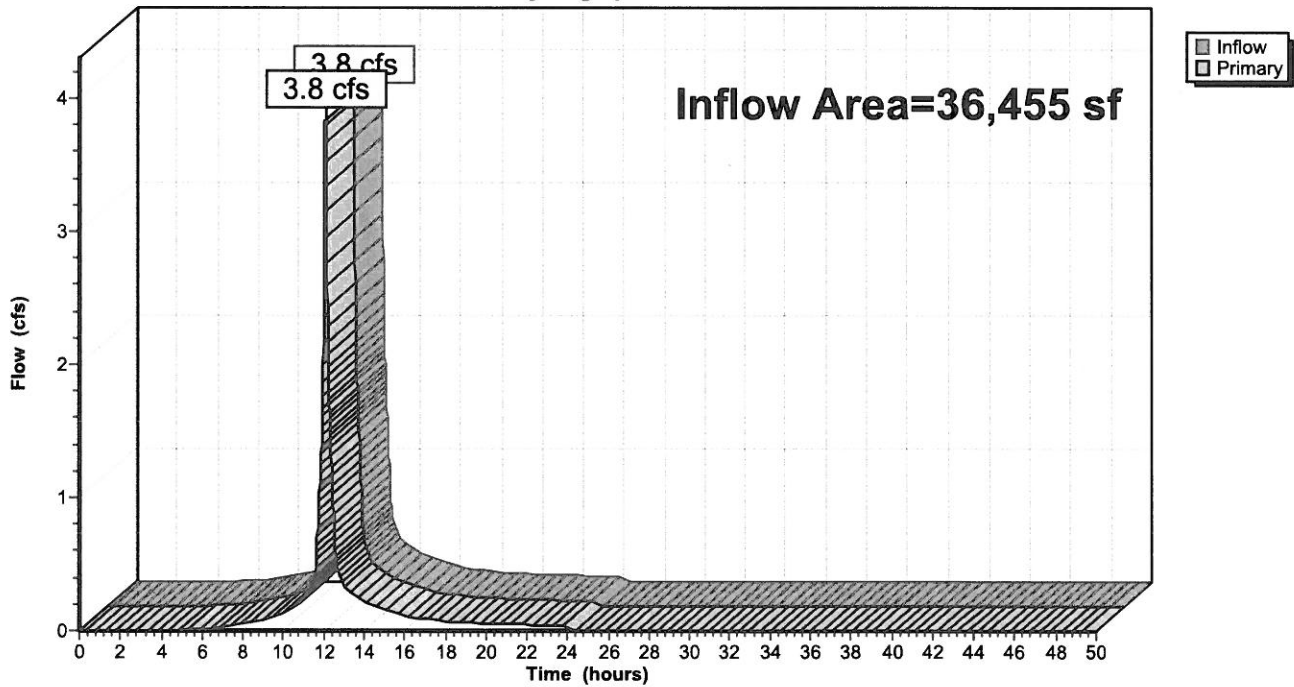
Summary for Link SP1:

Inflow Area = 36,455 sf, 53.79% Impervious, Inflow Depth = 4.07" for 25-YR event
Inflow = 3.8 cfs @ 12.08 hrs, Volume= 12,363 cf
Primary = 3.8 cfs @ 12.08 hrs, Volume= 12,363 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-50.00 hrs, dt= 0.01 hrs

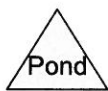
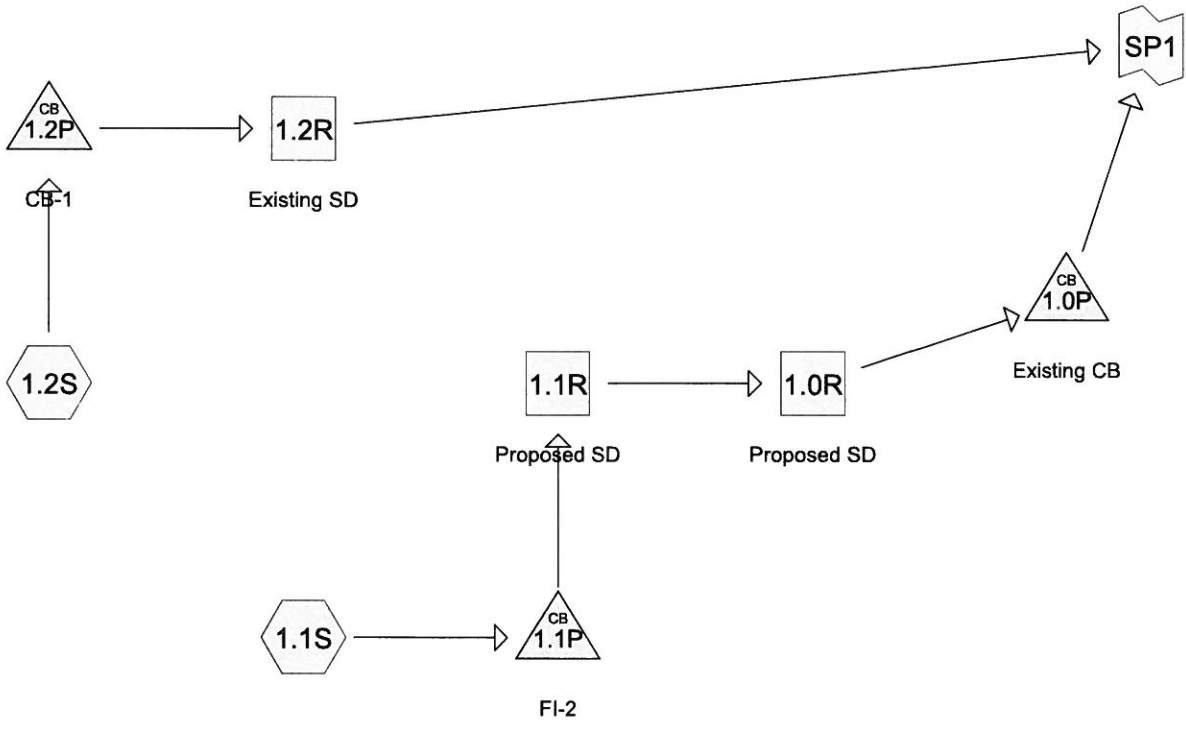
Link SP1:

Hydrograph



Appendix 1B

**Hydrologic Modeling
Proposed Conditions
HydroCAD Summary**



Area Listing (all nodes)

Area (sq-ft)	CN	Description (subcatchment-numbers)
16,120	80	>75% Grass cover, Good, HSG D (1.1S, 1.2S)
1,900	98	Existing Pavement (1.1S, 1.2S)
1,770	98	Existing Roof (1.1S)
490	98	Existing Walkways (1.1S, 1.2S)
450	98	Proposed Concrete (1.2S)
10,195	98	Proposed Pavement (1.1S, 1.2S)
325	98	Proposed Retaining Walls (1.1S)
3,680	98	Proposed Roof (1.1S)
1,525	98	Proposed Walkways (1.1S)
36,455	90	TOTAL AREA

Time span=0.00-50.00 hrs, dt=0.01 hrs, 5001 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1.1S: Runoff Area=20,560 sf 64.76% Impervious Runoff Depth=2.26"
 Flow Length=68' Tc=6.0 min CN=92 Runoff=1.2 cfs 3,865 cf

Subcatchment 1.2S: Runoff Area=15,895 sf 44.16% Impervious Runoff Depth=1.91"
 Flow Length=73' Tc=6.0 min CN=88 Runoff=0.8 cfs 2,526 cf

Reach 1.0R: Proposed SD Avg. Flow Depth=0.24' Max Vel=8.22 fps Inflow=1.2 cfs 3,865 cf
 12.0" Round Pipe n=0.013 L=16.0' S=0.0687 '/ Capacity=9.3 cfs Outflow=1.2 cfs 3,865 cf

Reach 1.1R: Proposed SD Avg. Flow Depth=0.32' Max Vel=6.46 fps Inflow=1.2 cfs 3,865 cf
 10.0" Round Pipe n=0.013 L=105.0' S=0.0336 '/ Capacity=4.0 cfs Outflow=1.2 cfs 3,865 cf

Reach 1.2R: Existing SD Avg. Flow Depth=0.30' Max Vel=4.15 fps Inflow=0.8 cfs 2,526 cf
 12.0" Round Pipe n=0.025 L=170.0' S=0.0521 '/ Capacity=4.2 cfs Outflow=0.8 cfs 2,526 cf

Pond 1.0P: Existing CB Peak Elev=16.83' Inflow=1.2 cfs 3,865 cf
 12.0" Round Culvert n=0.025 L=38.0' S=0.0329 '/ Outflow=1.2 cfs 3,865 cf

Pond 1.1P: FI-2 Peak Elev=22.07' Inflow=1.2 cfs 3,865 cf
 10.0" Round Culvert n=0.013 L=16.0' S=0.0131 '/ Outflow=1.2 cfs 3,865 cf

Pond 1.2P: CB-1 Peak Elev=24.77' Inflow=0.8 cfs 2,526 cf
 12.0" Round Culvert n=0.013 L=27.0' S=0.0126 '/ Outflow=0.8 cfs 2,526 cf

Link SP1: Inflow=2.0 cfs 6,391 cf
 Primary=2.0 cfs 6,391 cf

Total Runoff Area = 36,455 sf Runoff Volume = 6,391 cf Average Runoff Depth = 2.10"
44.22% Pervious = 16,120 sf 55.78% Impervious = 20,335 sf

Time span=0.00-50.00 hrs, dt=0.01 hrs, 5001 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1.1S: Runoff Area=20,560 sf 64.76% Impervious Runoff Depth=4.28"
 Flow Length=68' Tc=6.0 min CN=92 Runoff=2.2 cfs 7,341 cf

Subcatchment 1.2S: Runoff Area=15,895 sf 44.16% Impervious Runoff Depth=3.86"
 Flow Length=73' Tc=6.0 min CN=88 Runoff=1.6 cfs 5,112 cf

Reach 1.0R: Proposed SD Avg. Flow Depth=0.33' Max Vel=9.78 fps Inflow=2.2 cfs 7,341 cf
 12.0" Round Pipe n=0.013 L=16.0' S=0.0687 ' Capacity=9.3 cfs Outflow=2.2 cfs 7,341 cf

Reach 1.1R: Proposed SD Avg. Flow Depth=0.45' Max Vel=7.57 fps Inflow=2.2 cfs 7,341 cf
 10.0" Round Pipe n=0.013 L=105.0' S=0.0336 ' Capacity=4.0 cfs Outflow=2.2 cfs 7,341 cf

Reach 1.2R: Existing SD Avg. Flow Depth=0.43' Max Vel=5.01 fps Inflow=1.6 cfs 5,112 cf
 12.0" Round Pipe n=0.025 L=170.0' S=0.0521 ' Capacity=4.2 cfs Outflow=1.6 cfs 5,112 cf

Pond 1.0P: Existing CB Peak Elev=17.10' Inflow=2.2 cfs 7,341 cf
 12.0" Round Culvert n=0.025 L=38.0' S=0.0329 ' Outflow=2.2 cfs 7,341 cf

Pond 1.1P: FI-2 Peak Elev=22.58' Inflow=2.2 cfs 7,341 cf
 10.0" Round Culvert n=0.013 L=16.0' S=0.0131 ' Outflow=2.2 cfs 7,341 cf

Pond 1.2P: CB-1 Peak Elev=25.01' Inflow=1.6 cfs 5,112 cf
 12.0" Round Culvert n=0.013 L=27.0' S=0.0126 ' Outflow=1.6 cfs 5,112 cf

Link SP1: Inflow=3.8 cfs 12,453 cf
 Primary=3.8 cfs 12,453 cf

Total Runoff Area = 36,455 sf Runoff Volume = 12,453 cf Average Runoff Depth = 4.10"
44.22% Pervious = 16,120 sf 55.78% Impervious = 20,335 sf

Summary for Subcatchment 1.1S:

Runoff = 2.2 cfs @ 12.08 hrs, Volume= 7,341 cf, Depth= 4.28"

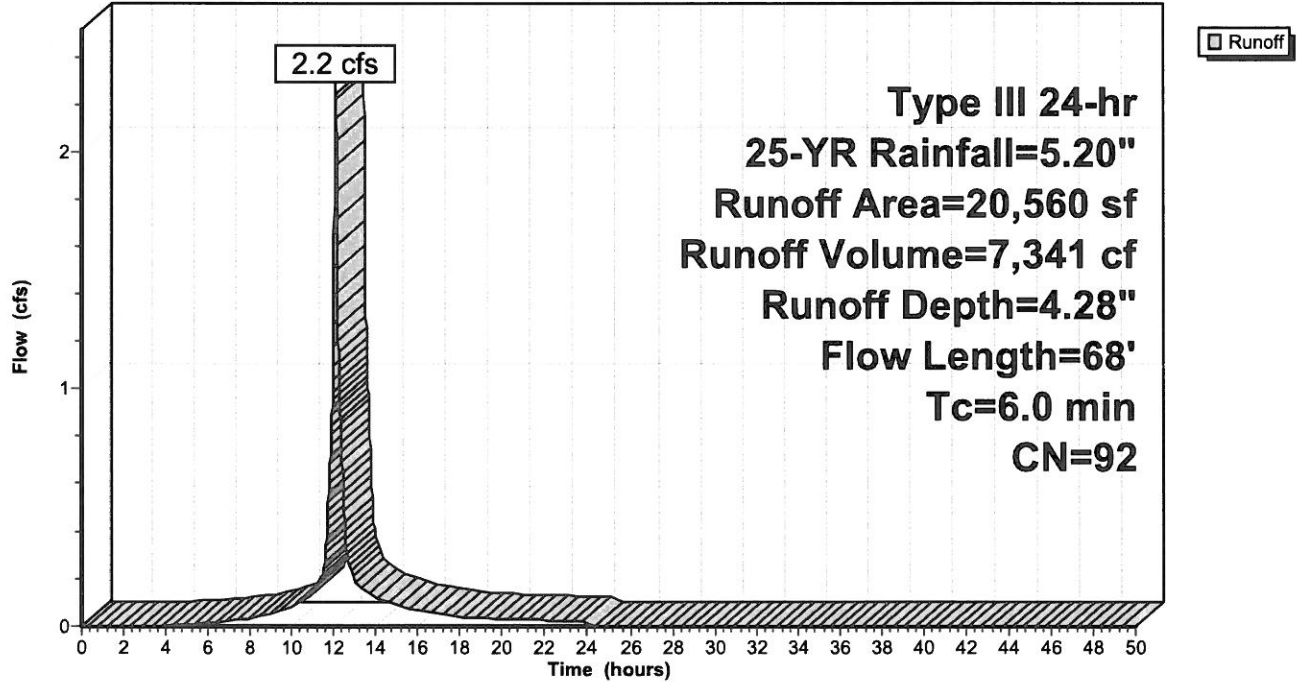
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-50.00 hrs, dt= 0.01 hrs
 Type III 24-hr 25-YR Rainfall=5.20"

	Area (sf)	CN	Description
*	1,770	98	Existing Roof
*	3,680	98	Proposed Roof
*	1,115	98	Existing Pavement
*	1,525	98	Proposed Walkways
*	325	98	Proposed Retaining Walls
*	4,735	98	Proposed Pavement
*	165	98	Existing Walkways
	7,245	80	>75% Grass cover, Good, HSG D
	20,560	92	Weighted Average
	7,245		35.24% Pervious Area
	13,315		64.76% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.4	23	0.0278	0.11		Sheet Flow, A-B Grass: Short n= 0.150 P2= 2.00"
0.0	13	0.0500	4.54		Shallow Concentrated Flow, B-C Paved Kv= 20.3 fps
0.2	32	0.1128	2.35		Shallow Concentrated Flow, C-D Short Grass Pasture Kv= 7.0 fps
2.4					Direct Entry,
6.0	68	Total			

Subcatchment 1.1S:

Hydrograph



Summary for Subcatchment 1.2S:

Runoff = 1.6 cfs @ 12.09 hrs, Volume= 5,112 cf, Depth= 3.86"

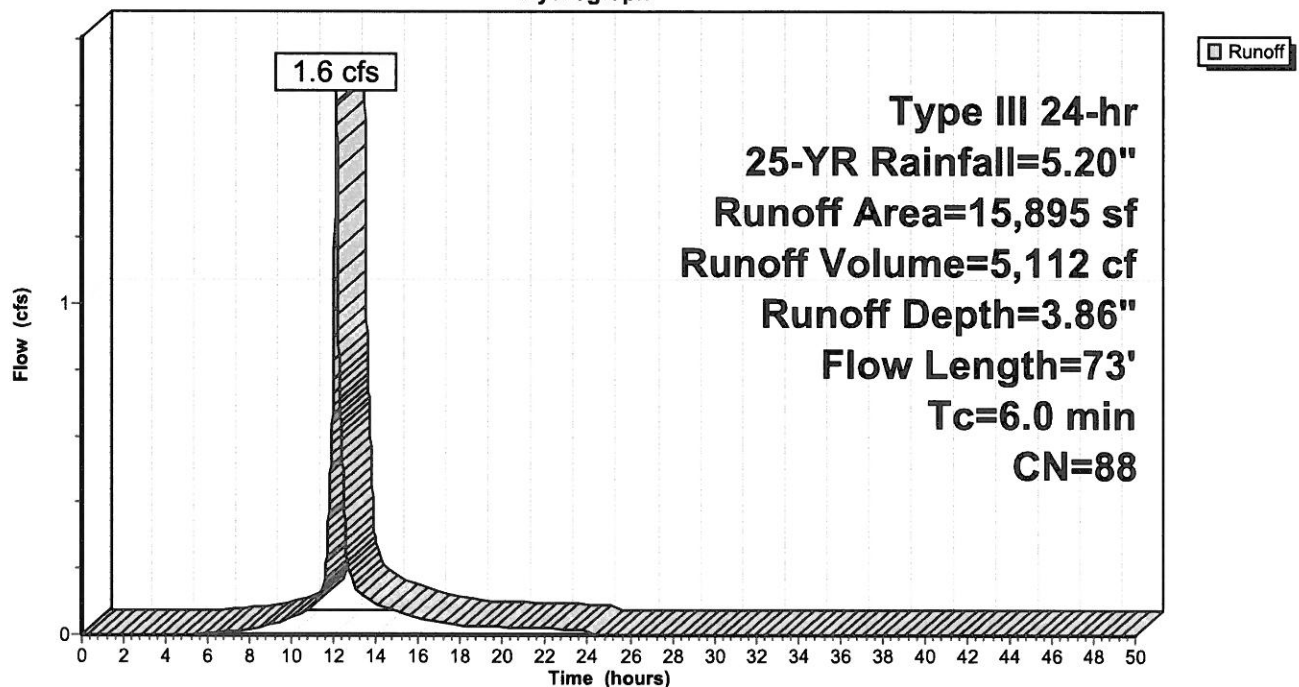
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-50.00 hrs, dt= 0.01 hrs
Type III 24-hr 25-YR Rainfall=5.20"

	Area (sf)	CN	Description
*	785	98	Existing Pavement
*	5,460	98	Proposed Pavement
*	450	98	Proposed Concrete
*	325	98	Existing Walkways
	8,875	80	>75% Grass cover, Good, HSG D
	15,895	88	Weighted Average
	8,875		55.84% Pervious Area
	7,020		44.16% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.9	34	0.0882	0.20		Sheet Flow, A-B Grass: Short n= 0.150 P2= 2.00"
0.2	39	0.1410	2.63		Shallow Concentrated Flow, B-C Short Grass Pasture Kv= 7.0 fps
2.9					Direct Entry,
6.0	73	Total			

Subcatchment 1.2S:

Hydrograph



Summary for Reach 1.0R: Proposed SD

Inflow Area = 20,560 sf, 64.76% Impervious, Inflow Depth = 4.28" for 25-YR event
 Inflow = 2.2 cfs @ 12.09 hrs, Volume= 7,341 cf
 Outflow = 2.2 cfs @ 12.09 hrs, Volume= 7,341 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-50.00 hrs, dt= 0.01 hrs
 Max. Velocity= 9.78 fps, Min. Travel Time= 0.0 min
 Avg. Velocity = 3.20 fps, Avg. Travel Time= 0.1 min

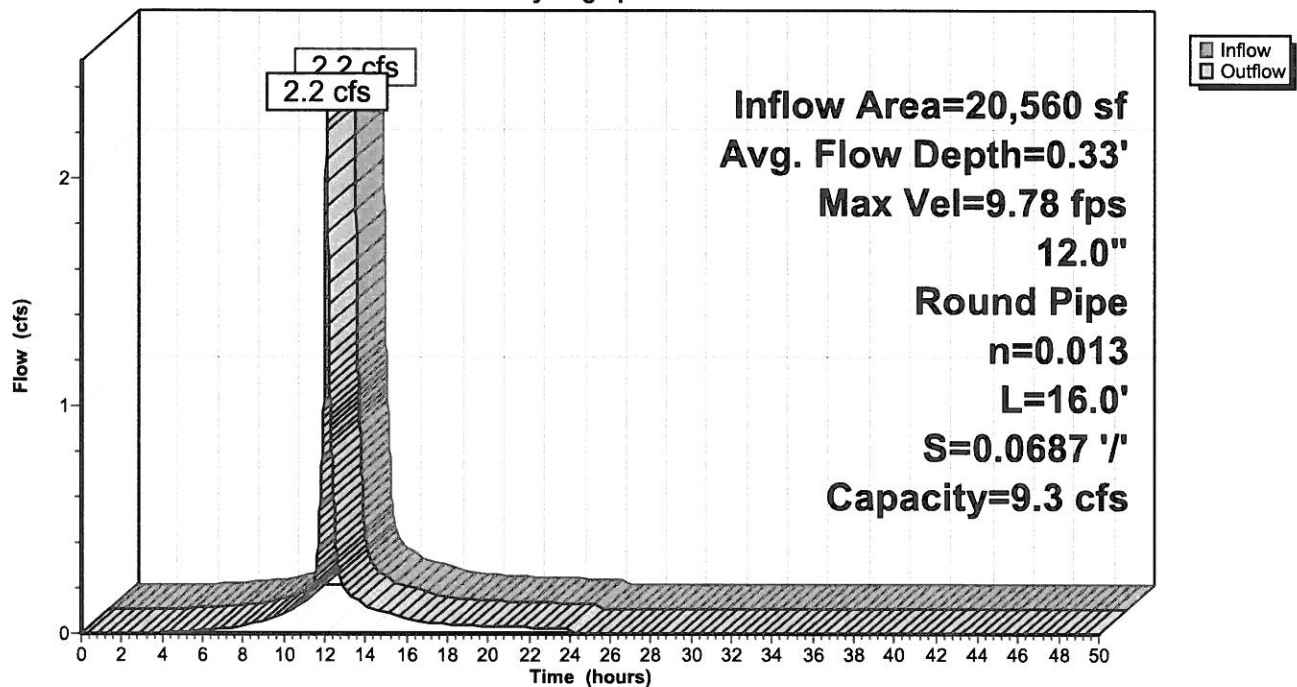
Peak Storage= 4 cf @ 12.09 hrs
 Average Depth at Peak Storage= 0.33'
 Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 9.3 cfs

12.0" Round Pipe
 n= 0.013 Corrugated PE, smooth interior
 Length= 16.0' Slope= 0.0687 '/'
 Inlet Invert= 17.45', Outlet Invert= 16.35'



Reach 1.0R: Proposed SD

Hydrograph



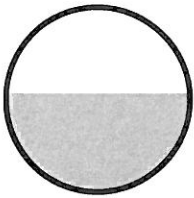
Summary for Reach 1.1R: Proposed SD

Inflow Area = 20,560 sf, 64.76% Impervious, Inflow Depth = 4.28" for 25-YR event
 Inflow = 2.2 cfs @ 12.08 hrs, Volume= 7,341 cf
 Outflow = 2.2 cfs @ 12.09 hrs, Volume= 7,341 cf, Atten= 0%, Lag= 0.2 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-50.00 hrs, dt= 0.01 hrs
 Max. Velocity= 7.57 fps, Min. Travel Time= 0.2 min
 Avg. Velocity = 2.55 fps, Avg. Travel Time= 0.7 min

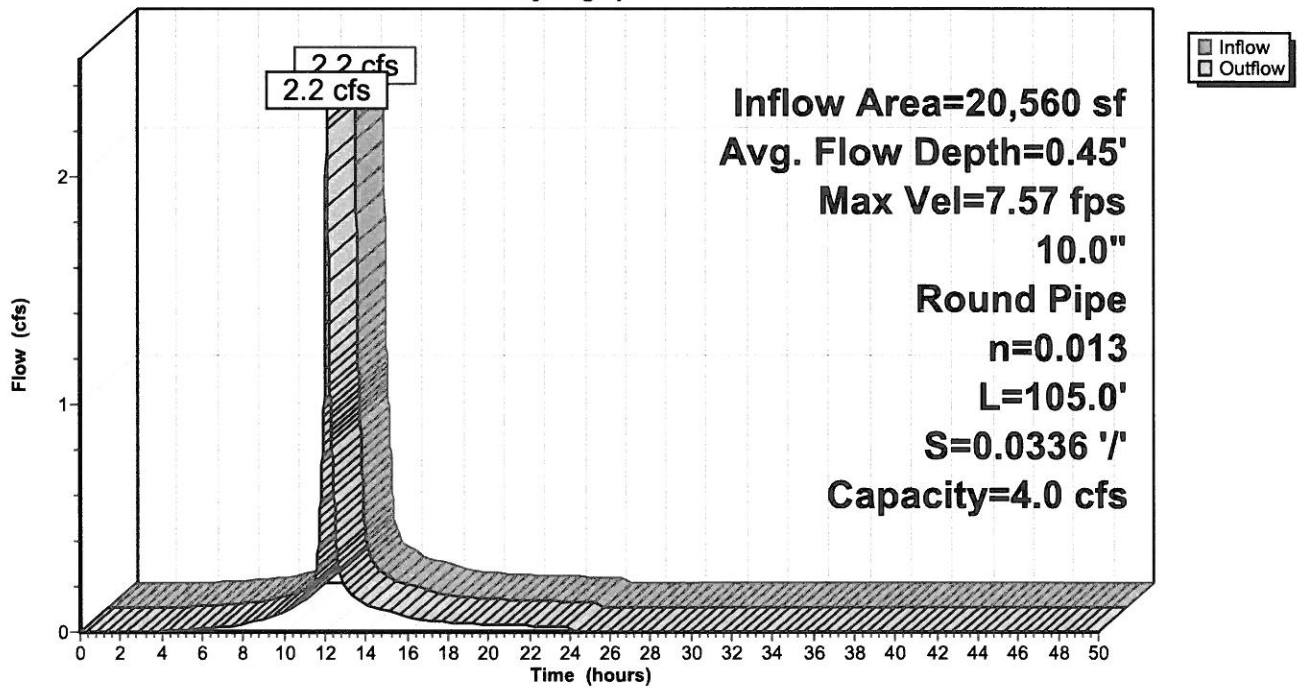
Peak Storage= 31 cf @ 12.09 hrs
 Average Depth at Peak Storage= 0.45'
 Bank-Full Depth= 0.83' Flow Area= 0.5 sf, Capacity= 4.0 cfs

10.0" Round Pipe
 n= 0.013 Corrugated PE, smooth interior
 Length= 105.0' Slope= 0.0336 '/'
 Inlet Invert= 21.08', Outlet Invert= 17.55'



Reach 1.1R: Proposed SD

Hydrograph



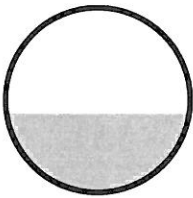
Summary for Reach 1.2R: Existing SD

Inflow Area = 15,895 sf, 44.16% Impervious, Inflow Depth = 3.86" for 25-YR event
 Inflow = 1.6 cfs @ 12.09 hrs, Volume= 5,112 cf
 Outflow = 1.6 cfs @ 12.09 hrs, Volume= 5,112 cf, Atten= 0%, Lag= 0.4 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-50.00 hrs, dt= 0.01 hrs
 Max. Velocity= 5.01 fps, Min. Travel Time= 0.6 min
 Avg. Velocity = 1.67 fps, Avg. Travel Time= 1.7 min

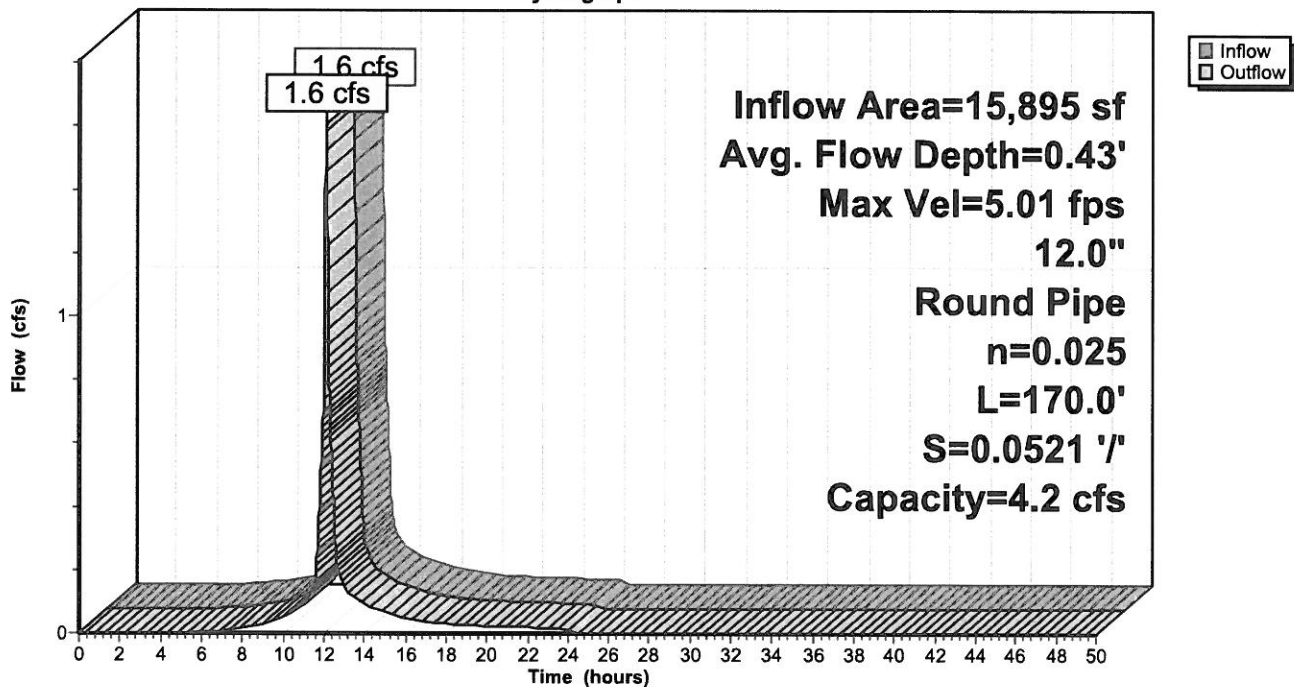
Peak Storage= 54 cf @ 12.09 hrs
 Average Depth at Peak Storage= 0.43'
 Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 4.2 cfs

12.0" Round Pipe
 n= 0.025 Corrugated metal
 Length= 170.0' Slope= 0.0521 '/
 Inlet Invert= 23.86', Outlet Invert= 15.00'



Reach 1.2R: Existing SD

Hydrograph



Summary for Pond 1.0P: Existing CB

Inflow Area = 20,560 sf, 64.76% Impervious, Inflow Depth = 4.28" for 25-YR event
 Inflow = 2.2 cfs @ 12.09 hrs, Volume= 7,341 cf
 Outflow = 2.2 cfs @ 12.09 hrs, Volume= 7,341 cf, Atten= 0%, Lag= 0.0 min
 Primary = 2.2 cfs @ 12.09 hrs, Volume= 7,341 cf

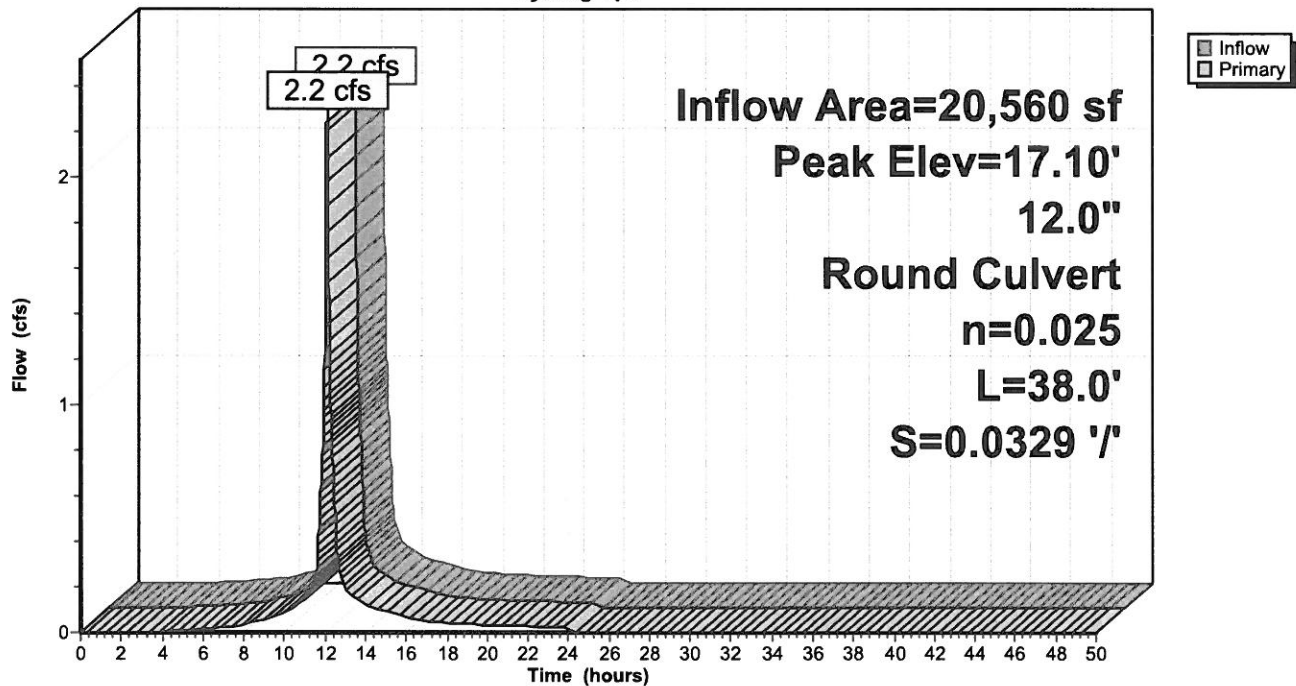
Routing by Dyn-Stor-Ind method, Time Span= 0.00-50.00 hrs, dt= 0.01 hrs
 Peak Elev= 17.10' @ 12.09 hrs
 Flood Elev= 18.75'

Device	Routing	Invert	Outlet Devices
#1	Primary	16.25'	12.0" Round CMP_Round 12" L= 38.0' CMP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 16.25' / 15.00' S= 0.0329 '/ Cc= 0.900 n= 0.025 Corrugated metal, Flow Area= 0.79 sf

Primary OutFlow Max=2.2 cfs @ 12.09 hrs HW=17.10' TW=0.00' (Dynamic Tailwater)
 ↳ 1=CMP_Round 12" (Inlet Controls 2.2 cfs @ 3.14 fps)

Pond 1.0P: Existing CB

Hydrograph



Summary for Pond 1.1P: FI-2

Inflow Area = 20,560 sf, 64.76% Impervious, Inflow Depth = 4.28" for 25-YR event
 Inflow = 2.2 cfs @ 12.08 hrs, Volume= 7,341 cf
 Outflow = 2.2 cfs @ 12.08 hrs, Volume= 7,341 cf, Atten= 0%, Lag= 0.0 min
 Primary = 2.2 cfs @ 12.08 hrs, Volume= 7,341 cf

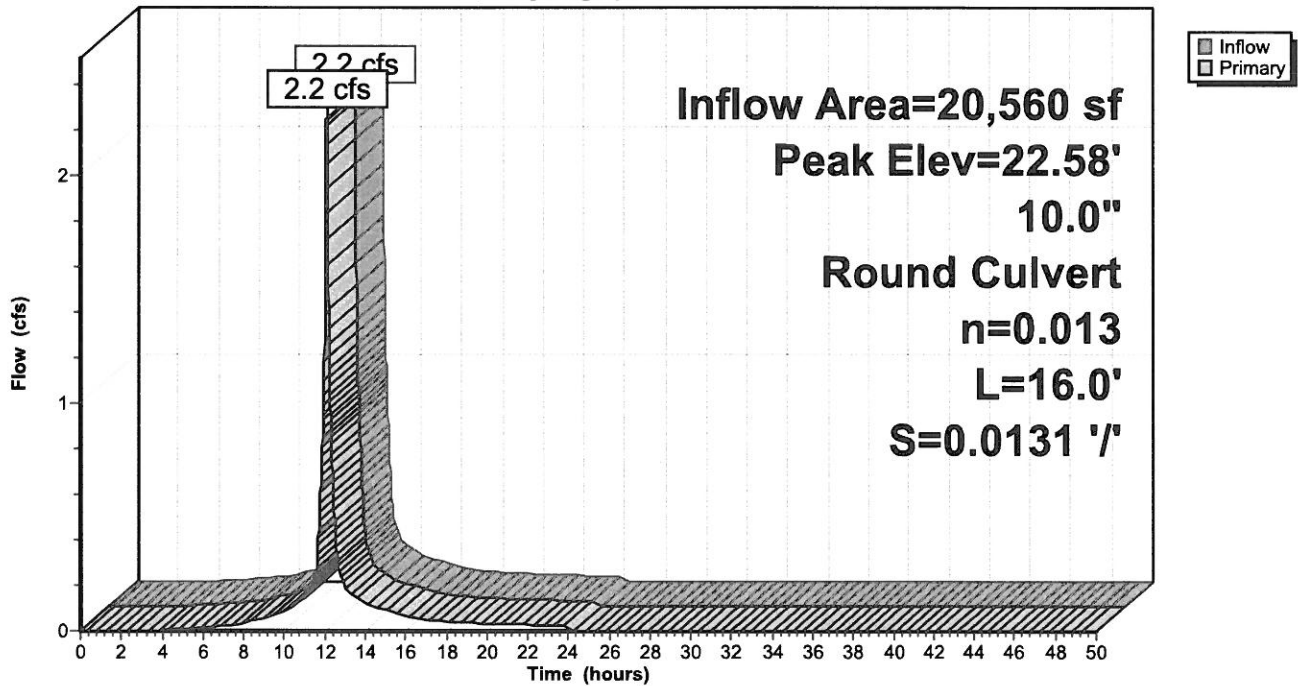
Routing by Dyn-Stor-Ind method, Time Span= 0.00-50.00 hrs, dt= 0.01 hrs
 Peak Elev= 22.58' @ 12.08 hrs
 Flood Elev= 32.38'

Device	Routing	Invert	Outlet Devices
#1	Primary	21.39'	10.0" Round SD-5 L= 16.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 21.39' / 21.18' S= 0.0131 '/ Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.55 sf

Primary OutFlow Max=2.2 cfs @ 12.08 hrs HW=22.57' TW=21.52' (Dynamic Tailwater)
 ↳1=SD-5 (Barrel Controls 2.2 cfs @ 4.11 fps)

Pond 1.1P: FI-2

Hydrograph



Summary for Pond 1.2P: CB-1

Inflow Area = 15,895 sf, 44.16% Impervious, Inflow Depth = 3.86" for 25-YR event
 Inflow = 1.6 cfs @ 12.09 hrs, Volume= 5,112 cf
 Outflow = 1.6 cfs @ 12.09 hrs, Volume= 5,112 cf, Atten= 0%, Lag= 0.0 min
 Primary = 1.6 cfs @ 12.09 hrs, Volume= 5,112 cf

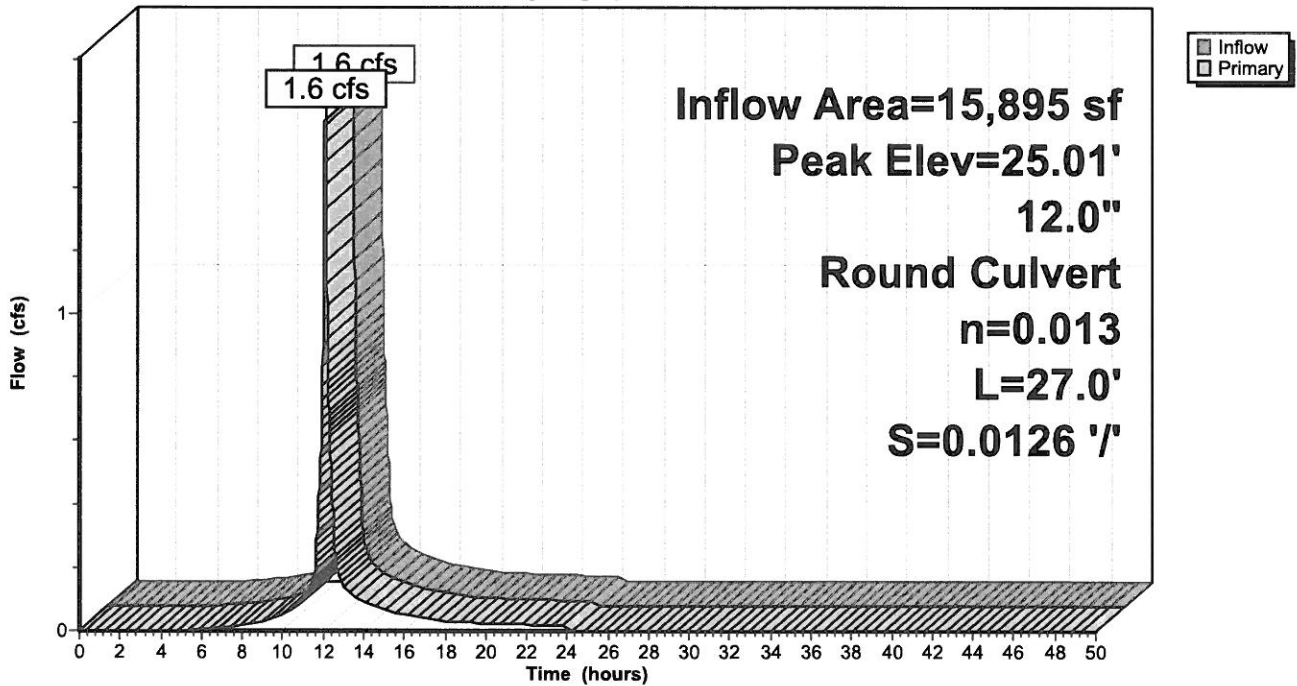
Routing by Dyn-Stor-Ind method, Time Span= 0.00-50.00 hrs, dt= 0.01 hrs
 Peak Elev= 25.01' @ 12.09 hrs
 Flood Elev= 28.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	24.30'	12.0" Round SD-1 L= 27.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 24.30' / 23.96' S= 0.0126 '/ Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.6 cfs @ 12.09 hrs HW=25.01' TW=24.29' (Dynamic Tailwater)
 ↳1=SD-1 (Barrel Controls 1.6 cfs @ 3.80 fps)

Pond 1.2P: CB-1

Hydrograph



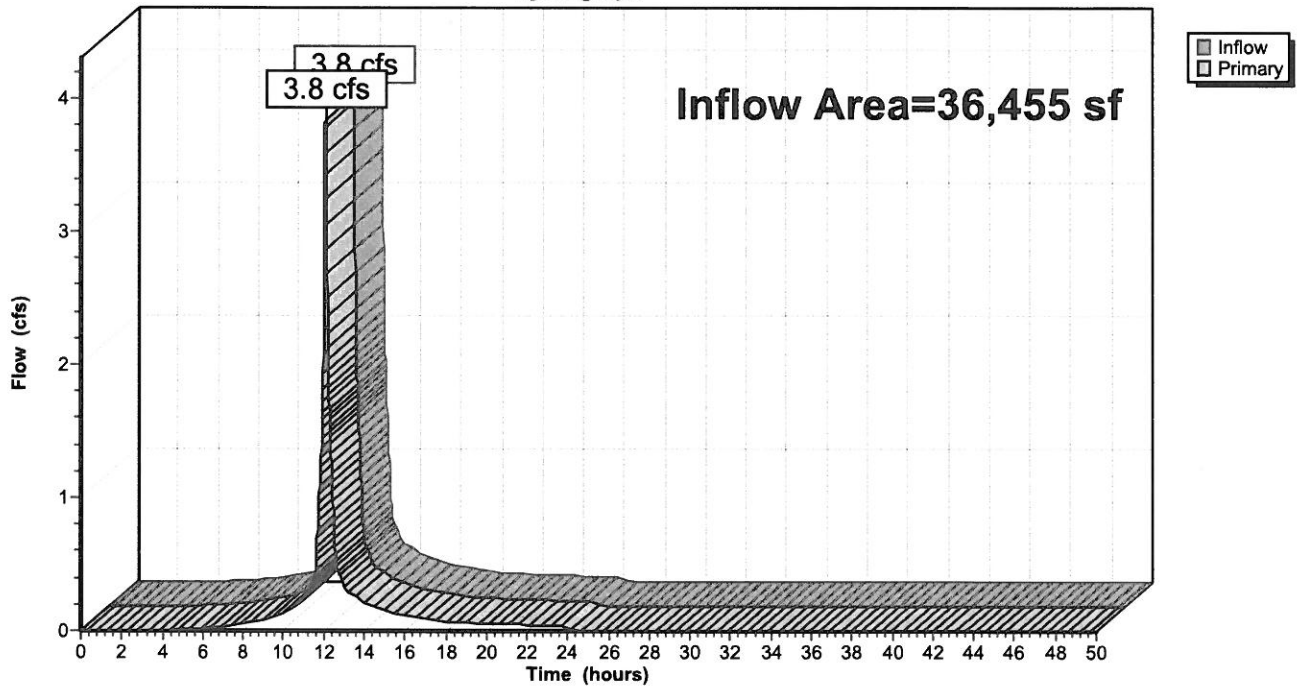
Summary for Link SP1:

Inflow Area = 36,455 sf, 55.78% Impervious, Inflow Depth = 4.10" for 25-YR event
Inflow = 3.8 cfs @ 12.09 hrs, Volume= 12,453 cf
Primary = 3.8 cfs @ 12.09 hrs, Volume= 12,453 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-50.00 hrs, dt= 0.01 hrs

Link SP1:

Hydrograph



Appendix 2

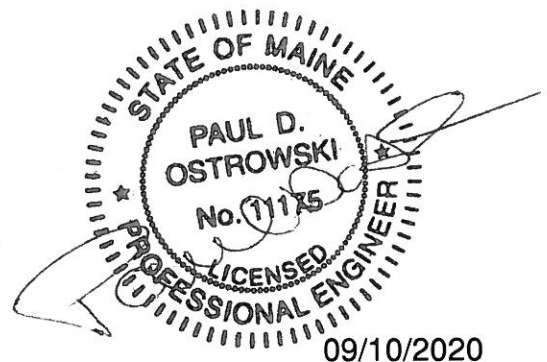
Inspection, Maintenance and Housekeeping Plan



INSPECTION, MAINTENANCE, AND HOUSEKEEPING PLAN

For:
Rice Public Library
8 Wentworth Street
Kittery, Maine

By:
Sebago Technics, Inc.
75 John Roberts Road, Suite 4A
South Portland, Maine



Introduction

The following plan outlines the anticipated inspection and maintenance procedures for the erosion and sedimentation control measures as well as stormwater management facilities for the project. This plan also outlines several housekeeping requirements that shall be followed during and after construction. These procedures shall be followed in order to ensure the intended function of the designed measures and to prevent unreasonably adverse impacts to the surrounding environment.

The procedures outlined in this Inspection, Maintenance and Housekeeping Plan are provided as an overview of the anticipated practices to be used on this site. In some instances, additional measures may be required due to unexpected conditions. For additional detail on any of the erosion and sedimentation control measures or stormwater management devices to be utilized on this project, refer to the most recently revised edition of the "Maine Erosion and Sedimentation Control BMP" manual and/or the "Stormwater Management for Maine: Best Management Practices" manual as published by the Maine Department of Environmental Protection (MDEP).

During Construction

- 1. Inspection: During the construction process, it is the Contractor's responsibility to comply with the inspection and maintenance procedures outlined in this section. These responsibilities include inspecting disturbed and impervious areas, erosion control measures, materials storage areas that are exposed to precipitation, and locations where vehicles enter or exit the site. These areas shall be inspected at least once a week as well as before and after a storm event (0.5" of rainfall), and prior to completing permanent stabilization measures. A person with knowledge of erosion and stormwater control, including the standards and conditions in any applicable permits, shall conduct the inspections.
2. Maintenance: All measures shall be maintained in an effective operating condition until areas are permanently stabilized. If Best Management Practices (BMPs) need to be maintained or modified, additional BMPs are necessary, or other corrective action is needed, implementation must be completed within 7 calendar days and prior to any storm event (0.5" of rainfall).
3. Documentation: A log summarizing the inspections and any corrective action taken must be maintained on-site. The log must include the name(s) and qualifications of the person making the inspections, the date(s) of the inspections, and major observations about the operation and maintenance of erosion and sedimentation controls, material storage areas, and vehicle access

points to the site. Major observations must include BMPs that need maintenance, BMPs that failed to operate as designed or proved inadequate for a particular location, and locations where additional BMPs are needed. For each BMP requiring maintenance, BMP needing replacement, and location needing additional BMPs, note in the log the corrective action taken and when it was taken. The log must be made accessible to the appropriate regulatory agency upon request. The permittee shall retain a copy of the log for a period of at least three years from the completion of permanent stabilization.

4. **Specific Inspection and Maintenance Tasks:** The following is a list of erosion control and stormwater management measures and the specific inspection and maintenance tasks to be performed during construction.

A. Sediment Barriers:

- Hay bale barriers, silt fences, and filter berms shall be inspected immediately after each rainfall and at least daily during prolonged rainfall.
- If the fabric on a silt fence or filter barrier should decompose or become ineffective prior to the end of the expected usable life and the barrier is still necessary, it shall be replaced.
- Sediment deposits should be removed after each storm event (0.5" of rainfall). They must be removed before deposits reach approximately one-half the height of the barrier.
- Filter berms shall be reshaped as needed.
- Any sediment deposits remaining in place after the silt fence or filter barrier is no longer required should be dressed to conform to the existing grade, prepared, and seeded.

B. Riprap Materials:

- Once a riprap installation has been completed, it should require very little maintenance. It shall, however, be inspected periodically to determine if high flows have caused scour beneath the riprap or dislodged any of the stone.

C. Erosion Control Blankets:

- Inspect these reinforced areas semi-annually and after significant rainfall events for slumping, sliding, seepage, and scour. Pay close attention to unreinforced areas adjacent to the erosion control blankets, which may experience accelerated erosion.
- Review all applicable inspection and maintenance procedures recommended by the specific blanket manufacturer. These tasks shall be included in addition to the requirements of this plan.

D. Stabilized Construction Entrances/Exits:

- The exit shall be maintained in a condition that will prevent tracking of sediment onto public rights-of-way.
- When the control pad becomes ineffective, the stone shall be removed along with the collected soil material. The entrance should then be reconstructed.
- Areas that have received mud-tracking or sediment deposits shall be swept or

washed. Washing shall be done on an area stabilized with aggregate, which drains into an approved sediment-trapping device (not into storm drains, ditches, or waterways).

E. Temporary Seed and Mulch:

- Mulched areas should be inspected after rain events to check for rill erosion.
- If less than 90% of the soil surface is covered by mulch, additional mulch shall be applied in bare areas.
- In applications where seeding and mulch have been applied in conjunction with erosion control blankets, the blankets must be inspected after rain events for dislocation or undercutting.
- Mulch shall continue to be reapplied until 95% of the soil surface has established temporary vegetative cover.

F. Stabilized Temporary Drainage Swales:

- Sediment accumulation in the swale shall be removed once the cross section of the swale is reduced by 25%.
- The swales shall be inspected after rainfall events. Any evidence of sloughing of the side slopes or channel erosion shall be repaired and corrective action should be taken to prevent reoccurrence of the problem.
- In addition to the stabilized lining of the channel (i.e. erosion control blankets), stone check dams may be needed to further reduce channel velocity.

5. **Housekeeping:** The following general performance standards apply to the proposed project.

- A. Spill prevention: Controls must be used to prevent pollutants from being discharged from materials on-site, including storage practices to minimize exposure of the materials to stormwater, and appropriate spill prevention, containment, and response planning and implementation.
- B. Groundwater protection: During construction, liquid petroleum products and other hazardous materials with the potential to contaminate groundwater may not be stored or handled in areas of the site draining to an infiltration area. An "infiltration area" is any area of the site that by design or as a result of soils, topography and other relevant factors, accumulates runoff that infiltrates into the soil. Dikes, berms, sumps, and other forms of secondary containment that prevent discharge to groundwater may be used to isolate portions of the site for the purposes of storage and handling of these materials.
- C. Fugitive sediment and dust: Actions must be taken to ensure that activities do not result in noticeable erosion of soils or fugitive dust emissions during or after construction. Oil may not be used for dust control.
- D. Debris and other materials: Litter, construction debris, and chemicals exposed to stormwater must be prevented from becoming a pollutant source.
- E. Trench or foundation dewatering: Trench dewatering is the removal of water from

trenches, foundations, cofferdams, ponds, and other areas within the construction area that retain water after excavation. In most cases, the collected water is heavily silted and hinders correct and safe construction practices. The collected water must be removed from the ponded area, either through gravity or pumping, and must be spread through natural wooded buffers or removed to areas that are specifically designed to collect the maximum amount of sediment possible, like a cofferdam sedimentation basin. Avoid allowing the water to flow over disturbed areas of the site. Equivalent measures may be taken if approved.

Post-Construction

1. **Inspection:** After construction, it is the responsibility of the owner or assigned heirs to comply with the inspection and maintenance procedures outlined in this section. All measures must be maintained in effective operating condition. The owner shall inspect and maintain the BMPs, including but not limited to any parking areas, catch basins, drainage swales, detention basins and ponds, pipes and related structures, in accordance with all municipal and state inspection, cleaning and maintenance requirements of the approved post-construction stormwater management plan.

2. **Specific Inspection and Maintenance Tasks:** The following is a list of permanent erosion control and stormwater management measures and the inspection and maintenance tasks to be performed after construction. If the BMP requires maintenance, repair or replacement to function as intended by the approved post-construction stormwater management plan, the owner or operator of the BMP shall take corrective action(s) to address the deficiency or deficiencies as soon as possible after the deficiency is discovered and shall provide a record of the deficiency and corrective action(s) to the local municipality in the annual report.
 - A. **Vegetated Areas:**
 - Inspect vegetated areas, particularly slopes and embankments, early in the growing season or after heavy rains (>0.5") to identify active or potential erosion problems.
 - Replant bare areas or areas with sparse growth. Where rill erosion is evident, armor the area with an appropriate lining or divert the erosive flows to on-site areas able to withstand the concentrated flows.

 - B. **Ditches, Swales and Other Open Channels:**
 - Inspect ditches, swales, level spreaders and other open stormwater channels in the spring, in the late fall, and after heavy rains to remove any obstructions to flow. Remove accumulated sediments and debris, remove woody vegetative growth that could obstruct flow, and repair any erosion of the ditch lining.
 - Vegetated ditches must be mowed at least annually or otherwise maintained to control the growth of woody vegetation and maintain flow capacity.
 - Any woody vegetation growing through riprap linings must also be removed. Repair any slumping side slopes as soon as practicable.
 - If the ditch has a riprap lining, replace riprap in areas where any underlying filter fabric or underdrain gravel is showing through the stone or where stones have dislodged.

C. Culverts:

- Inspect culverts in the spring, in the late fall, and after heavy rains (>0.5") to remove any obstructions to flow.
- Remove accumulated sediments and debris at the inlet, at the outlet, and within the conduit.
- Inspect and repair any erosion damage at the culvert's inlet and outlet.

D. Removal of Winter Sand:

- Clear accumulations of winter sand in parking lots and along roadways at least once a year, preferably in the spring.
- Accumulations on pavement may be removed by pavement sweeping.
- Accumulations of sand along road shoulders may be removed by grading excess sand to the pavement edge and removing it manually or by a front-end loader or other acceptable method.

E. Outlet Control Structures:

- Inspect outlet structures two times per year (preferably in spring and fall) to ensure that the outlet structures are working in their intended fashion and that they are free of debris.
- Clean structures when sediment depths reach 12 inches from invert of outlet.
- At a minimum, remove floating debris and hydrocarbons at the time of the inspection.

3. Documentation:

- A. The owner or operator of a BMP or a qualified post-construction stormwater inspector hired by that person, shall, as required by the local municipality, provide a completed and signed certification on a form provided by the local municipality, certifying that the person has inspected the BMP(s) and that they are adequately maintained and functioning as intended by the approved post-construction stormwater management plan, or that they required maintenance or repair, including the record of the deficiency and corrective action(s) taken.
- B. A log summarizing the inspections and any corrective action taken must be maintained. The log must include the name(s) and qualifications of the person making the inspections, the date(s) of the inspections, and major observations about the operation and maintenance of controls. Major observations must include BMPs that need maintenance, BMPs that failed to operate as designed or proved inadequate for a particular location, and locations where additional BMPs are needed. For each BMP requiring maintenance, BMP needing replacement, and location needing additional BMPs, note in the log the corrective action taken and when it was taken. The log must be made accessible to the appropriate regulatory agency upon request. A sample "Stormwater Inspection and Maintenance Form" has been included as Attachment 1 of this Inspection, Maintenance, and Housekeeping Plan.

4. **Duration of Maintenance:** Perform maintenance as described and required for any associated permits unless and until the system is formally accepted by a municipality or quasi-municipal district, or is placed under the jurisdiction of a legally created association that will be responsible for the maintenance of the system. If a municipality or quasi-municipal district chooses to accept a stormwater management system, or a component of a stormwater system, it must provide a letter to the MDEP stating that it assumes responsibility for the system. The letter must specify the components of the system for which the municipality or district will assume responsibility, and that the municipality or district agrees to maintain those components of the system in compliance with MDEP standards. Upon such assumption of responsibility, and approval by the MDEP, the municipality, quasi-municipal district, or association becomes a co-permittee for this purpose only and must comply with all terms and conditions of the permit.

Attachments

Attachment 1 – Stormwater Inspection Maintenance and Housekeeping Log Form

General Site

INSPECTION MAINTENANCE AND HOUSEKEEPING FORM			
General Information			
Project Name:		Inspection Date:	
Project Location:		Current Weather:	
		Date / Amount Last Precip:	
BMP Owner:		Company conducting inspection:	
Owner Mailing Address:		Company Mailing Address	
Owner Phone #:		Company Phone #:	
Owner Email:		Inspector Name:	
		Inspector Email:	
Site Element	Suggested Maintenance (recom'd frequency)	Observations	Inspection Notes/Recommended Action
Vegetated Areas	Inspect Slopes/Embankments for erosion (annually)		
	Replant bare areas or areas of sparse growth (annually)		
Paved Surfaces	Clear accumulated winter sand (annually)		
	Remove sediment along edges of parking and within low spots/pockets (annually)		
Ditches/Swales	Remove obstructions/debris/sediment (monthly)		
	Inspect for erosion/repair as needed (annually)		
	Remove woody vegetation (annually)		
	Mow vegetated ditches (annually)		
Catch Basins	Remove sediment/debris from sump (annually)		
Culverts	Remove sediment/debris from inlet/outlet aprons (annually)		
	Inspect inlet/outlet aprons for erosion, repair as needed (annually)		
	Inspect, repair as needed, riprap aprons for dislodged/sparse coverage (annually)		
Pipe Outlets	Remove sediment/debris from outlet aprons (annually)		
	Inspect outlet aprons for erosion, repair as needed (annually)		
	Inspect, repair as needed, riprap aprons for dislodged/sparse coverage (annually)		
Additional Notes/Observations:			

Appendix 3

Subsurface Investigations



United States
Department of
Agriculture

NRCS

Natural
Resources
Conservation
Service

A product of the National
Cooperative Soil Survey,
a joint effort of the United
States Department of
Agriculture and other
Federal agencies, State
agencies including the
Agricultural Experiment
Stations, and local
participants

Custom Soil Resource Report for York County, Maine



Custom Soil Resource Report Soil Map



Map Scale: 1:1,170 if printed on A portrait (8.5" x 11") sheet.

0 15 30 60 90 Meters

0 50 100 200 300 Feet

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 19N WGS84

MAP LEGEND

- Area of Interest (AOI)
- Area of Interest (AOI)
- Soils**
- Soil Map Unit Polygons
- Soil Map Unit Lines
- Soil Map Unit Points
- Special Point Features**
- Blowout
- Borrow Pit
- Clay Spot
- Closed Depression
- Gravel Pit
- Gravelly Spot
- Landfill
- Lava Flow
- Marsh or swamp
- Mine or Quarry
- Miscellaneous Water
- Perennial Water
- Rock Outcrop
- Saline Spot
- Sandy Spot
- Severely Eroded Spot
- Sinkhole
- Slide or Slip
- Sodic Spot
- Spoil Area
- Stony Spot
- Very Stony Spot
- Wet Spot
- Other
- Special Line Features
- Water Features**
- Streams and Canals
- Transportation**
- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads
- Background**
- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
 Web Soil Survey URL:
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: York County, Maine
 Survey Area Data: Version 18, Sep 16, 2019

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Dec 31, 2009—Sep 9, 2017

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
LnC	Lyman loam, 8 to 15 percent slopes, rocky	0.6	14.6%
Ur	Urban land	3.7	85.4%
Totals for Area of Interest		4.3	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however,

Custom Soil Resource Report

onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

York County, Maine

LnC—Lyman loam, 8 to 15 percent slopes, rocky

Map Unit Setting

National map unit symbol: 2trq9
Elevation: 0 to 690 feet
Mean annual precipitation: 36 to 65 inches
Mean annual air temperature: 36 to 52 degrees F
Frost-free period: 60 to 160 days
Farmland classification: Not prime farmland

Map Unit Composition

Lyman, rocky, and similar soils: 86 percent
Minor components: 14 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Lyman, Rocky

Setting

Landform: Hills, mountains
Landform position (two-dimensional): Backslope, summit, shoulder
Landform position (three-dimensional): Mountaintop, mountainbase, mountainflank, crest, side slope
Down-slope shape: Convex
Across-slope shape: Convex
Parent material: Loamy supraglacial till derived from granite and gneiss and/or loamy supraglacial till derived from phyllite and/or loamy supraglacial till derived from mica schist

Typical profile

Oe - 0 to 1 inches: moderately decomposed plant material
A - 1 to 3 inches: loam
E - 3 to 5 inches: fine sandy loam
Bhs - 5 to 7 inches: loam
Bs1 - 7 to 11 inches: loam
Bs2 - 11 to 18 inches: channery loam
R - 18 to 28 inches: bedrock

Properties and qualities

Slope: 8 to 15 percent
Depth to restrictive feature: 11 to 24 inches to lithic bedrock
Natural drainage class: Somewhat excessively drained
Capacity of the most limiting layer to transmit water (Ksat): Very low to high (0.00 to 14.03 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water storage in profile: Low (about 3.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 3e
Hydrologic Soil Group: D
Hydric soil rating: No

Custom Soil Resource Report

Minor Components

Tunbridge, rocky

Percent of map unit: 6 percent

Landform: Hills, mountains

Landform position (two-dimensional): Backslope, summit, shoulder

Landform position (three-dimensional): Mountaintop, mountainbase, mountainflank, side slope, crest

Down-slope shape: Convex

Across-slope shape: Convex

Hydric soil rating: No

Skerry, rocky

Percent of map unit: 5 percent

Landform: Hills, mountains

Landform position (two-dimensional): Footslope, backslope

Landform position (three-dimensional): Mountaintop, mountainbase, mountainflank, crest, side slope

Microfeatures of landform position: Closed depressions, closed depressions, open depressions, open depressions

Down-slope shape: Concave

Across-slope shape: Concave

Hydric soil rating: No

Hermon, rocky

Percent of map unit: 2 percent

Landform: Mountains, hills

Landform position (two-dimensional): Summit, shoulder, backslope

Landform position (three-dimensional): Mountainflank, mountaintop, mountainbase, side slope, crest

Down-slope shape: Convex

Across-slope shape: Convex

Hydric soil rating: No

Brayton, rocky

Percent of map unit: 1 percent

Landform: Hills, mountains

Landform position (two-dimensional): Footslope, toeslope

Landform position (three-dimensional): Mountaintop, mountainbase, mountainflank, crest, side slope

Microfeatures of landform position: Closed depressions, closed depressions, open depressions, open depressions

Down-slope shape: Concave

Across-slope shape: Concave

Hydric soil rating: Yes

Ur—Urban land

Map Unit Setting

National map unit symbol: 9k6x

Custom Soil Resource Report

Elevation: 10 to 2,200 feet
Mean annual precipitation: 30 to 50 inches
Mean annual air temperature: 37 to 46 degrees F
Frost-free period: 70 to 160 days
Farmland classification: Not prime farmland

Map Unit Composition

Urban land: 90 percent
Minor components: 10 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Urban Land

Setting

Landform position (two-dimensional): Toeslope, footslope
Landform position (three-dimensional): Base slope, tread
Down-slope shape: Linear
Across-slope shape: Linear

Typical profile

H1 - 0 to 6 inches: variable

Properties and qualities

Slope: 0 to 8 percent
Natural drainage class: Moderately well drained
Depth to water table: About 24 to 72 inches
Available water storage in profile: Very low (about 0.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 8s
Hydric soil rating: No

Minor Components

Adams

Percent of map unit: 2 percent
Landform: Outwash plains
Landform position (two-dimensional): Footslope
Landform position (three-dimensional): Tread
Down-slope shape: Linear
Across-slope shape: Linear
Hydric soil rating: No

Scantic

Percent of map unit: 2 percent
Landform: Coastal plains
Landform position (two-dimensional): Toeslope
Landform position (three-dimensional): Dip
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: Yes

Buxton

Percent of map unit: 2 percent
Landform: Coastal plains
Landform position (two-dimensional): Footslope
Landform position (three-dimensional): Tread

Custom Soil Resource Report

Down-slope shape: Concave

Across-slope shape: Linear

Hydric soil rating: No

Sulfihemists

Percent of map unit: 2 percent

Landform: Salt marshes

Landform position (two-dimensional): Toeslope

Landform position (three-dimensional): Talf

Down-slope shape: Linear

Across-slope shape: Linear

Hydric soil rating: Yes

Croghan

Percent of map unit: 2 percent

Landform: Outwash plains

Landform position (two-dimensional): Toeslope

Landform position (three-dimensional): Tread

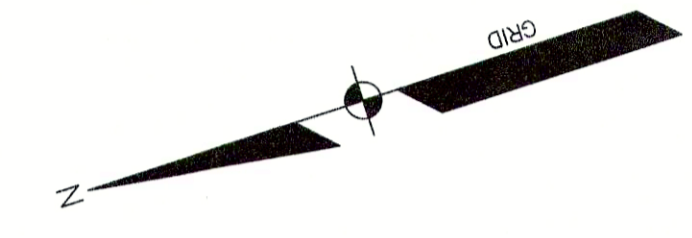
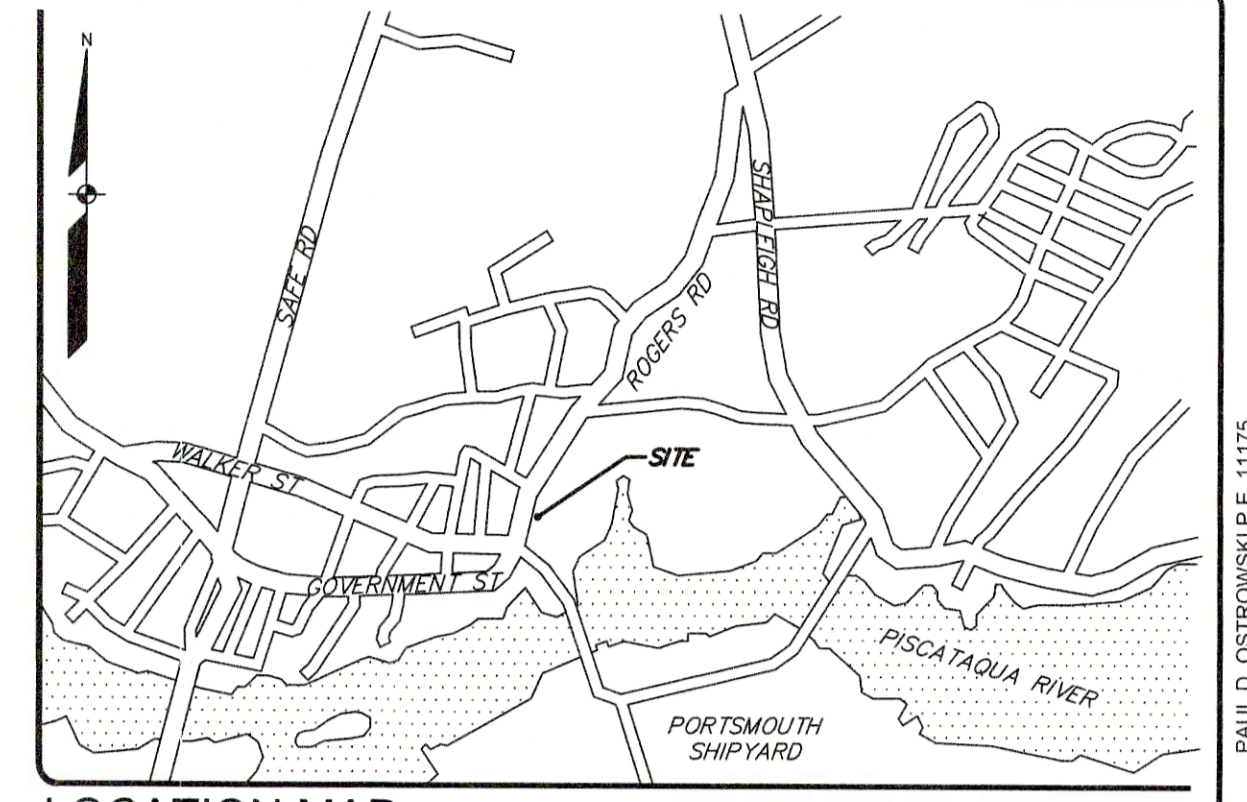
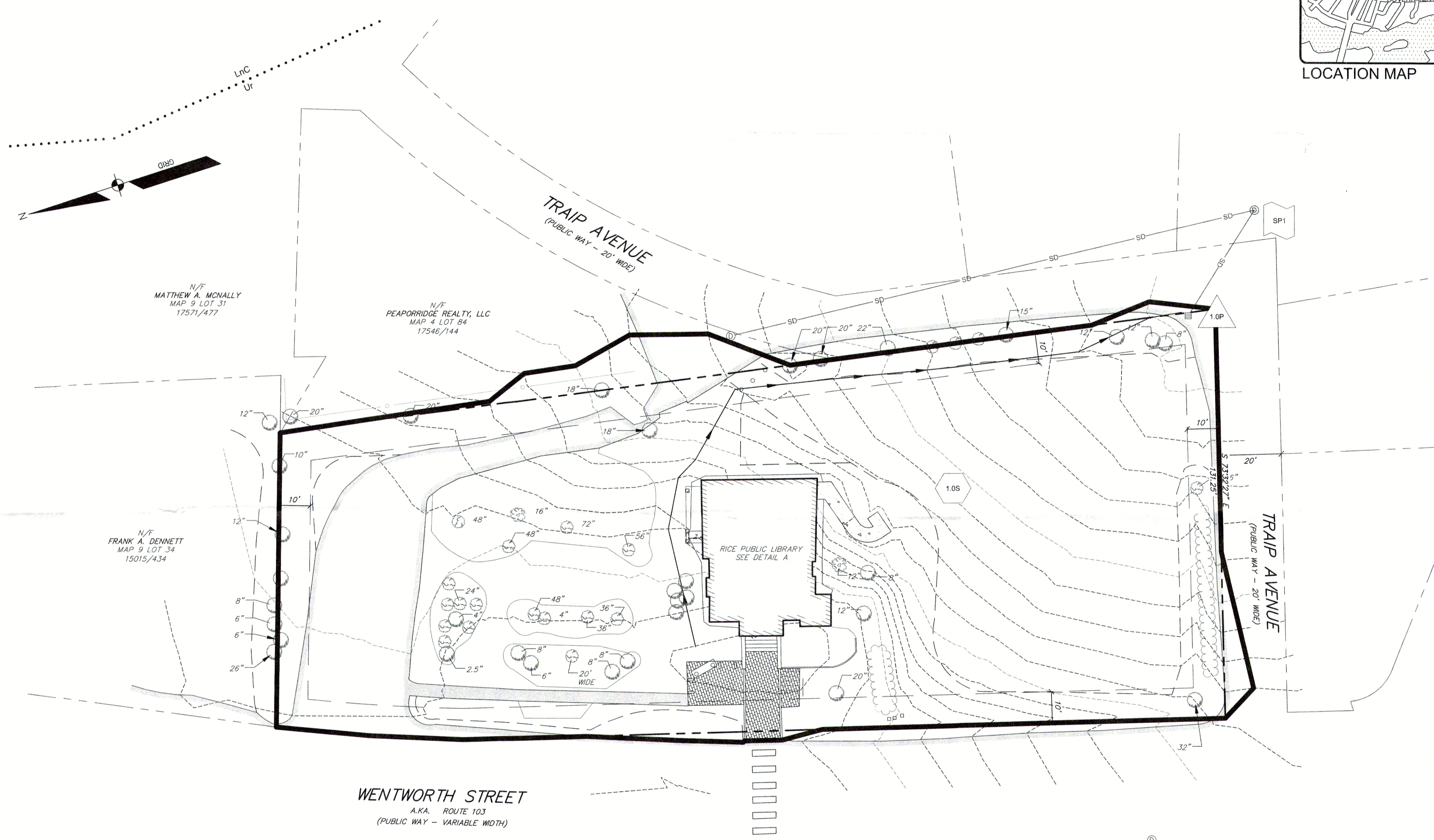
Down-slope shape: Linear

Across-slope shape: Linear

Hydric soil rating: No

Appendix 4

Stormwater Management Plans



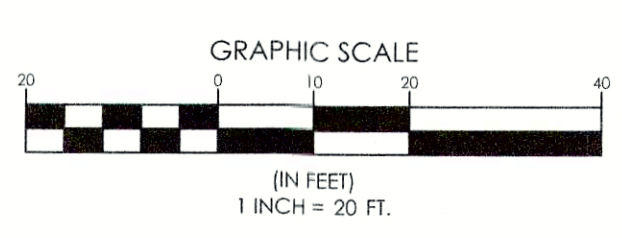
N/F
MATTHEW A. MCNALLY
MAP 9 LOT 31
17571/477

N/F
PEAPORRIDGE REALTY, LLC
MAP 4 LOT 84
17546/144

N/F
FRANK A. DENNETT
MAP 9 LOT 34
15015/434

RICE PUBLIC LIBRARY
SEE DETAIL A

WENTWORTH STREET
A.K.A. ROUTE 103
(PUBLIC WAY - VARIABLE WIDTH)



SOIL TYPE				
SYMBOL	SERIES	SCOPE	HSG	
Ur	URBAN LAND	0-8%	D	POORLY DRAINED

EXISTING CONDITIONS LEGEND

- WATERSHED BOUNDARY
- TIME OF CONCENTRATION
- REACH
- WATERSHED LABEL
- REACH
- STUDY POINT
- STORMWATER TREATMENT/DETENTION POND
- HSG #
- HSG #

NOT FOR CONSTRUCTION



REV#	BY	DATE	STATUS
E	ISGD	09/10/2020	ISSUED FOR FINAL SITE PLAN APPROVAL
D	ISGD	08/14/2020	ISSUED FOR DD
C	ISGD	07/23/2020	REVISED FOR VALUE ENGINEER
B	ISGD	03/12/2020	REVISED PER STAFF AND CLIENT COMMENTS
A	ISGD	02/06/2020	ISSUED FOR SITE PLAN REVIEW
	ISGD	08/27/2019	ISSUED FOR CLIENT USE

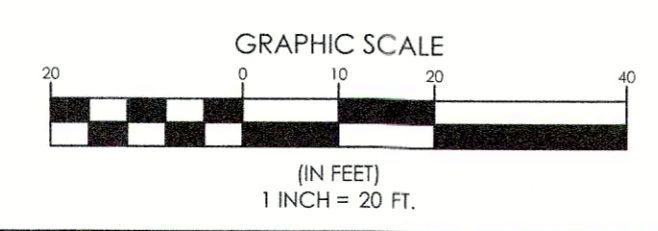
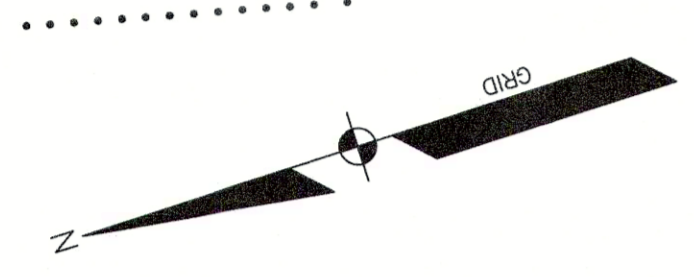
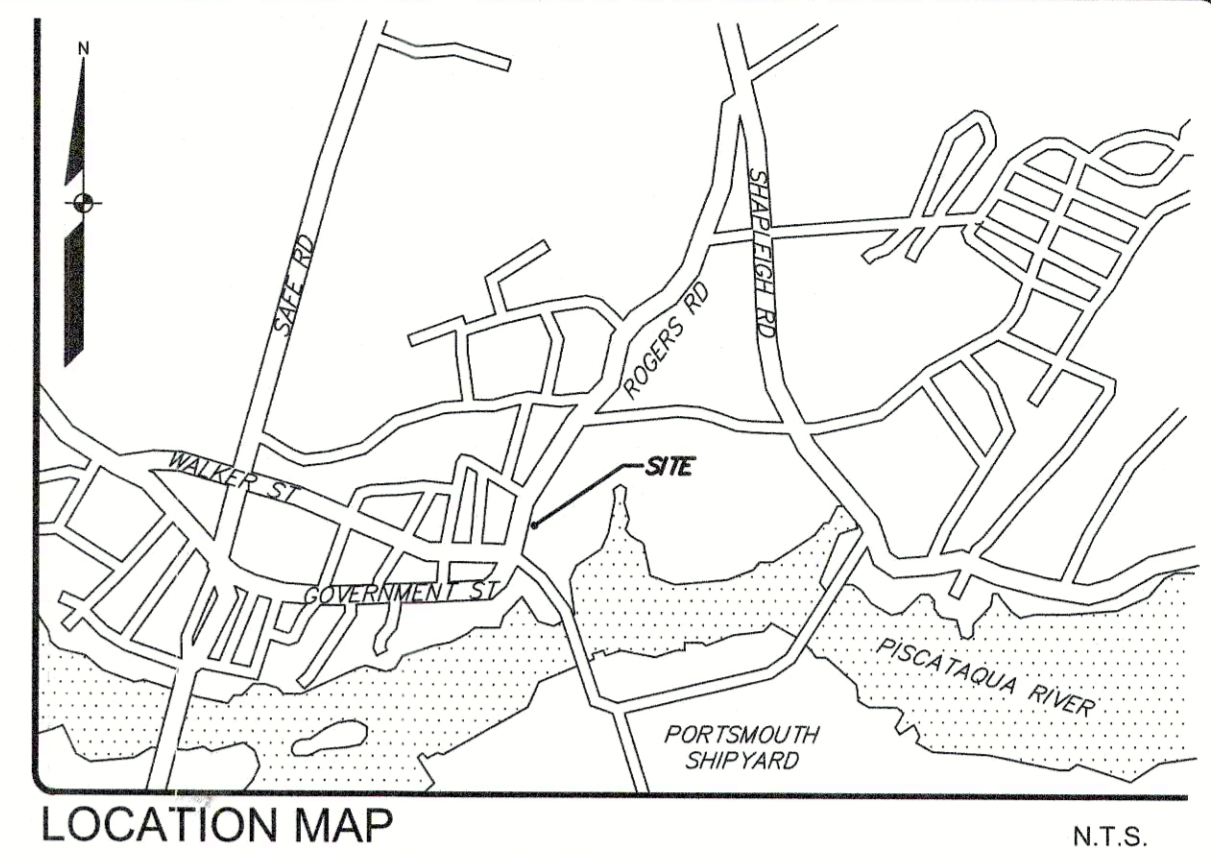
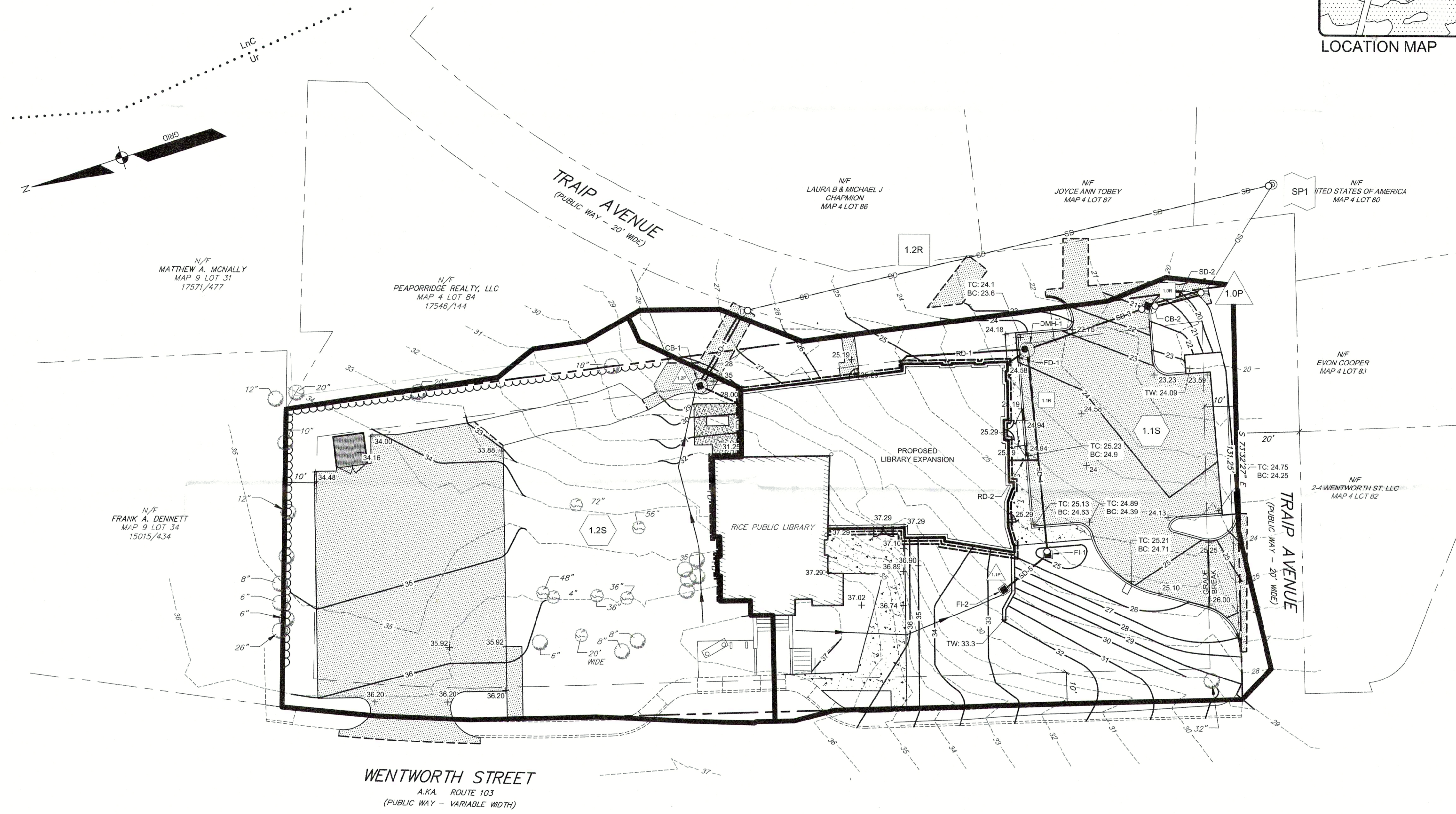
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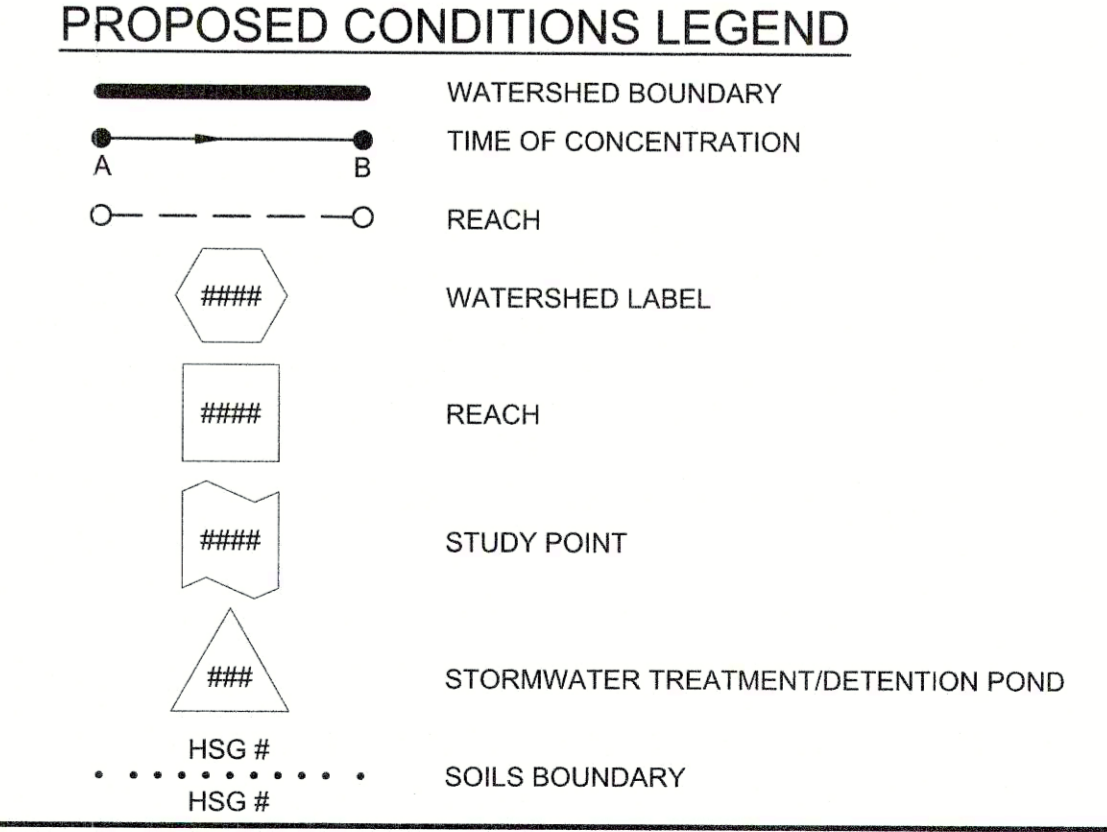
EXISTING CONDITIONS WATERSHED PLAN
OF:
RICE PUBLIC LIBRARY
8 WENTWORTH STREET
KITTERY, MAINE 03904
FOR:
LASSEL ARCHITECTS
P.O. BOX 370 MAIN STREET
SOUTH BERWICK, MAINE 03986
SCOTT SIMONS ARCHITECTS
10 ORK STREET
PORTLAND, MAINE 04101

DESIGNED	MKO
DRAWN	SRC
CHECKED	PDO
DATE	09/10/2020
SCALE	1" = 20'
PROJECT	18438

SHEET 1 OF 2



SOIL TYPE			
SYMBOL	SERIES	SCOPE	HSG
Ur	URBAN LAND	0-8%	D
			POORLY DRAINED



NOT FOR CONSTRUCTION

PAUL D. OSTROWSKI, P.E. 11175

STATE OF MAINE
 PROFESSIONAL ENGINEER
 No. 01175
 09/10/2020

REV.	BY:	DATE:	STATUS:
F	SGD	09/10/2020	ISSUED FOR FINAL SITE PLAN APPROVAL
E	SGD	08/14/2020	ISSUED FOR DD
D	SGD	07/23/2020	REVISED FOR VALUE ENGINEER
C	SGD	03/12/2020	REVISED PER STAFF AND CLIENT COMMENTS
B	SGD	02/06/2020	ISSUED FOR SITE PLAN REVIEW
A	SGD	08/27/2019	ISSUED FOR CLIENT USE

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 Tel. 207-200-2100

PROPOSED CONDITIONS WATERSHED PLAN

OF:
RICE PUBLIC LIBRARY
 8 WENTWORTH STREET
 KITTERY, MAINE 03904

FOR:
LASSEL ARCHITECTS
 P.O. BOX 370, 370 MAIN STREET
 SOUTH BERWICK, MAINE 03908

SCOTT SIMONS ARCHITECTS
 75 YORK STREET
 PORTLAND, MAINE 04101

DESIGNED	MKO
DRAWN	SRC
CHECKED	PDO
DATE	09/10/2020
SCALE	1" = 20'
PROJECT	18438

RICE PUBLIC LIBRARY

8 WENTWORTH STREET
KITTERY, MAINE 03904

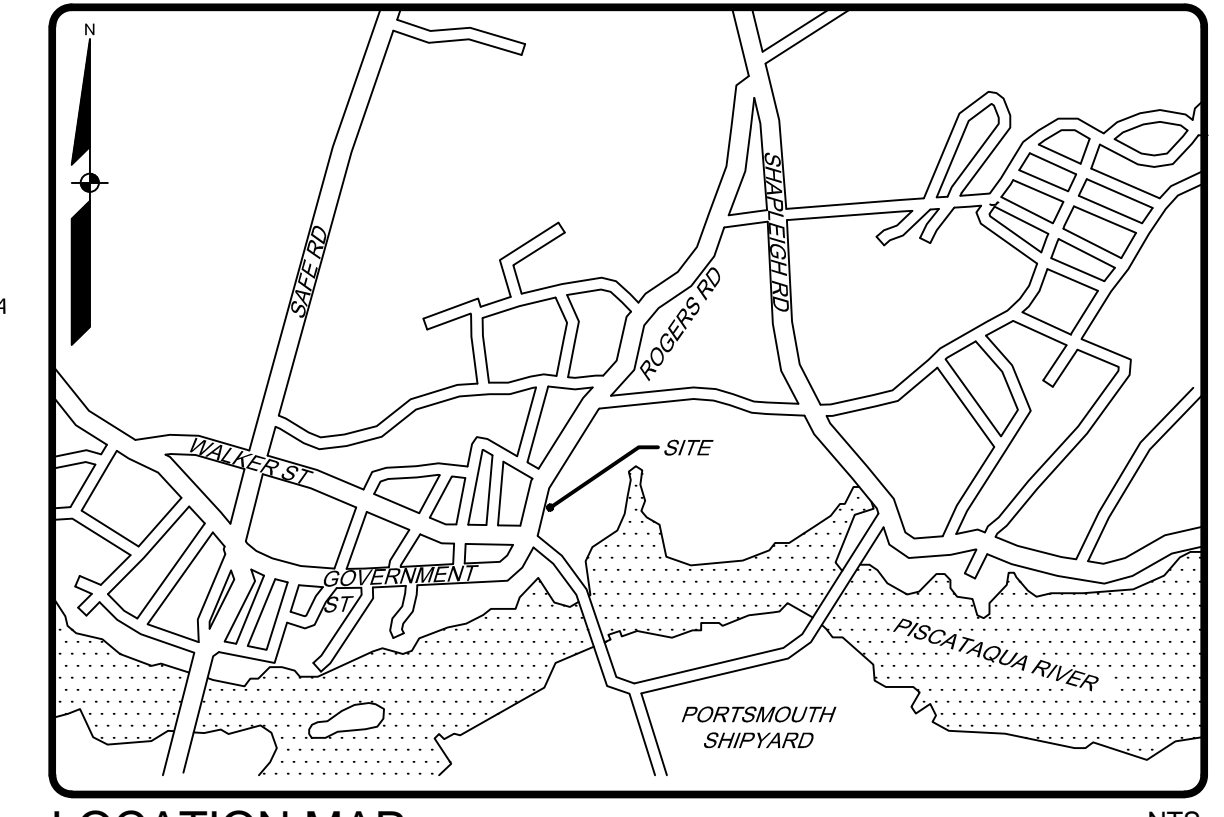
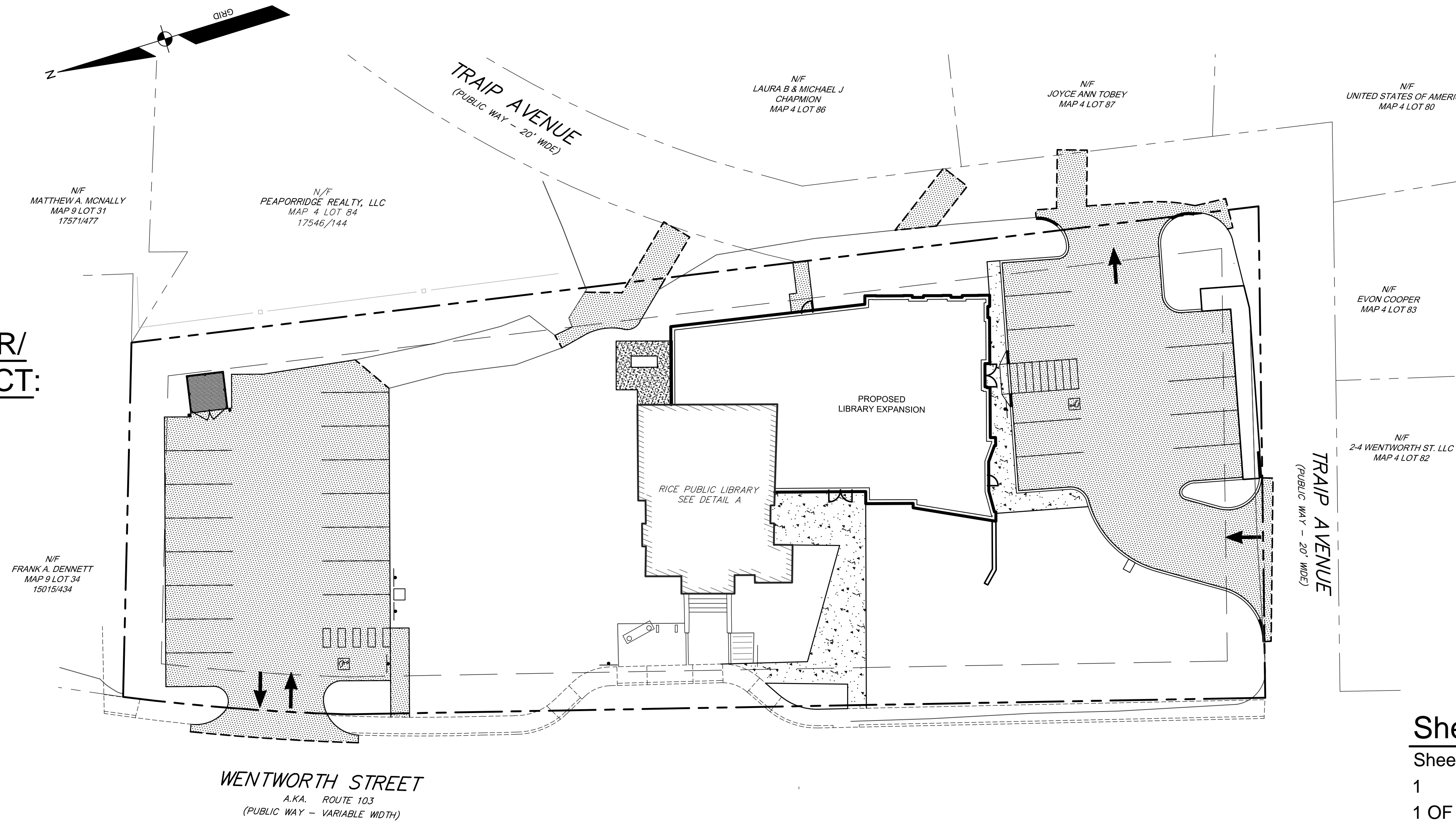
APPLICANT:

LASSEL ARCHITECT
P.O. BOX 370, 370 MAIN STREET
SOUTH BERWICK, MAINE 03908

SCOTT SIMONS ARCHITECTS
75 YORK STREET
PORTLAND, MAINE 04101

**ENGINEER/SURVEYOR/
LANDSCAPE ARCHITECT:**

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South Portland, ME 04106
Tel. 207-200-2100

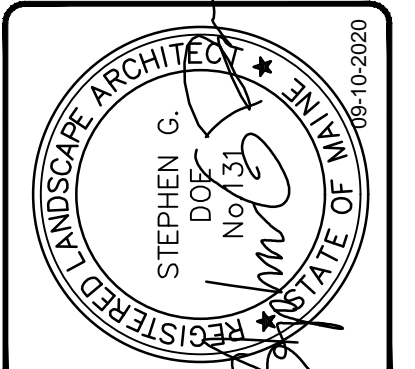


Sheet List Table

Sheet Number	Sheet Title
1	COVER SHEET
1 OF 1	EXISTING CONDITIONS
2	SITE PLAN
3	GRADING AND UTILITY PLAN
4	LANDSCAPE PLAN
5	EROSION CONTROL NOTES AND DETAILS
6	DETAILS
7	DETAILS
8	PHOTOMETRIC PLAN - BY OTHERS

SCALE: 1" = 20'

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F	SGD	09/10/2020	ISSUED FOR FINAL SITE PLAN APPROVAL
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D	SGD	07/23/2020	REVISED FOR VALUE ENGINEER
C	SGD	03/12/2020	REVISED PER STAFF AND CLIENT COMMENTS
B	SGD	02/06/2020	ISSUED FOR SITE PLAN REVIEW
A	SGD	08/27/2019	ISSUED FOR CLIENT USE

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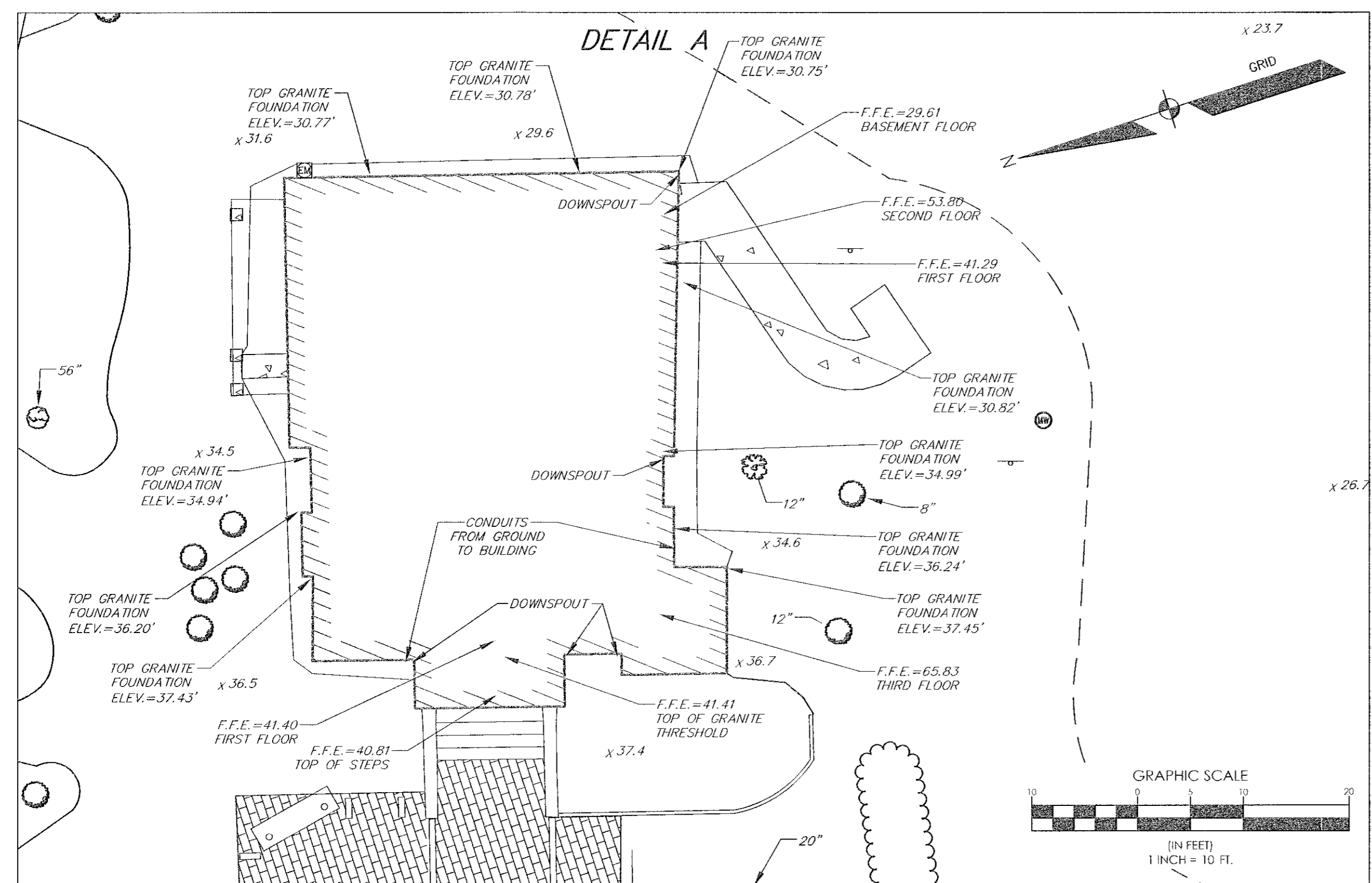
COVER SHEET
OF
RICE PUBLIC LIBRARY
8 WENTWORTH STREET
KITTERY, MAINE 03904
FOR:
LASSEL ARCHITECTS
P.O. BOX 370, 370 MAIN STREET
SOUTH BERWICK, MAINE 03908

SCOTT SIMONS ARCHITECTS
75 YORK STREET
PORTLAND, MAINE 04101

DESIGNED	SGD
DRAWN	SRC
CHECKED	SGD
DATE	09/10/2020
SCALE	1" = 20'
PROJECT	18438

SHEET 1 OF 8

MAP 4 LOT 88



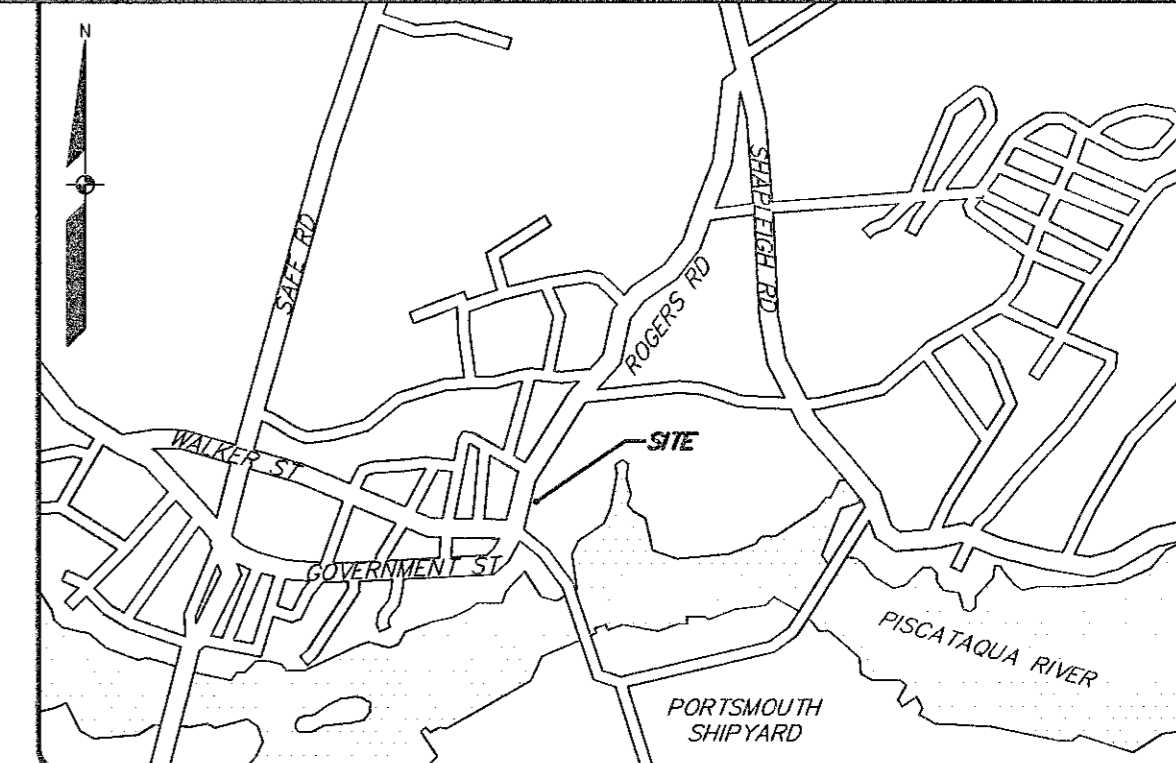
LEGEND

EXISTING

- PROPERTY LINE/R.O.W.
- - - ABUTTER LINE/R.O.W.
- MONUMENT
- IRON PIPE/ROD

BENCHMARK DESCRIPTION WITH ELEVATION

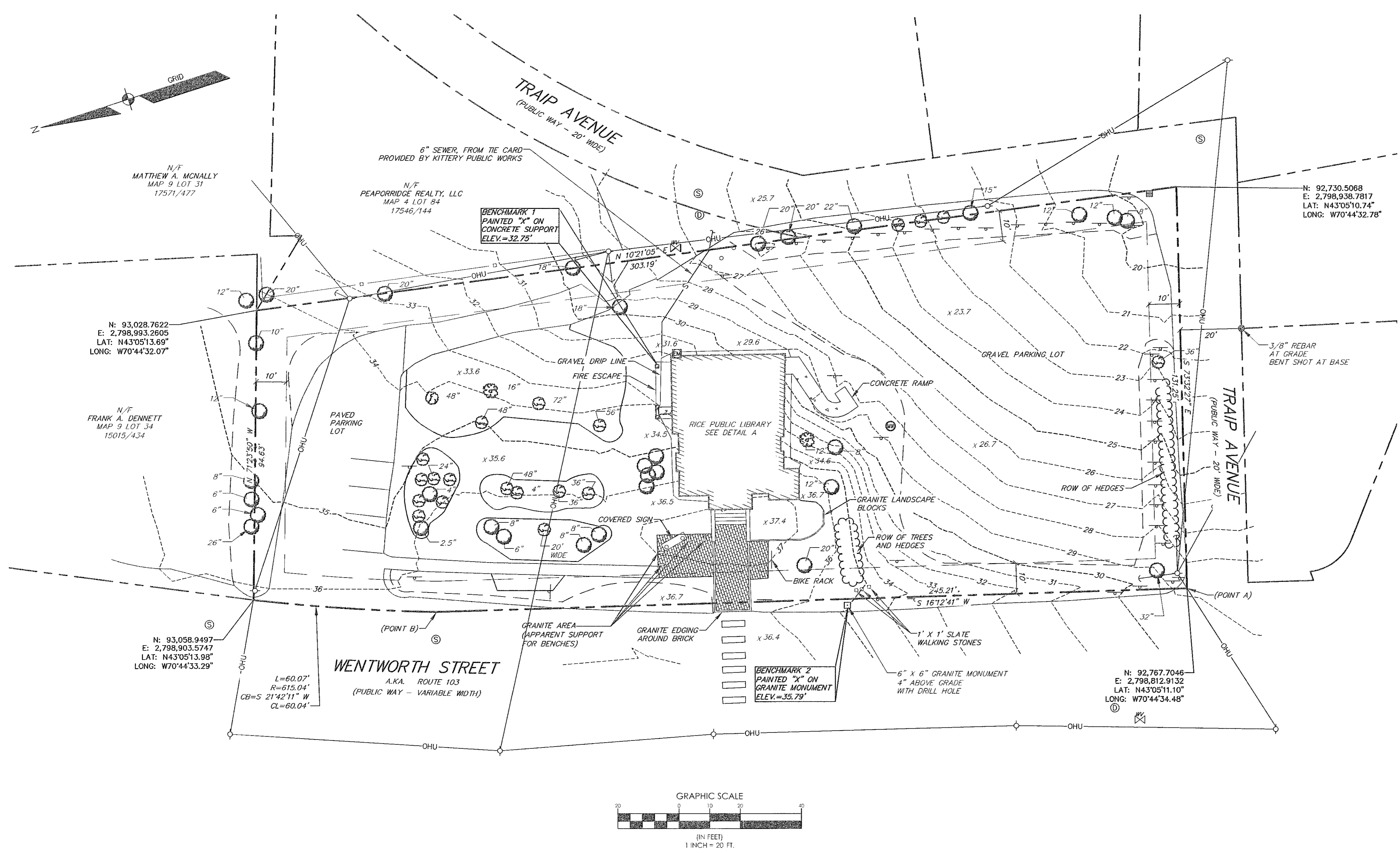
- BENCHMARK
- ▨ BUILDING
- ▨ DECK/STEPS/OVERHANG
- ▨ EDGE PAVEMENT
- ▨ EDGE CONCRETE
- ▨ EDGE GRAVEL
- ▨ LANDSCAPING
- ▨ TREELINE/HEDGE
- CONTOURS
- SPOT GRADE
- STOCKADE FENCE
- DECIDUOUS TREE
- CONIFEROUS TREE
- ORNAMENTAL SHRUB
- BOLLARD
- SIGN
- WATER GATE VALVE
- SANITARY MANHOLE
- DRAINAGE MANHOLE
- OHU OVERHEAD UTILITY
- ELECTRIC METER
- UTILITY POLE
- GUY WIRE
- MONITORING WELL
- SANITARY SEWER



LOCATION MAP N.T.S.

GENERAL NOTES:

- THE RECORD OWNER OF THE PARCEL IS RICE PUBLIC LIBRARY BY DEED DATED OCTOBER 6, 1975 AND RECORDED AT THE YORK COUNTY REGISTRY OF DEEDS (YCRD) IN BOOK 2099, PAGE 425.
- THE PROPERTY IS SHOWN AS LOT 88 ON THE TOWN OF KITTERY TAX MAP 4 AND IS LOCATED IN THE MIXED USE - KITTERY FORESIDE DISTRICT.
- SPACE AND BULK CRITERIA FOR THE MIXED USE - KITTERY FORESIDE DISTRICT ARE AS FOLLOWS:
 NET RESIDENTIAL DENSITY: 5,000 SQUARE FEET
 MINIMUM LOT SIZE: 5,000 SQUARE FEET
 MINIMUM STREET FRONTAGE: NONE
 MINIMUM FRONT YARD: 0/10 FEET+
 MINIMUM SIDE YARD: 10 FEET+
 MINIMUM REAR YARD: 10 FEET+
 MAXIMUM BUILDING HEIGHT: 40 FEET+
 MAXIMUM BUILDING COVERAGE: 60%
 * SEE ORDINANCE FOR MORE PARTICULAR INFORMATION.
- TOTAL AREA OF PARCEL IS APPROXIMATELY 34,947 SQUARE FEET OR 0.80 ACRES AS DEPICTED ON PLAN REFERENCE 6A.
- THE BOUNDARY AS DEPICTED HEREON IS BASED SOLELY ON PLAN REFERENCE 6A. TOPOGRAPHIC INFORMATION SHOWN HEREON IS BASED UPON A FIELD SURVEY PERFORMED BY SEBAGO TECHNICS, INC. IN JANUARY 17, 2019.
- PLAN REFERENCES:
 A. "STANDARD BOUNDARY SURVEY OF THE RICE PUBLIC LIBRARY LOT, WENTWORTH ST. AND TRAP AVENUE, KITTERY, MAINE" BY EASTERLY SURVEYING DATED JANUARY 18, 1991 AND RECORDED IN THE YCRD IN PLAN BOOK 201, PAGE 11
 B. "LOT SURVEY FOR THE KITTERY INVESTMENT GROUP IN KITTERY, MAINE" BY BRUCE L. POHOPEK DATED OCTOBER 25, 1989
- PLAN ORIENTATION IS GRID NORTH, MAINE STATE PLANE COORDINATE SYSTEM, WEST ZONE 1802-NAD83. ELEVATIONS DEPICTED HEREON ARE NAVD88, BASED ON DUAL FREQUENCY GPS OBSERVATIONS.
- UTILITY INFORMATION DEPICTED HEREON IS COMPILED USING PHYSICAL EVIDENCE LOCATED IN THE FIELD. UTILITIES DEPICTED HEREON MAY NOT NECESSARILY REPRESENT ALL EXISTING UTILITIES. CONTRACTORS AND/OR DESIGNERS NEED TO CONTACT DIG-SAFE SYSTEMS, INC. (1-888-DIG-SAFE) AND FIELD VERIFY EXISTING UTILITIES PRIOR TO CONSTRUCTION AND/OR EXCAVATION.
- THE LOCUS PROPERTY AS DEPICTED HEREON DOES NOT FALL WITHIN A SPECIAL FLOOD HAZARD AREA AS DELINEATED ON THE FLOOD INSURANCE RATE MAP FOR KITTERY, MAINE, YORK COUNTY, COMMUNITY-PANEL NUMBER 230171-0008-D, HAVING AN EFFECTIVE DATE OF JULY 3, 1986. THE LOCUS FALLS WITHIN AN AREA IDENTIFIED AS ZONE C, AREAS OF MINIMAL FLOODING.
- THE MONUMENTS MARKING THE BOUNDARY PER PLAN REFERENCE 6A WERE NOT FOUND. THE BEST EVIDENCE THAT WAS FIELD LOCATED INCLUDES THE GRANITE MONUMENT, 2.09 FEET OFFSET FROM THE BOUNDARY LINE PER PLAN REFERENCE 6A, FOUND ON WENTWORTH STREET AND THE REBAR FOUND ON TRAP AVENUE. PLAN REFERENCE 6A WAS PLACED BY HOLDING THE SCALED MEASUREMENTS FROM PLAN REFERENCE 6A'S SOUTHWESTERLY REBAR TO BE SET (POINT A) TO THE FOUND GRANITE MONUMENT AND THE NORTHERLY "ROW MONUMENT TO BE SET BY THE STATE OF MAINE D.O.T." (POINT B) TO THE FOUND GRANITE MONUMENT. USING THE SCALED MEASUREMENTS AS A BASE POINT THE PLAN WAS ROTATED TO THE 20 FOOT OFFSET FROM THE FOUND REBAR ON TRAP AVENUE.

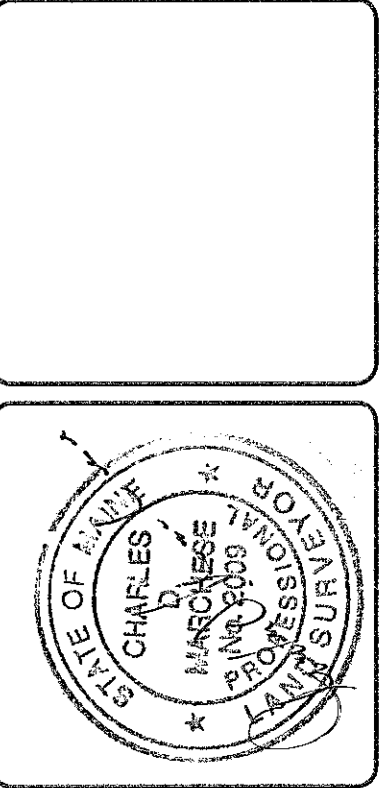


SURVEYOR'S STATEMENT

THIS SURVEY WAS PERFORMED UNDER MY DIRECT SUPERVISION AND TO THE BEST OF MY KNOWLEDGE AND BELIEF, IT WAS DONE IN ACCORDANCE WITH CHAPTER 90, PART 1 (PROFESSIONAL STANDARDS OF PRACTICE) AND PART 2 (TECHNICAL STANDARDS OF PRACTICE) OF THE MAINE BOARD OF LICENSURE FOR PROFESSIONAL LAND SURVEYORS.

Charles D. Marchese
 CHARLES D. MARCHESE, PLS 2009

FEBRUARY 5, 2020



REV.	DATE	STATUS
D	02-06-20	ISSUED FOR SITE PLAN REVIEW
C	08-27-19	ISSUED FOR CLIENT USE
B	02-20-19	UPDATED FOR CLIENT USE
A	01-10-19	ISSUED FOR CLIENT REVIEW

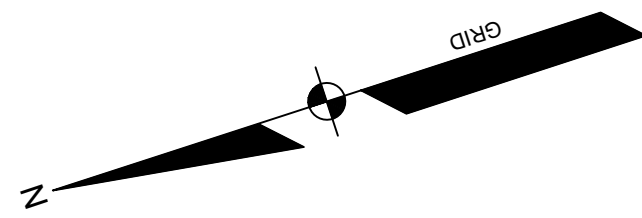
DRAWN: AJVS
 CHECKED: TSL/CDM

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 75 West Roberts Rd.
 South Portland, ME 04106
 Tel. 207-200-2100

EXISTING CONDITIONS SURVEY
 OF:
RICE PUBLIC LIBRARY
 8 WENTWORTH STREET
 KITTERY, MAINE 03904
 FOR:
LASSEL ARCHITECTS
 SCOTT SIMONS ARCHITECTS
 P.O. BOX 370, 370 MAIN STREET
 SOUTH BERWICK, MAINE 03908

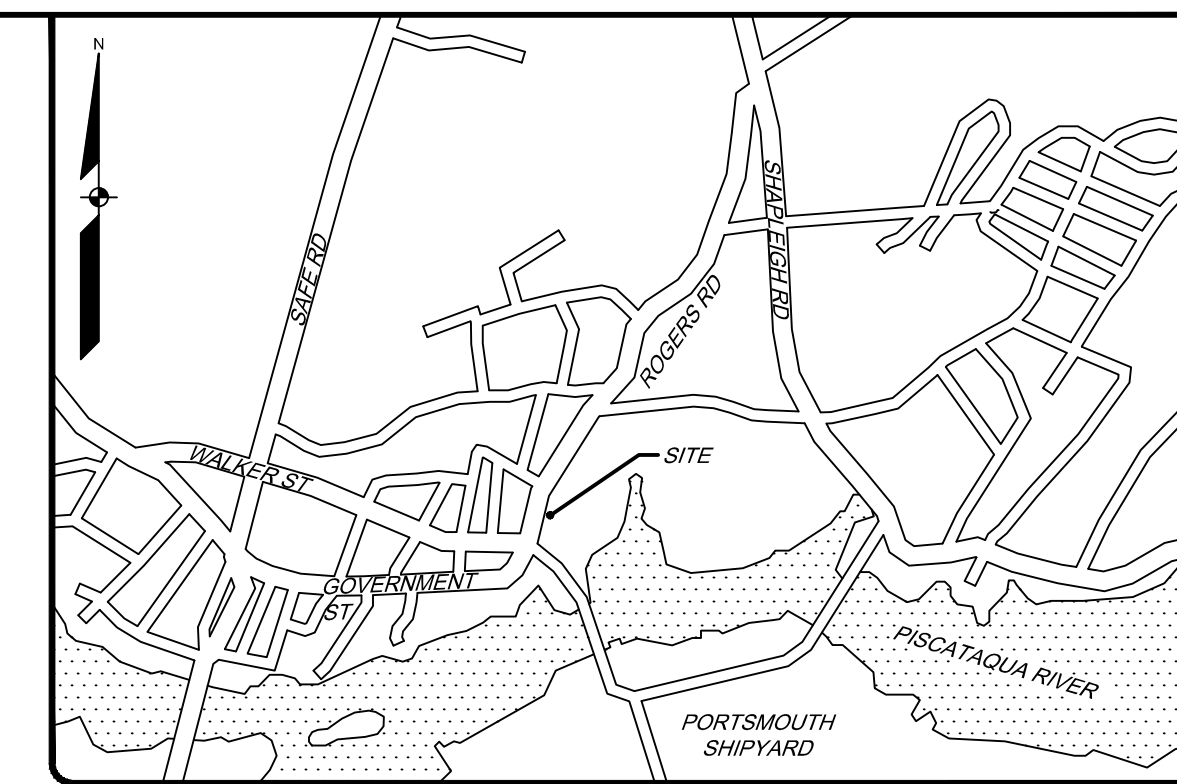
PROJECT NO.	SCALE
18438	1" = 20'

SHEET 1 OF 1



LEGEND

EXISTING	PROPOSED
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- - -	- - -
□	□
○	○
▭	▭
▬	▬
▬	▬
▬	▬
▬	▬
▬	▬
▬	▬
▬	▬
▬	▬
○	○
○	○
○	○
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LOCATION MAP N.T.S.

GENERAL NOTES:

- THE RECORD OWNER OF THE PARCEL IS THE INHABITANTS OF THE TOWN OF KITTEBY BY DEED DATED JULY 9, 2019 AND RECORDED AT THE YORK COUNTY REGISTRY OF DEEDS (YCRD) IN BOOK 17992, PAGE 618.
- THE PROPERTY IS SHOWN AS LOT 88 ON THE TOWN OF KITTEBY TAX MAP 4 AND IS LOCATED IN THE MIXED USE - KITTEBY FORESIDE DISTRICT.
- SPACE AND BULK CRITERIA FOR THE MIXED USE - KITTEBY FORESIDE DISTRICT ARE AS FOLLOWS:
 NET RESIDENTIAL DENSITY: 5,000 SQUARE FEET
 MINIMUM LOT SIZE: 5,000 SQUARE FEET
 MINIMUM STREET FRONTAGE: NONE
 MINIMUM FRONT YARD: 0/10 FEET*
 MINIMUM SIDE YARD: 10 FEET*
 MINIMUM REAR YARD: 10 FEET*
 MAXIMUM BUILDING HEIGHT: 40 FEET*
 MAXIMUM BUILDING COVERAGE: 60%
 *SEE ORDINANCE FOR MORE PARTICULAR INFORMATION.
- TOTAL AREA OF PARCEL IS APPROXIMATELY 34,947 SQUARE FEET OR 0.80 ACRES AS DEPICTED ON PLAN REFERENCE 6A.
- THE BOUNDARY AS DEPICTED HEREON IS BASED SOLELY ON PLAN REFERENCE 6A. TOPOGRAPHIC INFORMATION SHOWN HEREON IS BASED UPON A FIELD SURVEY PERFORMED BY SEBAGO TECHNICS, INC. IN JANUARY 17, 2019.
- PLAN REFERENCES:
 A. "STANDARD BOUNDARY SURVEY OF THE RICE PUBLIC LIBRARY LOT, WENTWORTH ST. AND TRAIPI AVE. KITTEBY, MAINE" BY EASTERLY SURVEYING DATED JANUARY 18, 1991 AND RECORDED IN THE YCRD IN PLAN BOOK 201, PAGE 11
 B. "LOT SURVEY FOR THE KITTEBY INVESTMENT GROUP IN KITTEBY, MAINE" BY BRUCE L. PPOHOPER DATED OCTOBER 25, 1989
- PLAN ORIENTATION IS GRID NORTH, MAINE STATE PLANE COORDINATE SYSTEM, WEST ZONE 1802-NAD83. ELEVATIONS DEPICTED HEREON ARE NAVD83, BASED ON DUAL FREQUENCY GPS OBSERVATIONS.
- UTILITY INFORMATION DEPICTED HEREON IS COMPILED USING PHYSICAL EVIDENCE LOCATED IN THE FIELD. UTILITIES DEPICTED HEREON MAY NOT NECESSARILY REPRESENT ALL EXISTING UTILITIES. CONTRACTORS AND/OR DESIGNERS NEED TO CONTACT DIG-SAFE SYSTEMS, INC. (1-888-DIG-SAFE) AND FIELD VERIFY EXISTING UTILITIES PRIOR TO CONSTRUCTION AND/OR EXCAVATION.
- THE LOCUS PROPERTY AS DEPICTED HEREON DOES NOT FALL WITHIN A SPECIAL FLOOD HAZARD AREA AS DELINEATED ON THE FLOOD INSURANCE RATE MAP FOR KITTEBY, MAINE, YORK COUNTY, COMMUNITY-PANEL NUMBER 230171-0008-D, HAVING AN EFFECTIVE DATE OF JULY 3, 1986. THE LOCUS FALLS WITHIN AN AREA IDENTIFIED AS ZONE G, AREAS OF MINIMAL FLOODING.
- THE MONUMENTS MARKING THE BOUNDARY PER PLAN REFERENCE 6A WERE NOT FOUND. THE BEST EVIDENCE THAT WAS FIELD LOCATED INCLUDES THE LIBRARY MONUMENT, 2.09 FEET OFFSET FROM THE BOUNDARY LINE PER PLAN REFERENCE 6A, FOUND ON WENTWORTH STREET AND THE REBAR FOUND ON TRAIPI AVENUE. PLAN REFERENCE 6A WAS PLACED BY HOLDING THE SCALED MEASUREMENTS FROM PLAN REFERENCE 6A'S SOUTHWESTERLY REBAR TO BE SET (POINT A) TO THE FOUND GRANITE MONUMENT AND THE NORTHERLY ROW MONUMENT TO BE SET BY THE STATE OF MAINE D.O.T. (POINT B) TO THE FOUND GRANITE MONUMENT. USING THE SCALED MEASUREMENTS AS A BASE POINT THE PLAN WAS ROTATED TO THE 20 FOOT OFFSET FROM THE FOUND REBAR ON TRAIPI AVENUE.

11. LOT COVERAGE CALCULATIONS:

	BUILDING	TOTAL NON-VEGETATED
EXISTING:	5.32%	44.58%
PROPOSED:	15.60%	35.28%

12. USE: LIBRARY
13. PARKING SUMMARY
- EXISTING: 35+ SPACES (GRAVEL LOT HAS UNDEFINED SPACES)
- REQUIRED:
- A. 1 SPACE / 350 SF OF PUBLIC SPACE (9,268 SF / 350) = 26 SPACES
 - B. 1 SPACE / 2 EMPLOYEES (6 EMPLOYEES / 2) = 3 SPACES
- PROPOSED: 27 SPACES + 2 SHORT-TERM SPACES OFF WENTWORTH STREET = 29 SPACES

14. BUILDING SUMMARY:

	EXISTING RICE LIBRARY	EXISTING TAYLOR LIBRARY (TO BE DISCONTINUED)	PROPOSED RICE LIBRARY
BASEMENT:	1629 SF	2480 SF	5394 SF
1ST FLOOR:	1629 SF	2480 SF	5363 SF
2ND FLOOR:	1629 SF	4960 SF	5363 SF
TOTAL:	4887 SF	4960 SF	16,120 SF

APPROVAL-TOWN OF KITTEBY PLANNING BOARD

DESIGNED	SGD
DRAWN	SRC
CHECKED	SGD
DATE	09/10/2020
SCALE	1" = 20'
PROJECT	18438

NOT FOR CONSTRUCTION

STEPHEN G. DOE R. L.A. 131 09-10-2020

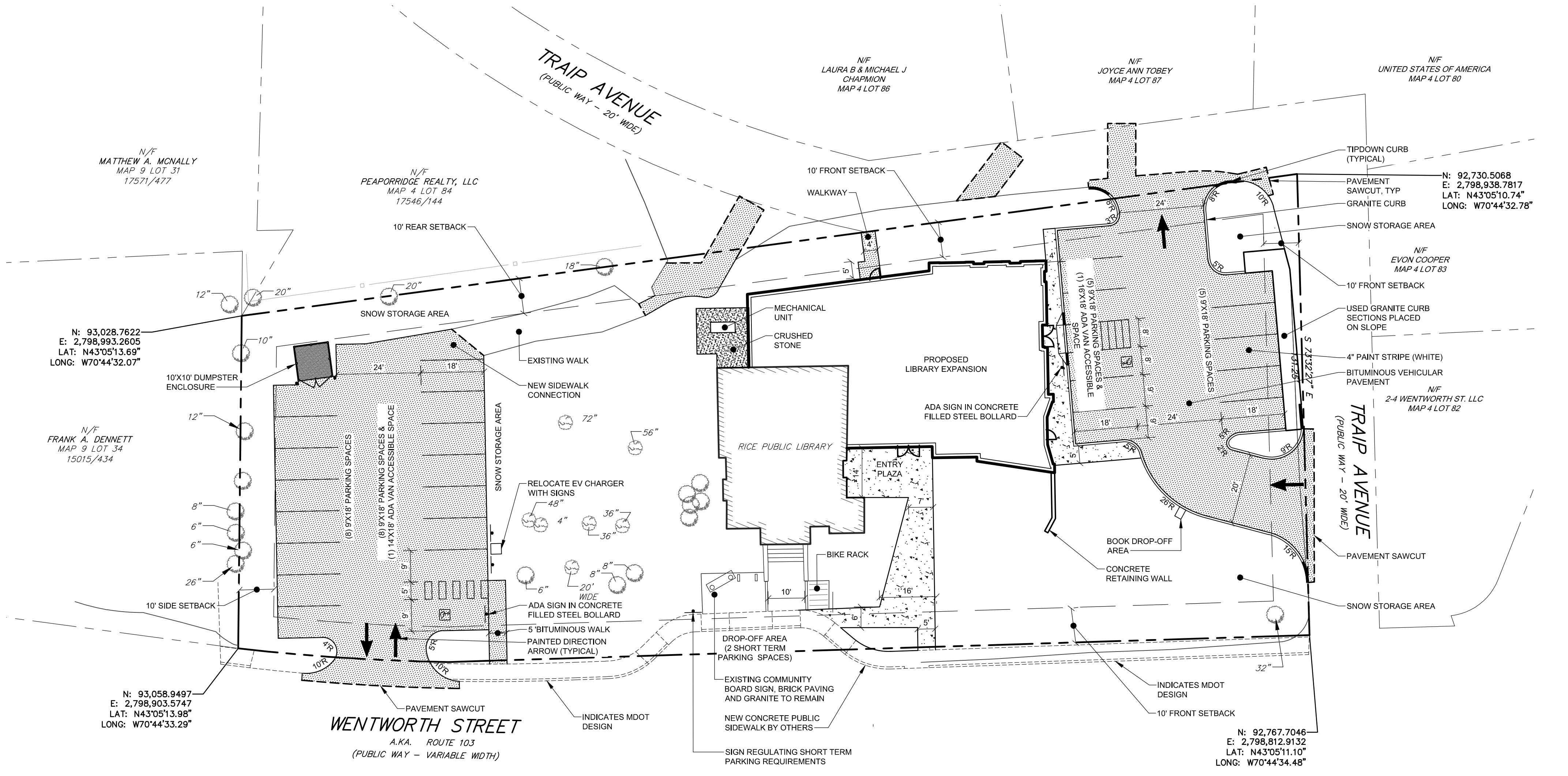
F	SGD	09/10/2020	ISSUED FOR FINAL SITE PLAN APPROVAL
E	SGD	08/14/2020	ISSUED FOR DD
D	SGD	07/23/2020	REVISED FOR VALUE ENGINEER
C	SGD	03/12/2020	REVISED PER STAFF AND CLIENT COMMENTS
B	SGD	02/06/2020	ISSUED FOR SITE PLAN REVIEW
A	SGD	08/27/2019	ISSUED FOR CLIENT USE
REV. BY:	DATE:	STATUS:	

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 WWW.SEBAGOTECHNICS.COM
 75 John Roberts Rd.
 Suite 4A
 South Portland, ME 04106
 Tel. 207-200-2100

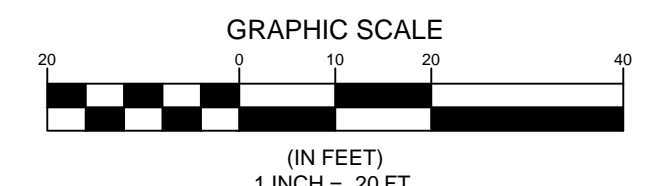
SITE PLAN OF RICE PUBLIC LIBRARY
 8 WENTWORTH STREET
 KITTEBY, MAINE 03904
 FOR: LASSAL ARCHITECTS
 P.O. BOX 370, 370 MAIN STREET
 SOUTH BERWICK, MAINE 03908

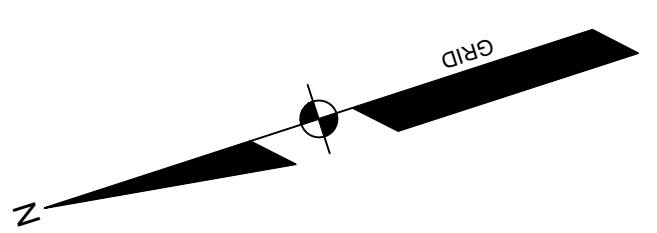
SCOTT SIMONS ARCHITECTS
 75 YORK STREET
 PORTLAND, MAINE 04101



GENERAL NOTES: (CONT.)

- WAIVERS:
 SEC. 16.8.4.13 A. SIDEWALKS ALONG TRAIPI AVENUE WAS WAIVED BY PLANNING BOARD AUGUST 27, 2020.





STORM DRAIN STRUCTURE DATA

STRUCTURE	RIM	INV. IN	INV. OUT:	DIAM.
CB-1	28.00		24.30 (SD-1)	48"
CB-2	21.44	17.55 (SD-3)	17.45 (SD-2)	48"
DMH-1	23.80	20.20 (SD-4) 20.20 (RD-1) 20.20 (FD-1)	20.10 (SD-3)	48"
FI-1	24.50	21.18 (SD-5)	21.08 (SD-4)	24"x24"
FI-2	32.39		21.39 (SD-5)	24"x24"

STORM DRAIN PIPE DATA

NAME	SIZE	LENGTH	SLOPE
FD-1	6"	5'	1.93%
RD-1	6"	36'	2.00%
RD-2	6"	9'	0.56%
SD-1	12"	25'	1.28%
SD-2	12"	16'	6.04%
SD-3	10"	37'	6.18%
SD-4	10"	62'	1.35%
SD-5	10"	16'	1.34%

NOT FOR
CONSTRUCTION

STEPHEN G. DOE R.L.A. 131

ISSUED FOR FINAL SITE PLAN APPROVAL

REV.	BY:	DATE:	STATUS:
F	SGD	08/10/2020	ISSUED FOR FINAL SITE PLAN APPROVAL
E	SGD	08/14/2020	ISSUED FOR DD
D	SGD	07/23/2020	REVISED FOR VALUE ENGINEER
C	SGD	03/12/2020	REVISED PER STAFF AND CLIENT COMMENTS
B	SGD	02/06/2020	ISSUED FOR SITE PLAN REVIEW
A	SGD	08/27/2019	ISSUED FOR CLIENT USE

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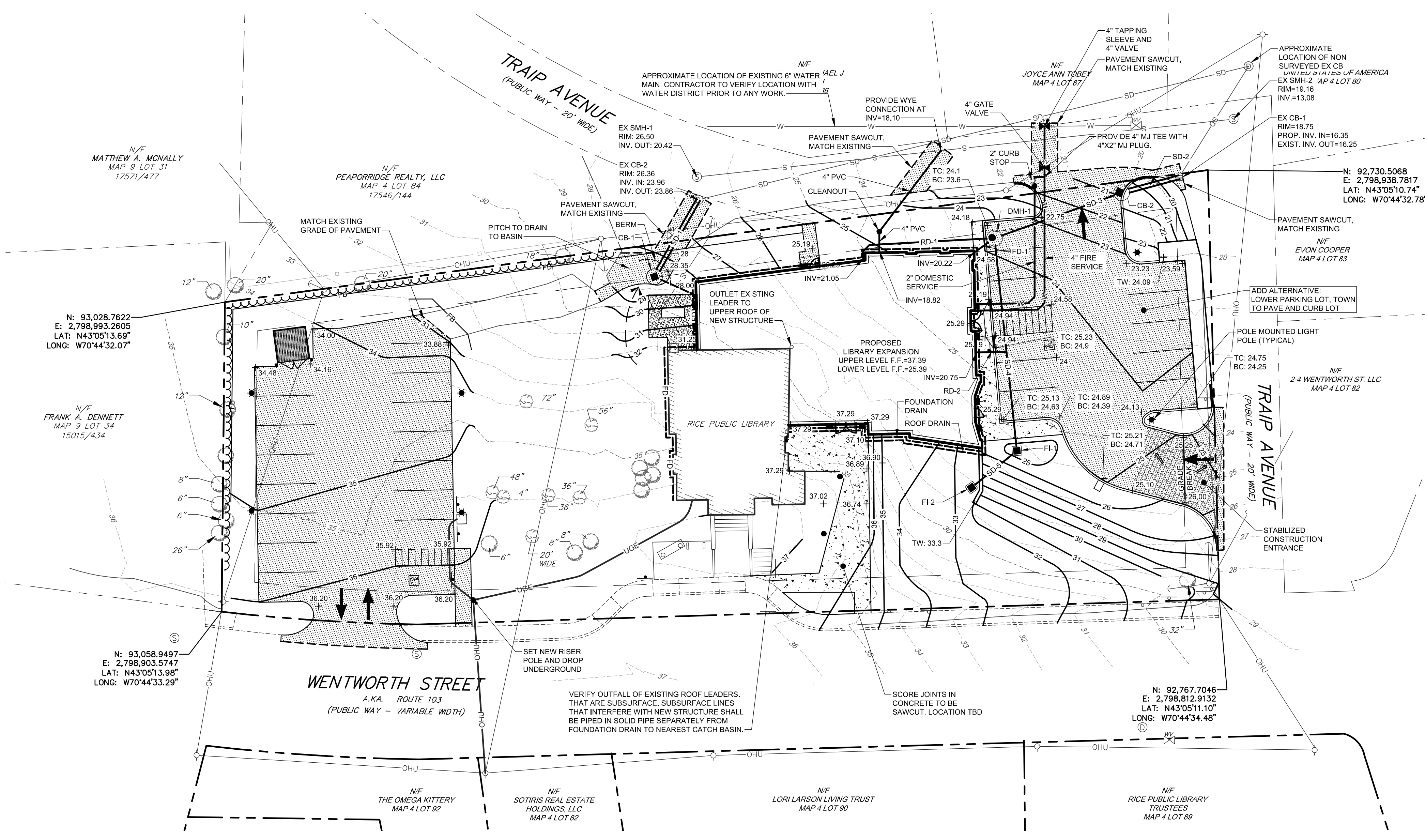
GRADING AND UTILITY PLAN
OF
RICE PUBLIC LIBRARY
8 WENTWORTH STREET
KITTEERY, MAINE 03904

FOR:
LASSEL ARCHITECTS
P.O. BOX 370, 370 MAIN STREET
SOUTH BERTWICK, MAINE 03908

SCOTT SIMONS ARCHITECTS
75 YORK STREET
PORTLAND, MAINE 04101

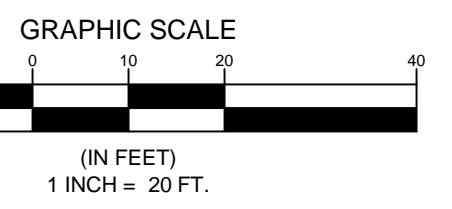
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DRAWN	SRC
CHECKED	SGD
DATE	09/10/2020
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PROJECT	18438

SHEET 3 OF 8

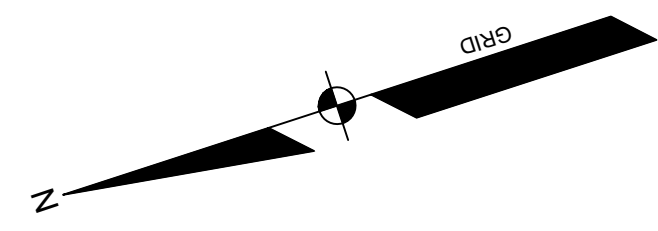


LEGEND

EXISTING	PROPOSED
(Symbol)	PROPERTY LINE/O.W.
(Symbol)	ABUTTER LINE/R.O.W.
(Symbol)	MONUMENT
(Symbol)	IRON PIPE/ROD
(Symbol)	BUILDING
(Symbol)	DECK/STEPS/ OVERHANG
(Symbol)	EDGE PAVEMENT
(Symbol)	EDGE CONCRETE
(Symbol)	PAVEMENT SAWCUT
(Symbol)	CURB LINE
(Symbol)	LANDSCAPING
(Symbol)	TREELINE/HEDGE
(Symbol)	CONTOURS
(Symbol)	SPOT GRADE
(Symbol)	STOCKADE FENCE
(Symbol)	DECIDUOUS TREE
(Symbol)	CONIFEROUS TREE
(Symbol)	ORNAMENTAL SHRUB
(Symbol)	BOLLARD
(Symbol)	SIGN
(Symbol)	WATER
(Symbol)	WATER GATE VALVE
(Symbol)	SANITARY MANHOLE
(Symbol)	SANITARY SEWER
(Symbol)	DRAINAGE MANHOLE
(Symbol)	STORM DRAIN
(Symbol)	UNDER DRAIN
(Symbol)	OVERHEAD UTILITY
(Symbol)	UNDERGROUND UTILITY
(Symbol)	ELECTRIC METER
(Symbol)	UTILITY POLE
(Symbol)	POLE LIGHT
(Symbol)	BOLLARD LIGHT
(Symbol)	BUILDING MOUNTED LIGHT
(Symbol)	GUY WIRE
(Symbol)	MONITORING WELL



18438.dwg Tab:GU

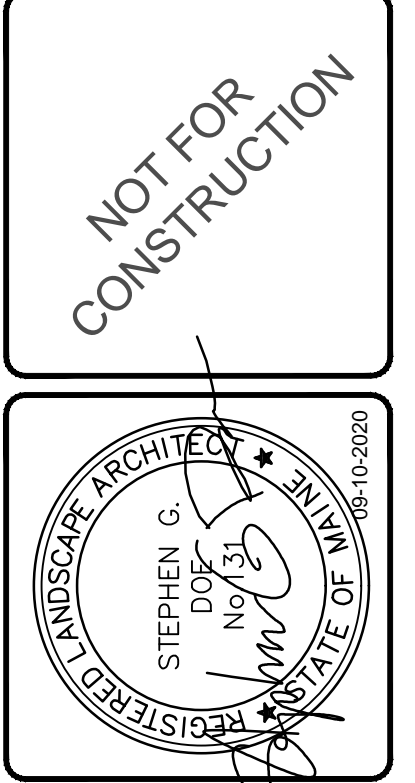


PLANT SCHEDULE

KEY BOTANICAL NAME	COMMON NAME	SIZE
HST HYDRANGEA SERRATA 'TINY TUFF STUFF'	TINY TUFF STUFF HYDRANGEA	#3 CONT.
RYK RHODODENDRON 'YAKUSHIMANUM 'KEN JANECK'	KEN JANECK RHODODENDRON	24'-30"
HSG HOSTA 'STAINED GLASS'	STAINED GLASS HOSTA	#1 CONT.
WF WALDSTEINIA FRAGARIODES	BARREN STRAWBERRY	#1 CONT.
FMR FOTHERGILLA GARDENII	DWARF BOTTLEBRUSH BUSH	#3 CONT.
MC MALLUS CAMELOT	CAMELOT CRABAPPLE	2' CAL
TP THUJA PLICATA 'SPRING GROVE'	SPRING GROVE ARBORVITAE	5-6' HGT
HS HYDRANGEA MACROPHYLLA 'SEASIDE SERANADE BAR HARBOR'	SEASIDE SERANADE BAR HARBOR HYDRANGEA	#3 CONT.

LEGEND

EXISTING	PROPOSED
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□	□
○	○
▨	▨
▬	▬
▬	▬
▬	▬
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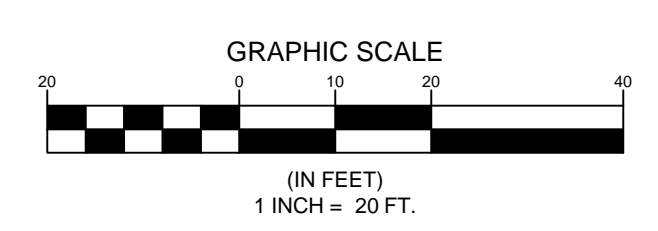
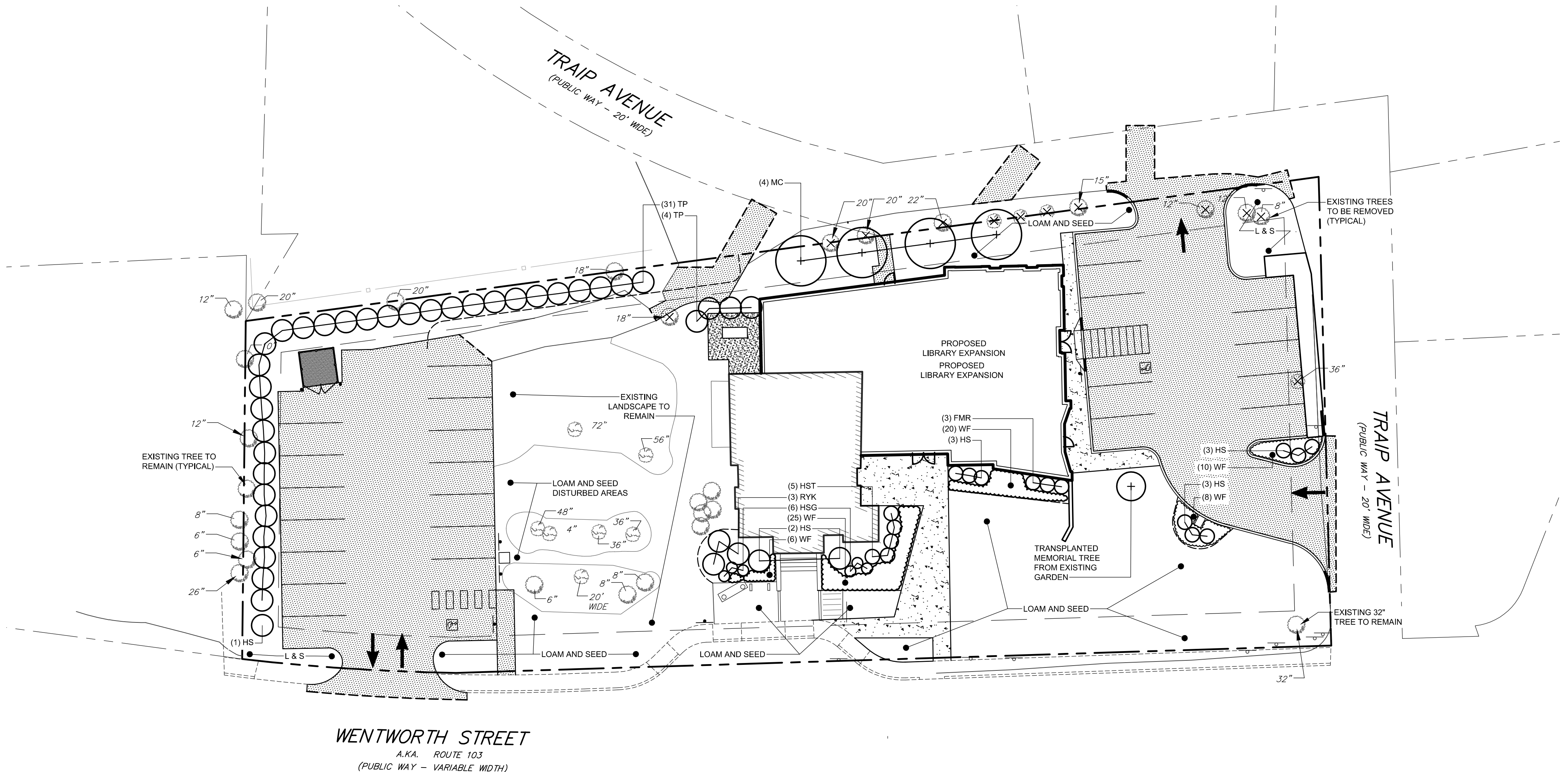
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LANDSCAPE PLAN
OF:
RICE PUBLIC LIBRARY
8 WENTWORTH STREET
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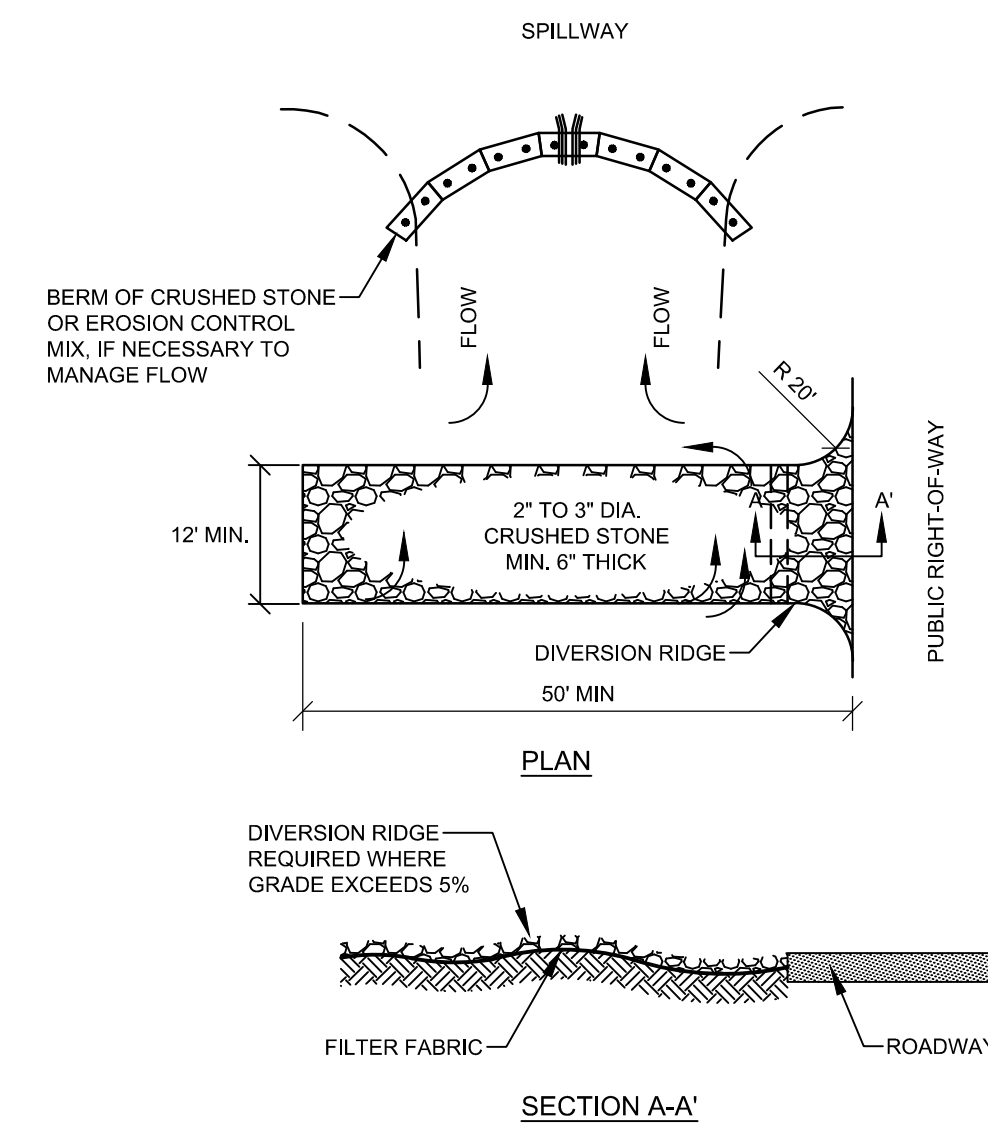
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DRAWN	SRC
CHECKED	SGD
DATE	09/10/2020
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SHEET 4 OF 8

MAP 4 LOT 88



18438.dwg, TAB Landscape

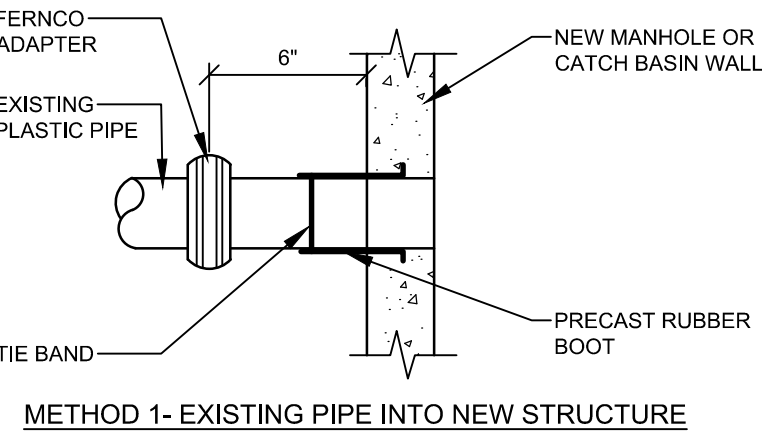


- NOTES:**
- STONE SIZE- AASHTO DESIGNATION M43, SIZE NO. 2 (2 1/2" TO 1 1/2"), USE CRUSHED STONE.
 - LENGTH- AS SHOWN ON PLANS, MIN. 50 FEET.
 - THICKNESS- NOT LESS THAN EIGHT (8) INCHES.
 - WIDTH- NOT LESS THAN FULL WIDTH OF ALL POINT OF INGRESS OR EGRESS.
 - MAINTENANCE- THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHT-OF-WAY. THIS MAY REQUIRE PERIODIC TOP DRESSING WITH ADDITIONAL STONE AS CONDITIONS DEMAND AND REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT. ALL SEDIMENT SPILLED, DROPPED, WASHED OR TRACKED ONTO PUBLIC RIGHT-OF-WAY MUST BE REMOVED IMMEDIATELY.

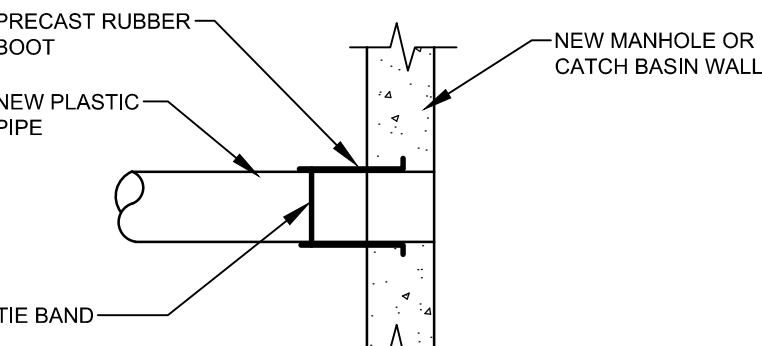
STABILIZED CONSTRUCTION ENTRANCE
NOT TO SCALE

- CONSTRUCTION SPECIFICATIONS:**
- THE ENTRANCE/EXIT PAD SHOULD HAVE A LENGTH OF 50 FEET OR MORE AND A 12 FOOT MINIMUM WIDTH (OR AS APPROPRIATE TO CONTAIN THE WHEEL BASE OF CONSTRUCTION VEHICLES PLUS 3 FEET ON EITHER SIDE).
 - THE PAD SHOULD BE 8 INCHES OR MORE THICK WITH ANGULAR AGGREGATE (2-3 INCH DIAMETER), APPROPRIATE RECLAIMED CONCRETE MATERIAL MAY BE USED.
 - THE AGGREGATE SHOULD BE PLACED OVER A GEOTEXTILE FILTER TO PREVENT THE STONES FROM PUSHING INTO THE NATIVE SOIL.
 - AT THE BOTTOM OF SLOPES, A DIVERSION RIDGE SHOULD BE PROVIDED TO INTERCEPT RUNOFF. BERMS MAY BE NECESSARY TO DIVERT WATER AROUND ANY EXPOSED SOIL, AND RUNOFF SHOULD BE DIRECTED TO A SEDIMENT TRAP.
 - THE WHEELS OF CONSTRUCTION EQUIPMENT MAY BE WASHED PRIOR TO EXITING THE SITE. WASHING SHOULD BE PERFORMED IN AN AREA THAT DRAINS TO A SEDIMENT TRAP OR BASIN.
 - THE PAD SHOULD BE INSPECTED WEEKLY, AND BEFORE AND AFTER EACH STORM. THE PAD MAY HAVE TO BE REPLACED IF THE VOIDS BECOME FILLED WITH SEDIMENT.
 - ALL SEDIMENT SPILLED, DROPPED, WASHED OR TRACKED ONTO PUBLIC RIGHT-OF-WAY MUST BE REMOVED IMMEDIATELY.

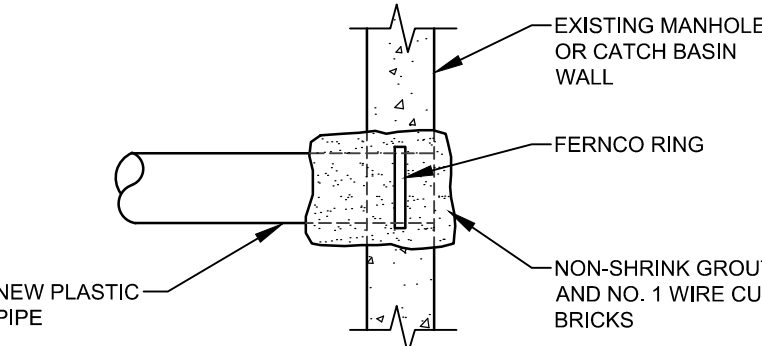
A PAD OF COURSE AGGREGATE AT THE CONSTRUCTION ENTRANCE/EXIT WILL REDUCE THE TRACKING OF SOIL FROM CONSTRUCTION TRAFFIC ONTO A PUBLIC STREET. SEDIMENTS FROM THE TIRE TREADS ARE KNOCKED LOOSE BY THE ANGULAR STONES AND ARE TRAPPED IN THE VOIDS BETWEEN THE STONES.



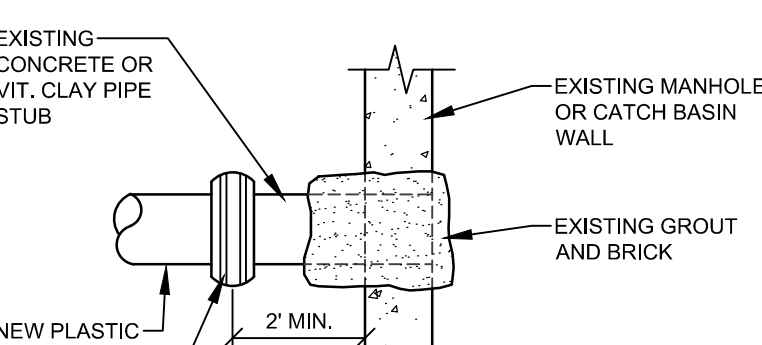
METHOD 1- EXISTING PIPE INTO NEW STRUCTURE



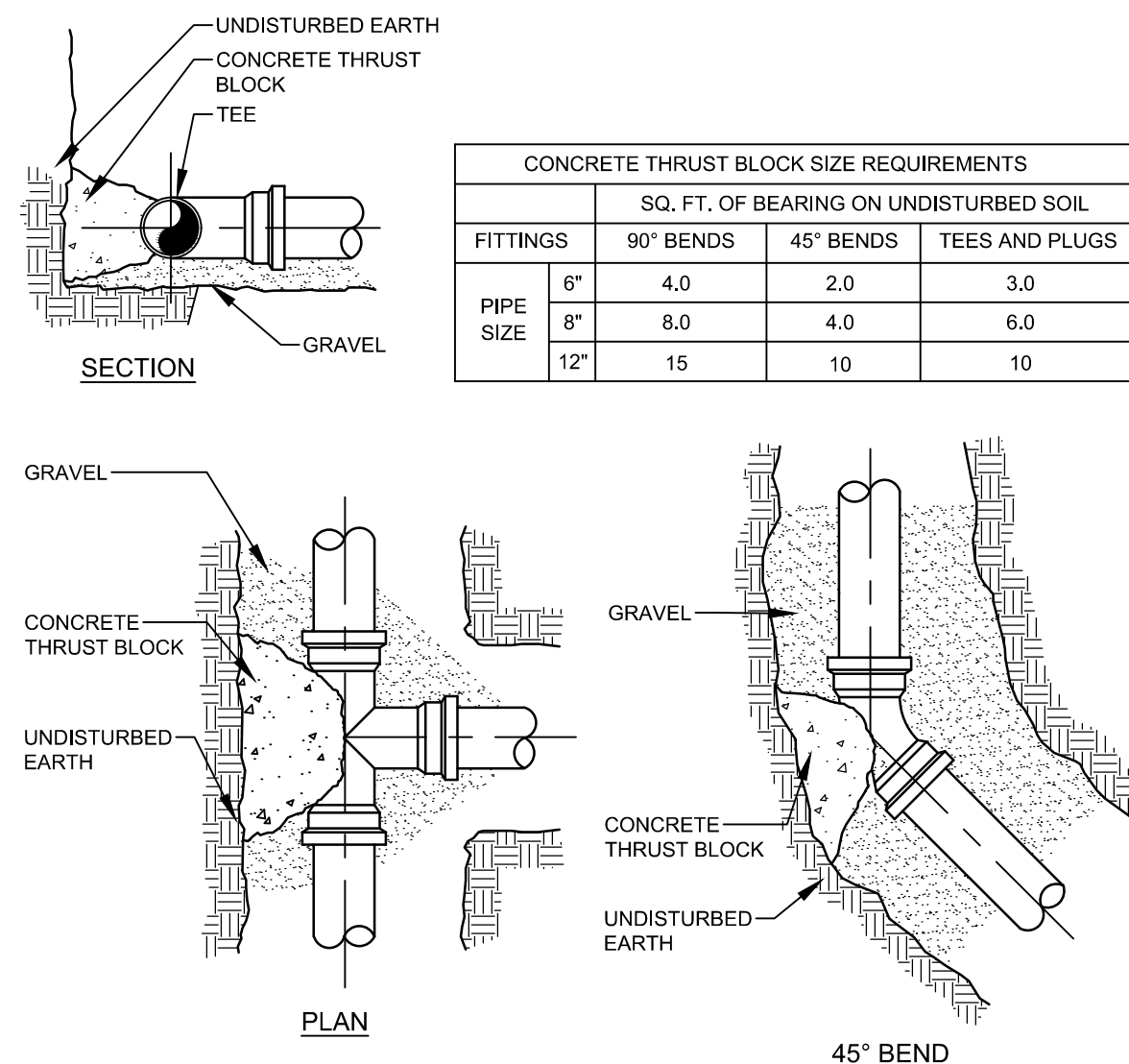
METHOD 2- NEW CONSTRUCTION



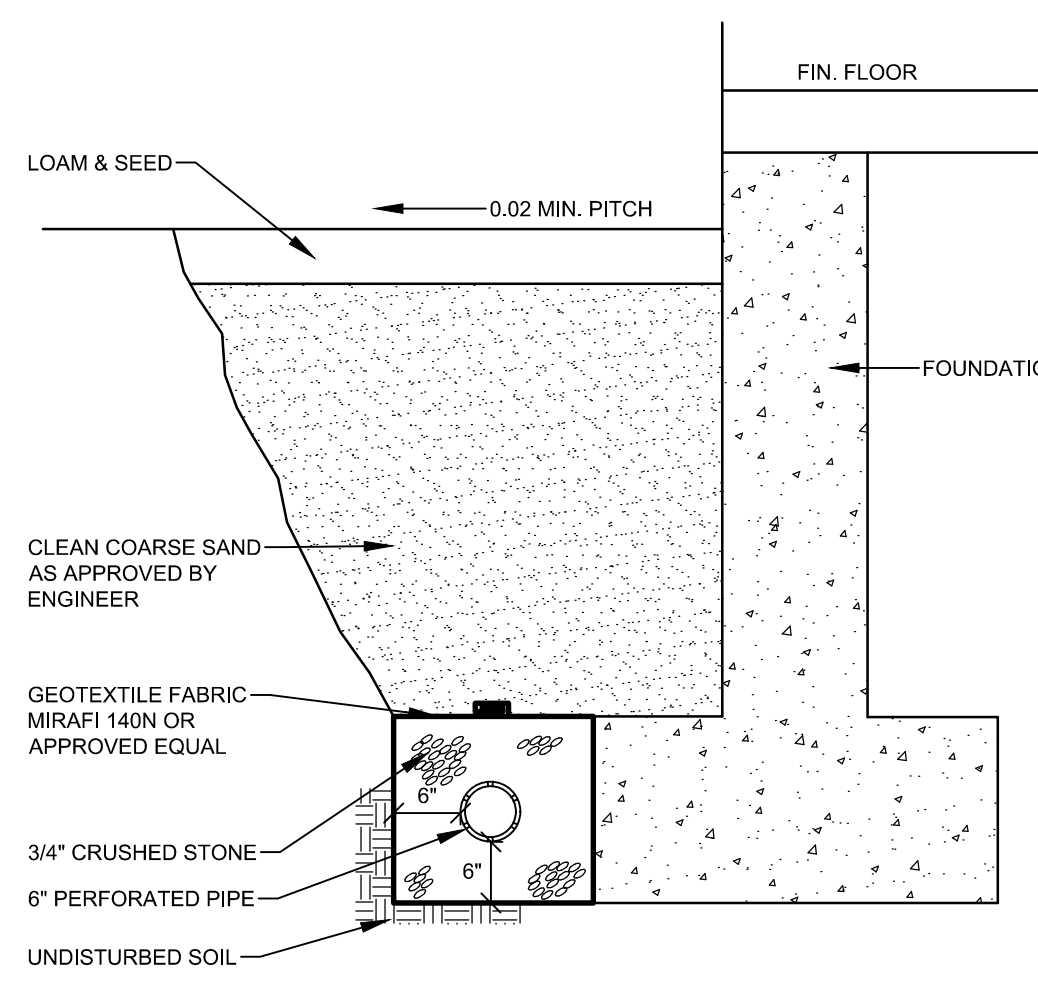
METHOD 3- NEW PIPE INTO EXISTING STRUCTURE



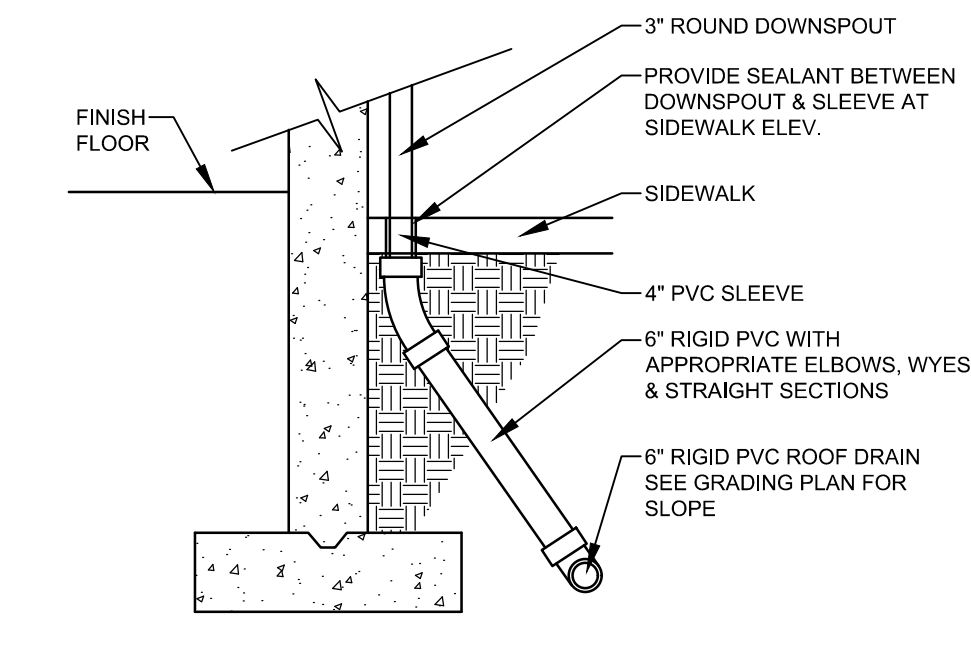
METHOD 4- NEW PIPE INTO EXISTING STUB



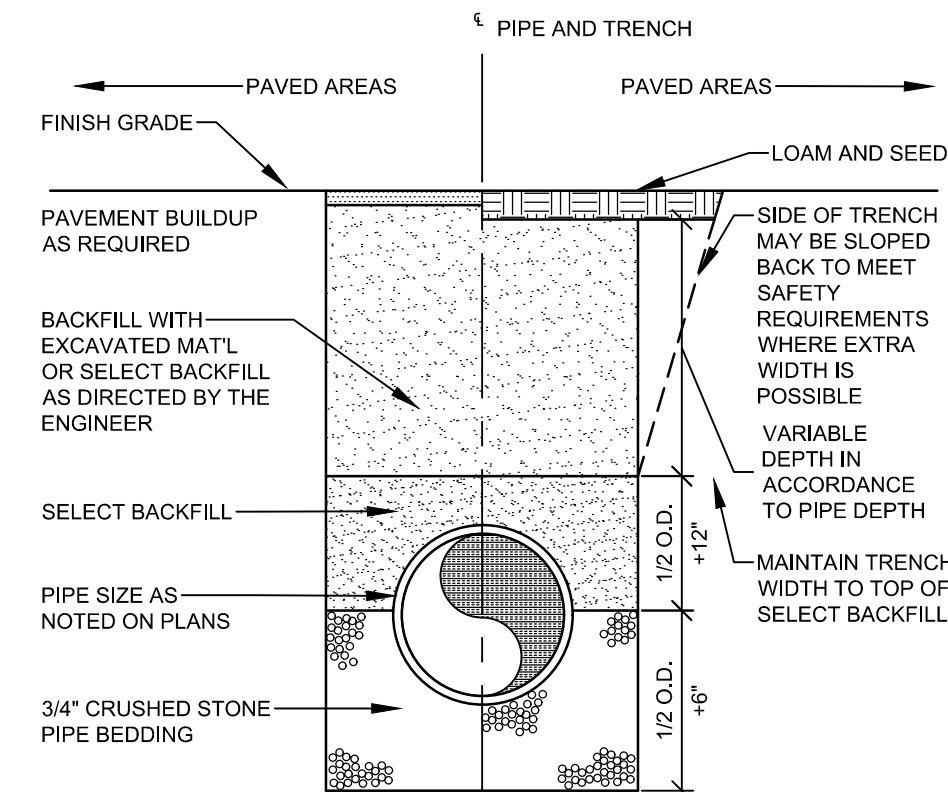
TEE & BEND DETAIL
NOT TO SCALE



FOUNDATION DRAIN SECTION
NOT TO SCALE



ROOF DRAIN CONNECTOR
NOT TO SCALE

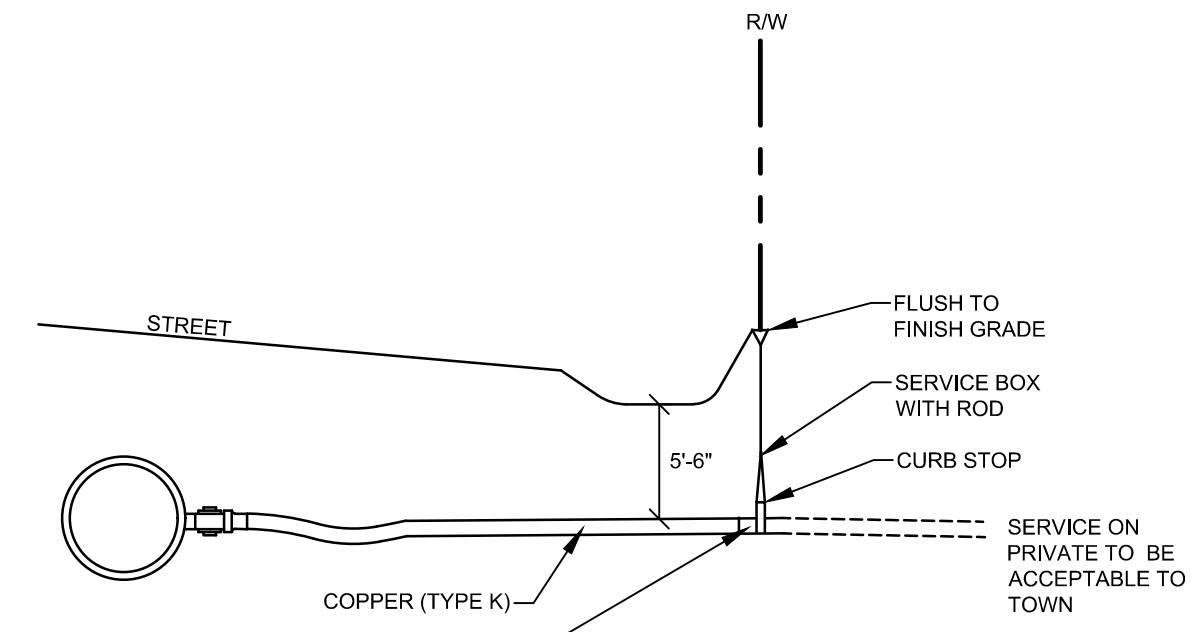


TRENCH BACKFILL SCHEDULE

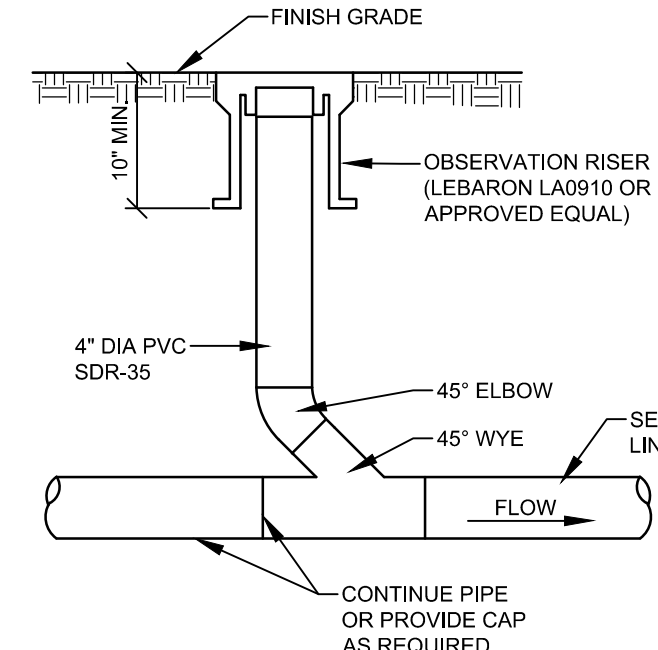
PIPE TYPE	PIPE BEDDING MATERIAL	SELECT BACKFILL
CORRUGATED METAL	MDOT 703.22 TYPE B UD BACKFILL	MDOT 703.22 TYPE B UD BACKFILL
DUCTILE IRON	MDOT 703.22 TYPE C UD BACKFILL	MDOT 703.22 TYPE C UD BACKFILL
REINFORCED CONCRETE	MDOT 703.22 TYPE C UD BACKFILL	MDOT 703.22 TYPE C UD BACKFILL
PVC-SDR 35 HDPE	MDOT 703.22 TYPE C UD BACKFILL	MDOT 703.22 TYPE C UD BACKFILL
PERFORATED PVC-SDR 35 HDPE	MDOT 703.22 TYPE C UD BACKFILL	MDOT 703.22 TYPE C UD BACKFILL

NOTE: ALL BRACING AND SHEETING SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL MEET ALL STATE AND O.S.H.A. SAFETY STANDARDS.

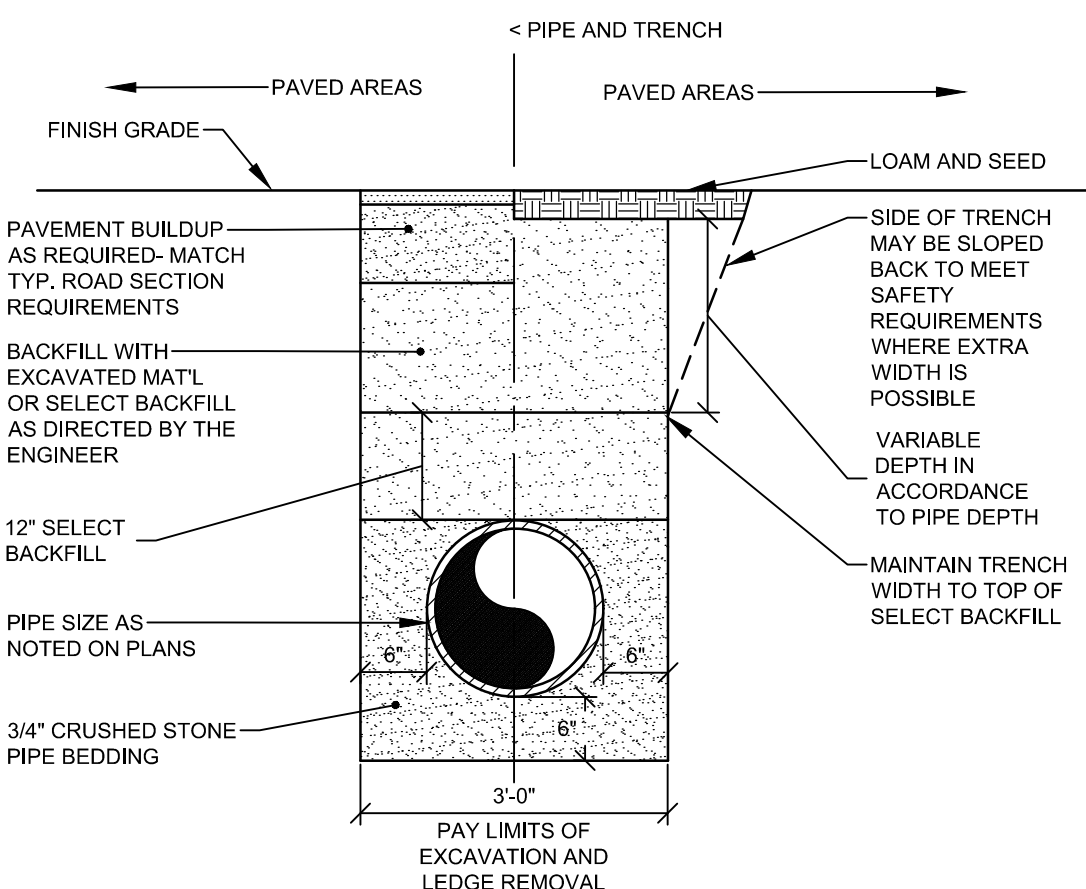
TRENCH SECTION
NOT TO SCALE



WATER SERVICE CONNECTION
NOT TO SCALE



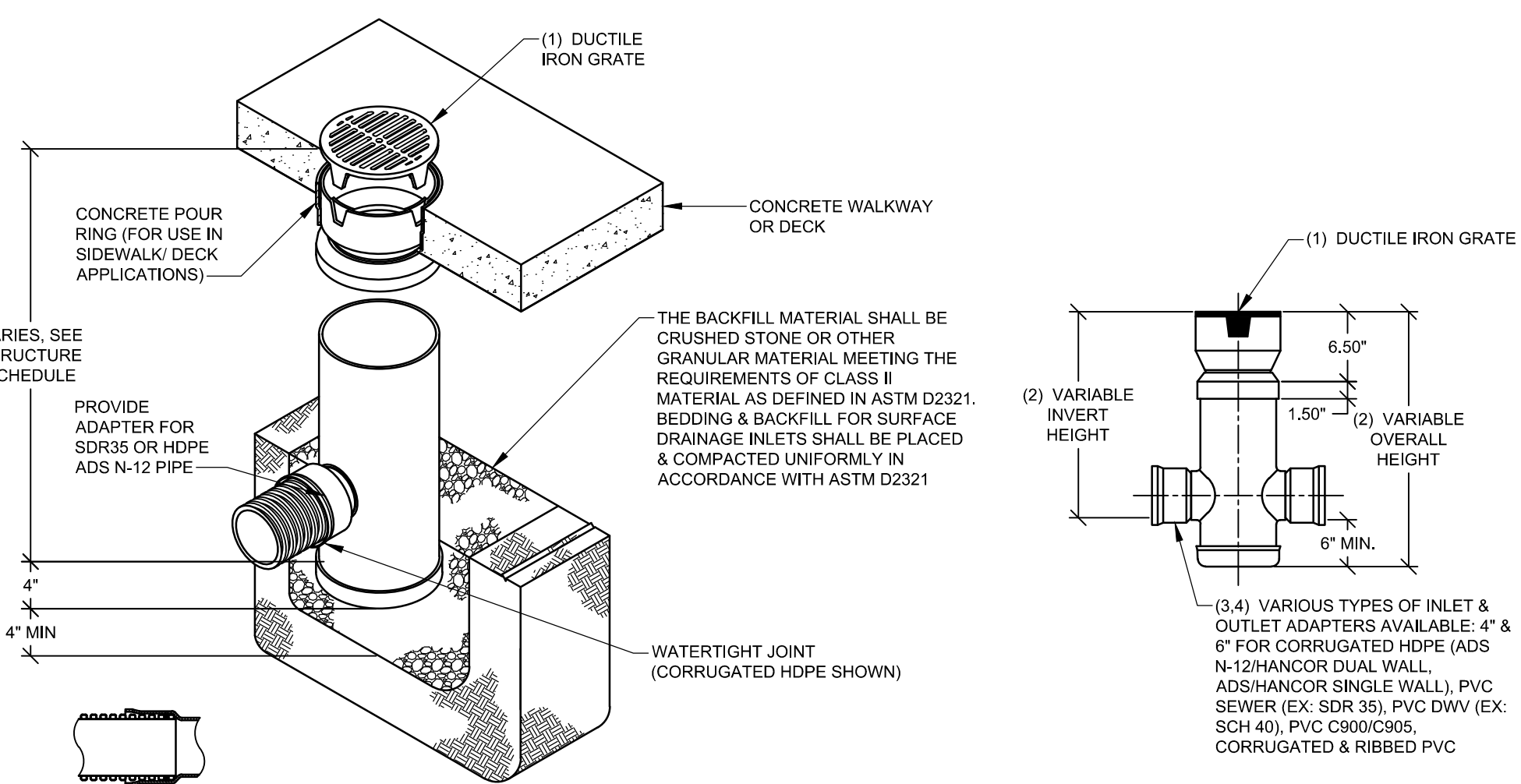
STORMTRAIN CLEANOUT
NOT TO SCALE



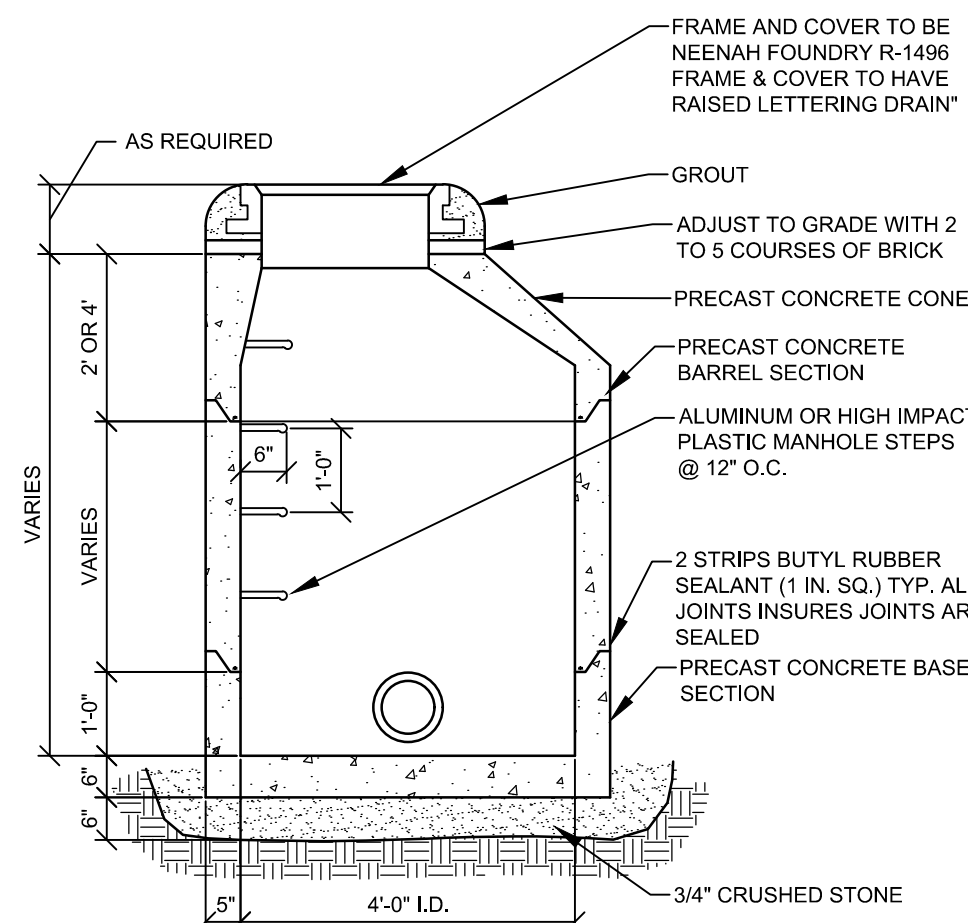
TRENCH SECTION WITHIN CITY R.O.W.
NOT TO SCALE

PLASTIC PIPE CONNECTIONS
NOT TO SCALE

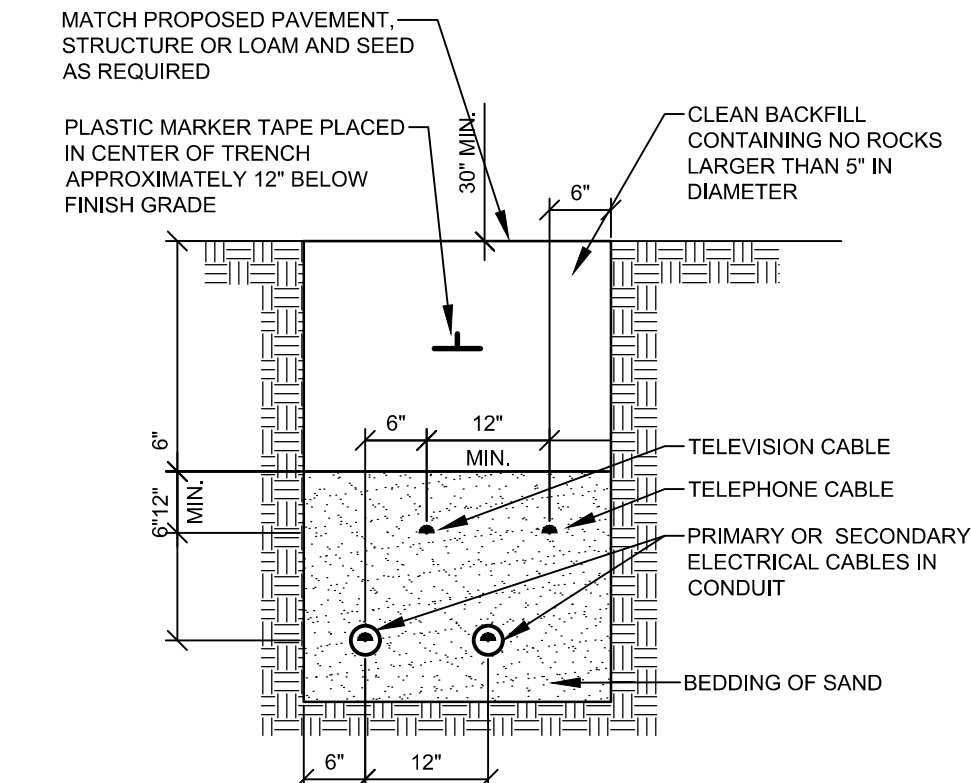
SEWER / FOUNDATION DRAIN SERVICE CONNECTION
NOT TO SCALE



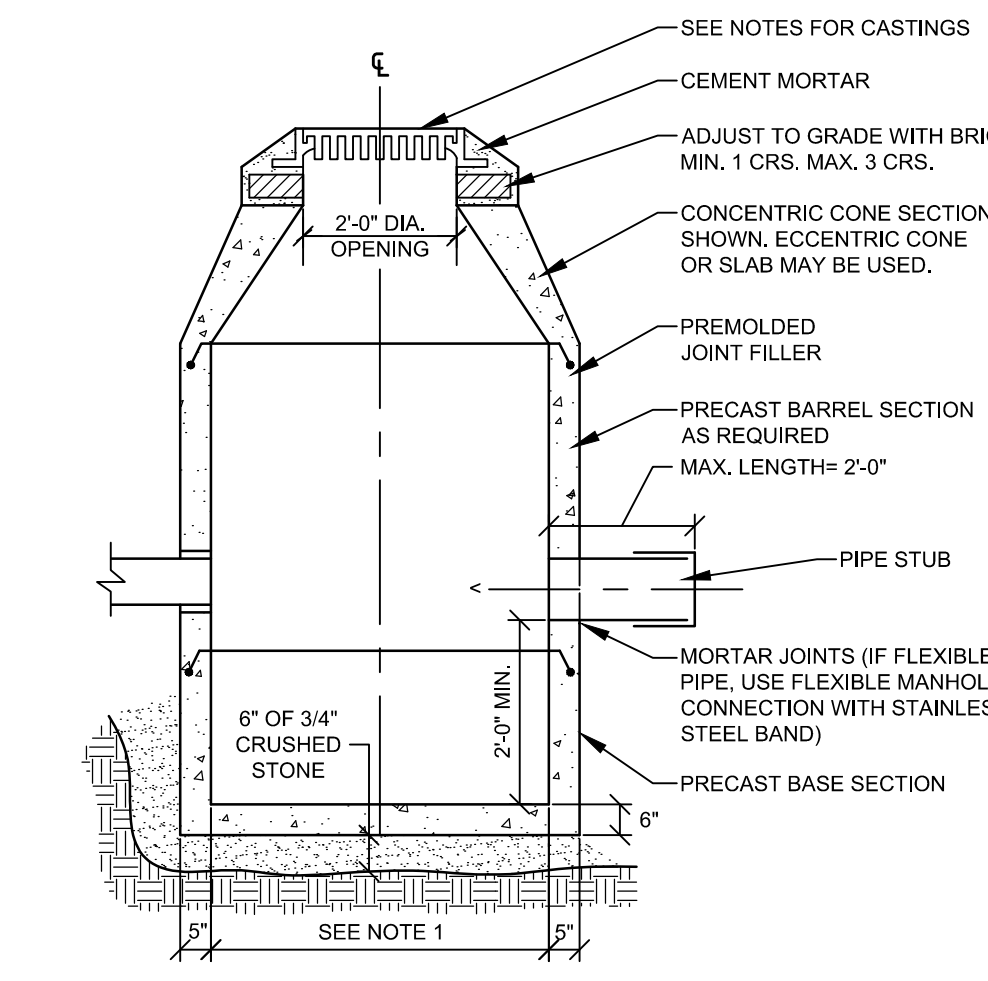
NYLOPLAST 10" DRAIN BASIN
NOT TO SCALE



PRECAST MANHOLE
NOT TO SCALE



UNDERGROUND CABLE INSTALLATION
NOT TO SCALE



CATCH BASIN
NOT TO SCALE

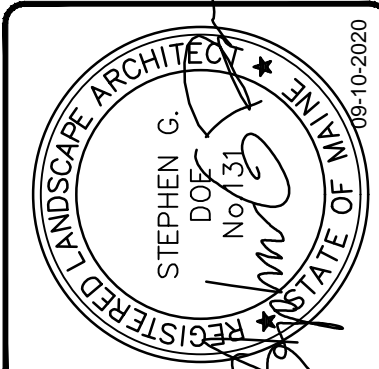
- NOTES:**
- 4'-0" I.D. TYPICAL. SOME STRUCTURES MAY REQUIRE LARGER I.D.
 - PROVIDE SHOP DRAWINGS
 - DRAINAGE STRUCTURES TO BE DESIGNED FOR H-20 LOADING.
 - PIPE SIZES AND INVERTS AS NOTED ON PLANS.
 - CATCH BASIN FRAME AND GRATE TO BE NEENAH FOUNDRY R-2554, OR APPROVED EQUAL.

GRATE OPTIONS	LOAD RATING	PART #	DRAWING #
PRECAST STANDARD	LIGHT DUTY	1096C28	7001-110-188
SOLID COVER	LIGHT DUTY	1096C29	7001-110-199
BRONZE	LIGHT DUTY	1096C28	7001-110-200
DOME	WA	1096C29	7001-110-201
DROP IN GRATE	LIGHT DUTY	1001D1	7001-110-020

1. GRATES/SOLID COVER SHALL BE DUCTILE IRON PER ASTM A536 GRADE 70-50-05, WITH THE EXCEPTION OF THE BRONZE GRATE.
2. CUSTOM DRAIN BASIN TO BE CUSTOM MANUFACTURED ACCORDING TO PLAN DETAILS. RISERS ARE NEEDED FOR BASINS OVER 84" DUE TO SHIPPING RESTRICTIONS.
3. ANDRAD DRAIN BASIN HAS FIXED ADAPTER LOCATIONS OF 0° & 180°. CUSTOM DRAIN BASIN ADAPTERS CAN BE MOUNTED ON ANY ANGLE 0° TO 360°.
4. DRAINAGE CONNECTION STUB JOINT TIGHTNESS SHALL CONFORM TO ASTM D3212 FOR CORRUGATED HDPE (ADS N-12/HANCOR DUAL WALL) & PVC SEWER (4" - 24").

- NOTES:**
- DRAINAGE STRUCTURES TO BE DESIGNED FOR H-20 LOADING.
 - PIPE SIZES AND INVERTS AS NOTED ON PLANS.
 - PIPE CONNECTIONS SHALL BE WATERTIGHT FLEXIBLE BOOT CONNECTORS.

NOT FOR CONSTRUCTION



REV.	BY	DATE	STATUS
F	SGD	08/10/2020	ISSUED FOR FINAL SITE PLAN APPROVAL
E	SGD	08/14/2020	ISSUED FOR DD
D	SGD	07/23/2020	REVISED FOR VALUE ENGINEER
C	SGD	03/12/2020	REVISED PER STAFF AND CLIENT COMMENTS
B	SGD	02/06/2020	ISSUED FOR SITE PLAN REVIEW
A	SGD	08/27/2019	ISSUED FOR CLIENT USE

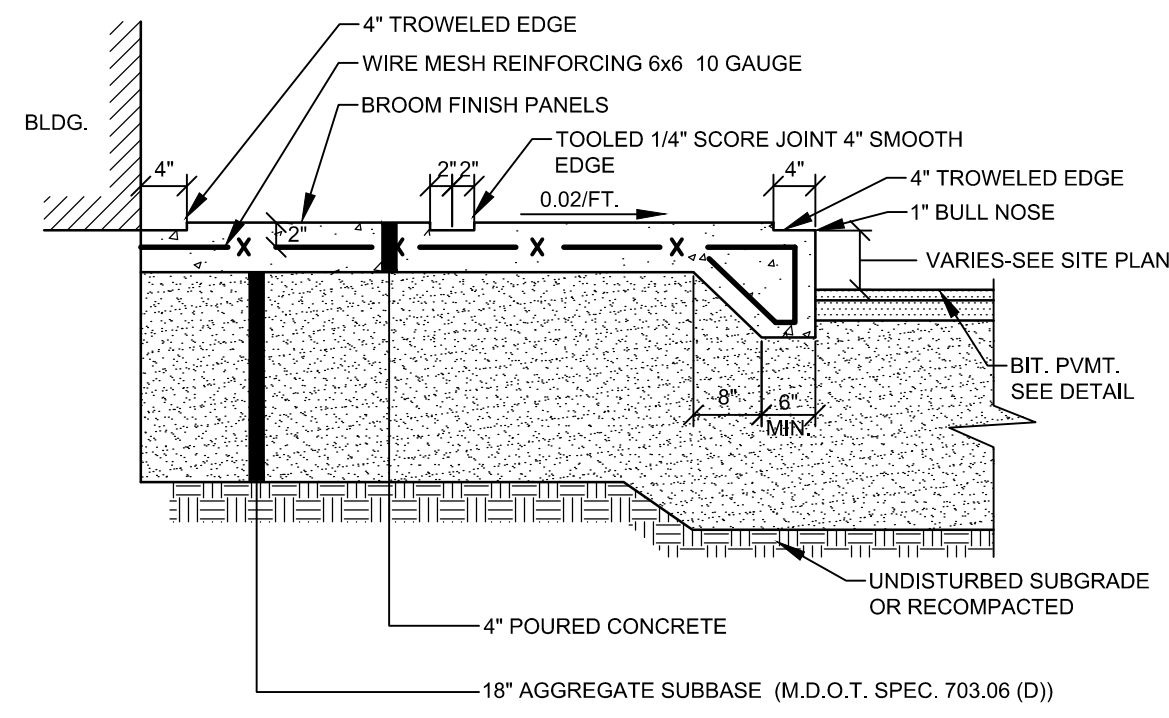


SCOTT SIMONS ARCHITECTS
75 Sibley Rd.
South Portland, ME 04106
Tel. 207-250-2100

DETAILS OF:
RICE PUBLIC LIBRARY
8 WENTWORTH STREET
KITTERY, MAINE 03904

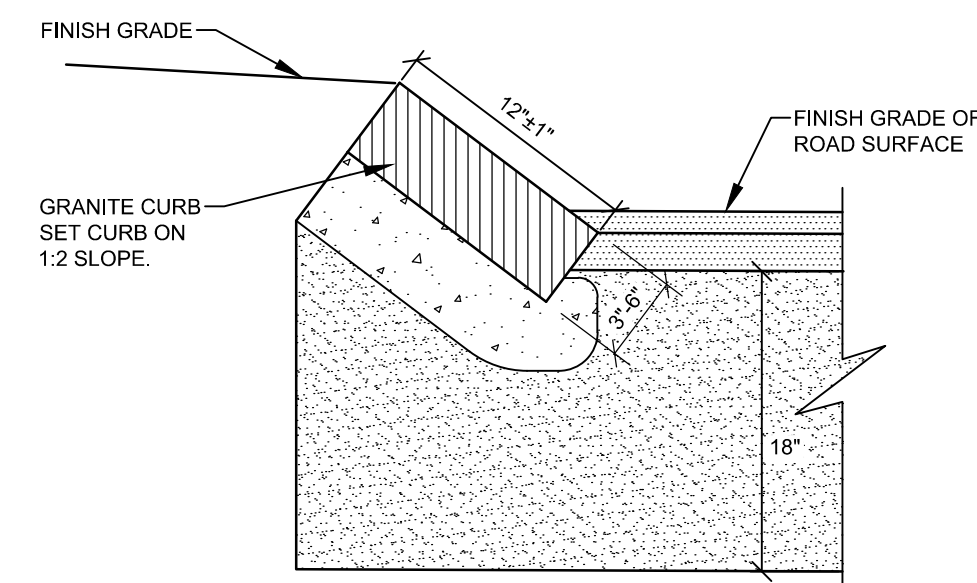
FOR:
LASSEL ARCHITECTS
P.O. BOX 370, 370 MAIN STREET
SOUTH BERWICK, MAINE 03908

DESIGNED	SGD
DRAWN	SRC
CHECKED	SGD
DATE	07/23/2020
SCALE	1" = 20'
PROJECT	18438



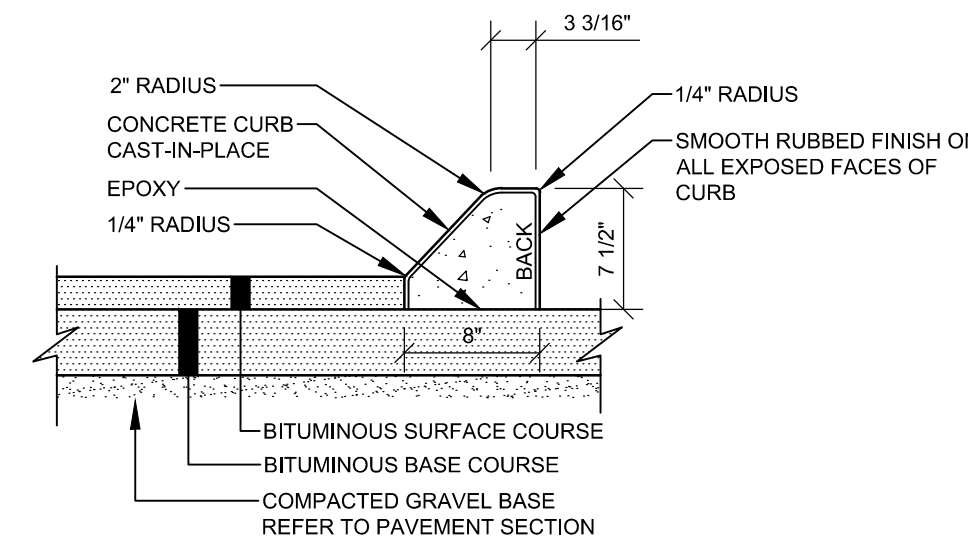
NOTE:
INSTALL 5'-0" SQUARE AREA BY 4" DEEP OF FROST-FREE MATERIAL
BELOW ALL HANDICAP RAMPS AND ENTRY POINTS AT BUILDING.

CONCRETE SIDEWALK
NOT TO SCALE



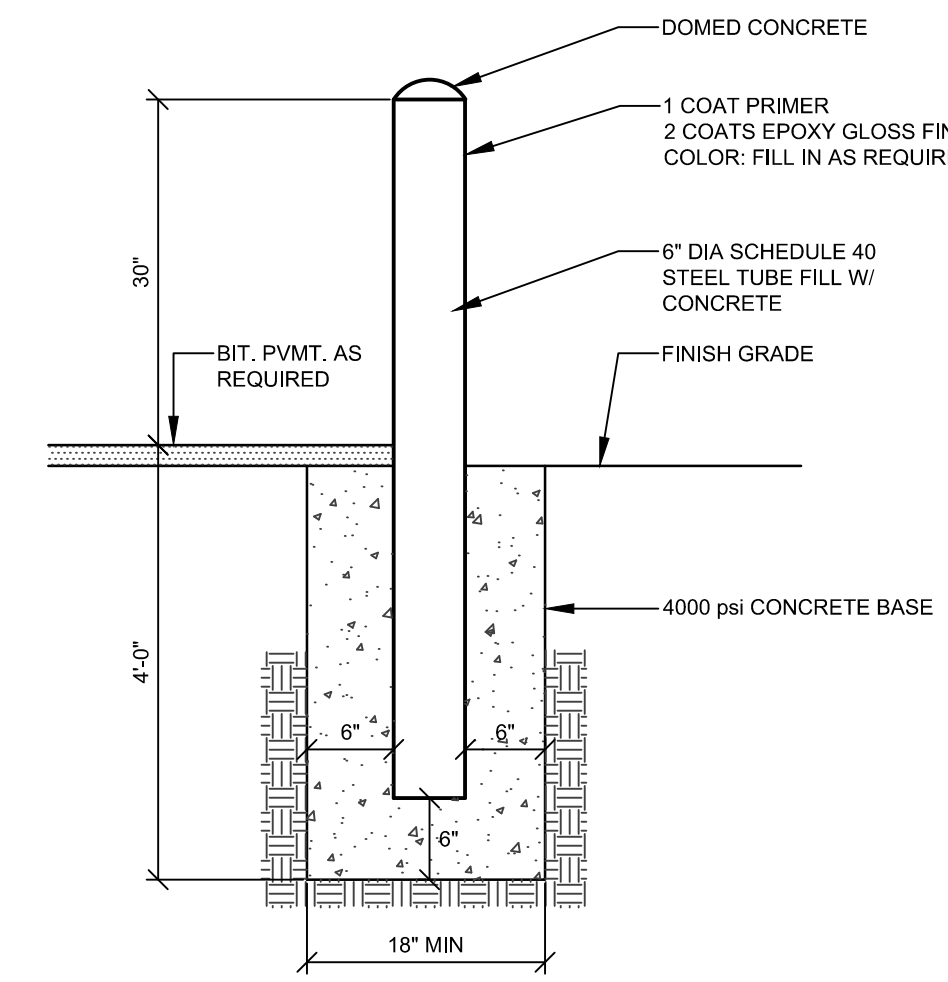
NOTE: REUSE EXISTING GRANITE CURB WHEN POSSIBLE

SLOPED GRANITE CURB (ALT #1)
NOT TO SCALE

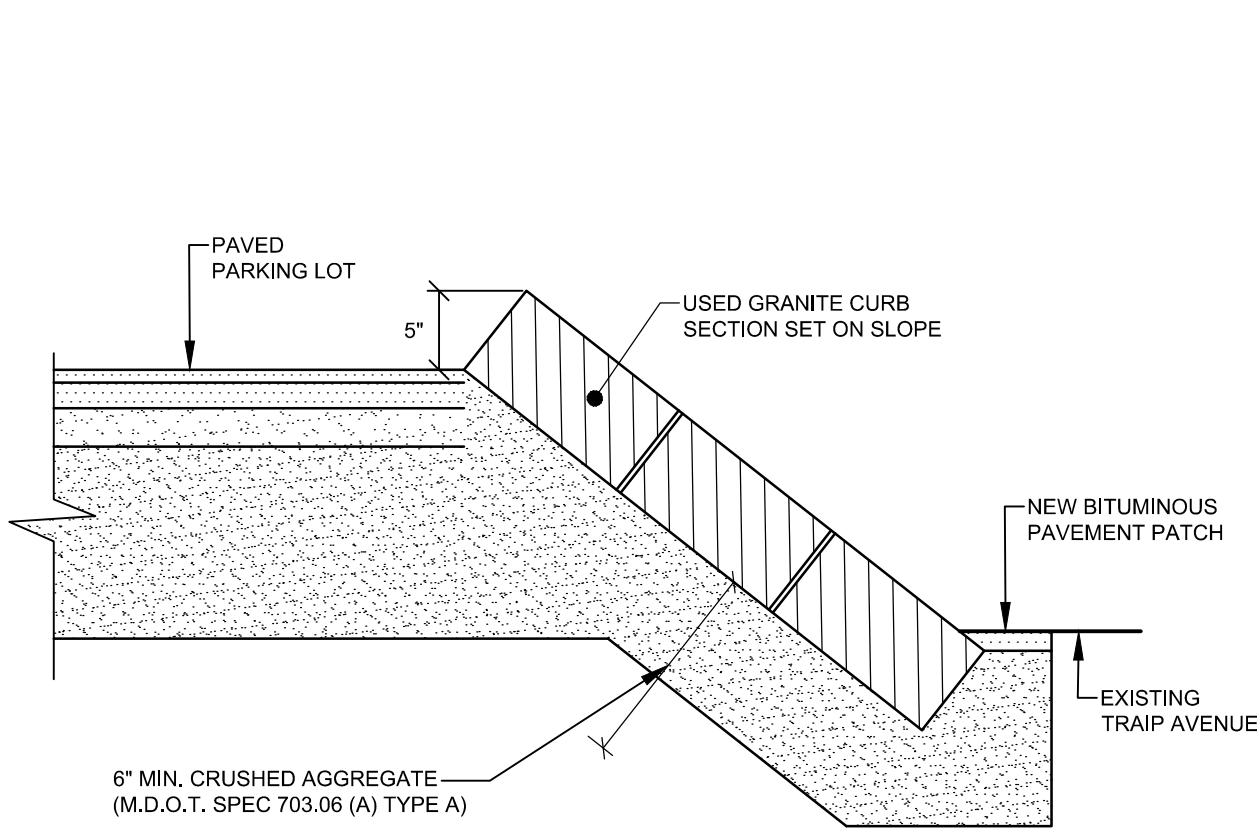


NOTES:
CONCRETE SHALL BE FIBER REINFORCED, 4000 PSI STRENGTH WITH 5% TO 7% AIR
ENTRAINMENT 1" TO 2" SLUMP.

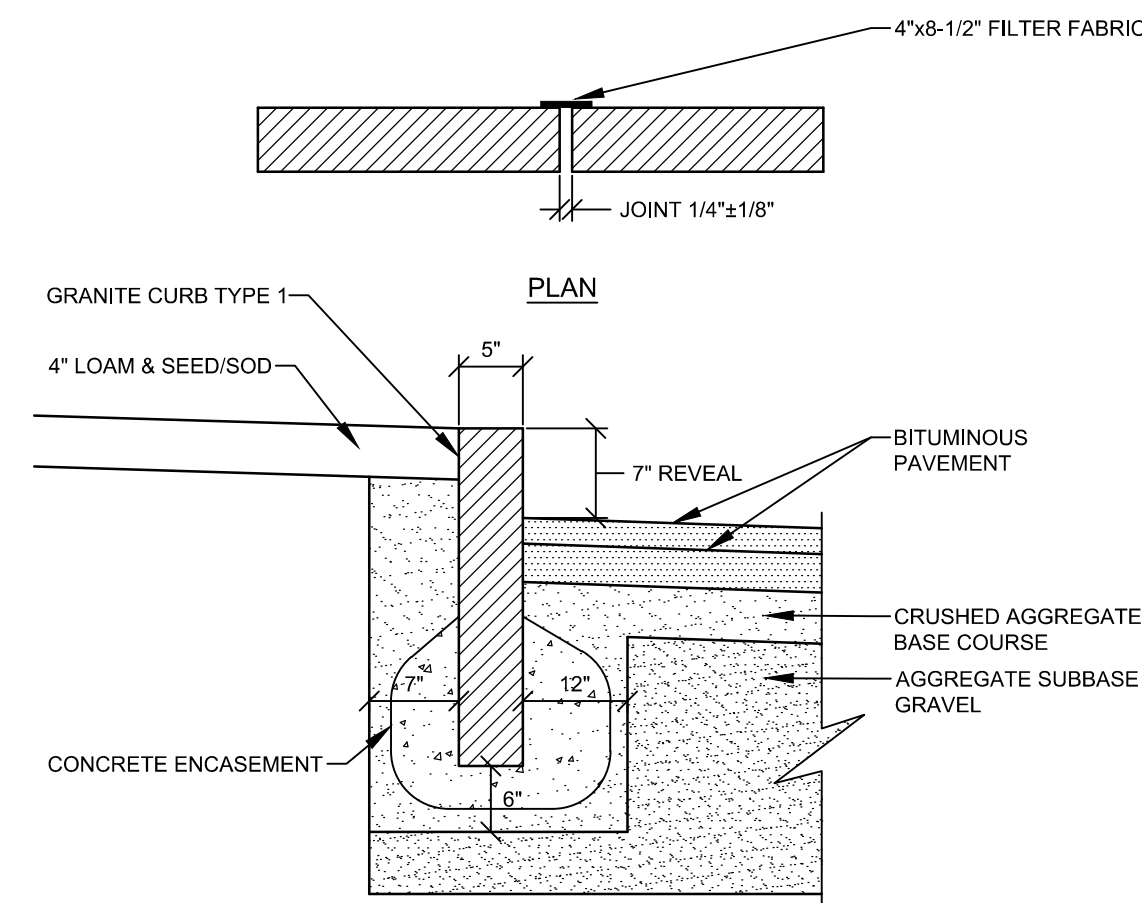
SLIPFORM CONCRETE CURB
NOT TO SCALE



METAL DOMED CONCRETE BOLLARD
NOT TO SCALE

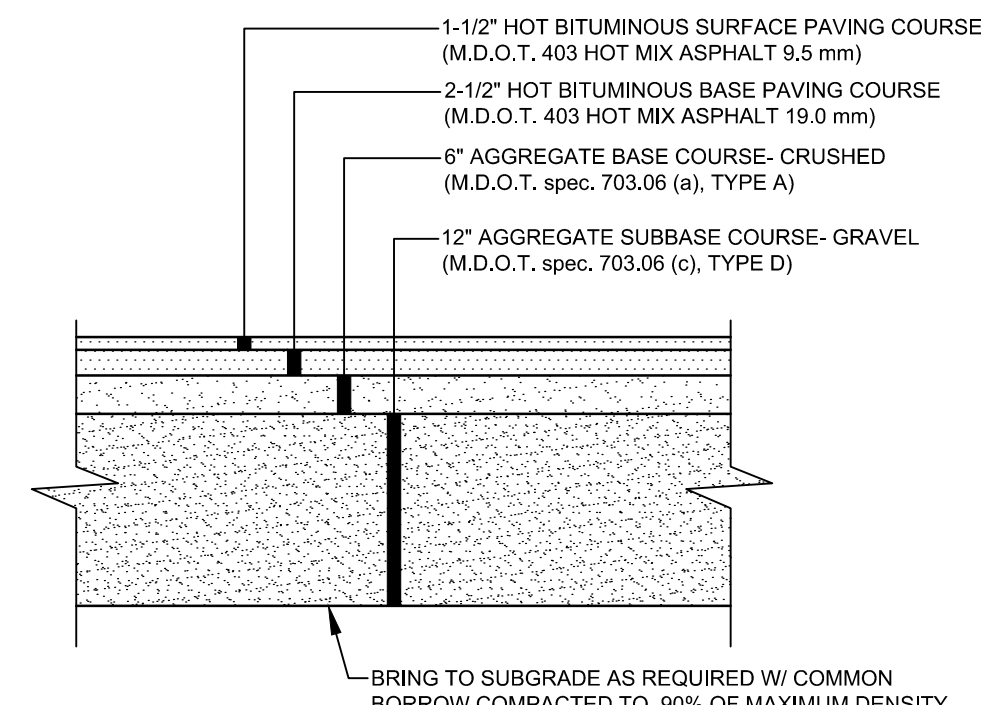


GRANITE CURB SLOPE
NOT TO SCALE



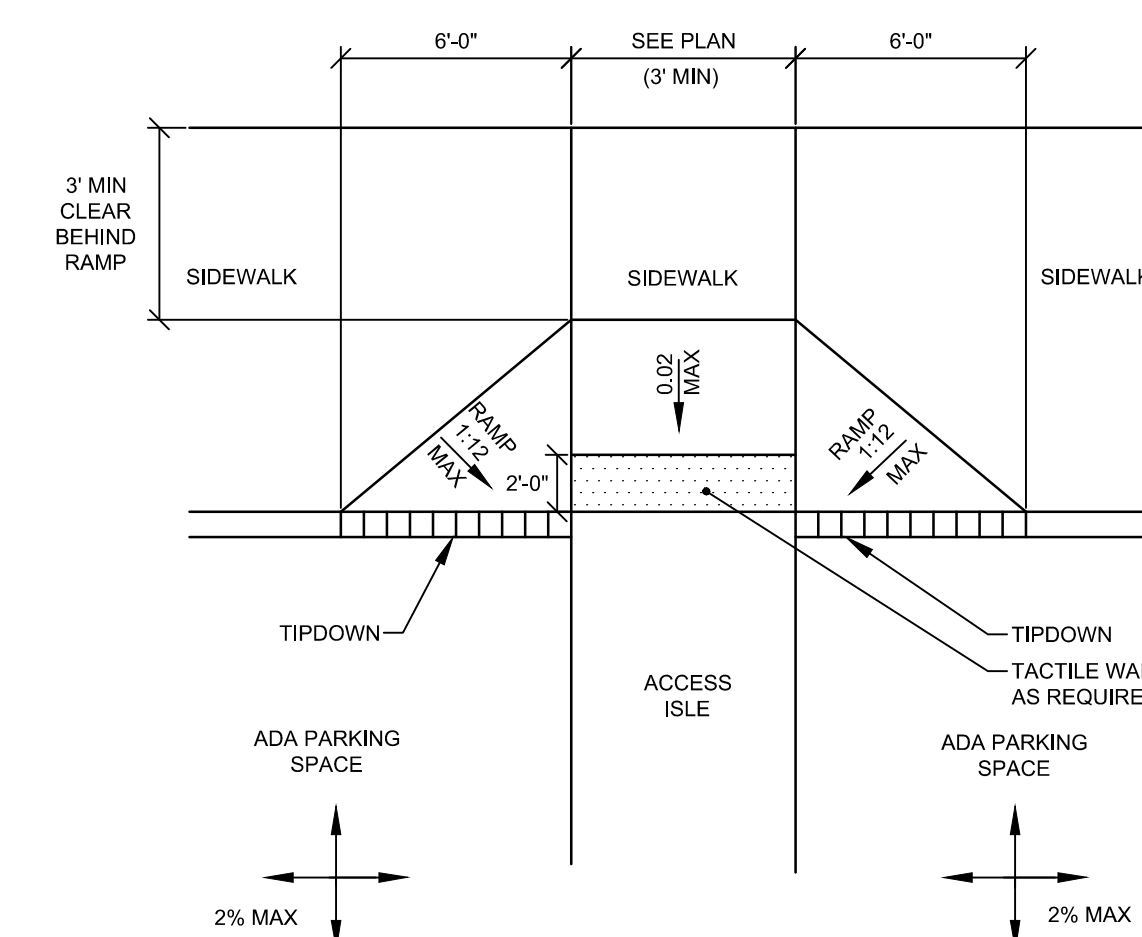
NOTES:
1. EXCAVATION INCIDENTAL TO COST OF CURB
2. SUBBASE SHALL BE COMPACTED TO A FIRM EVEN
SURFACE PRIOR TO SETTING OF CURB

VERTICAL GRANITE CURB IN TOWN R.O.W. (ALT #2)
NOT TO SCALE

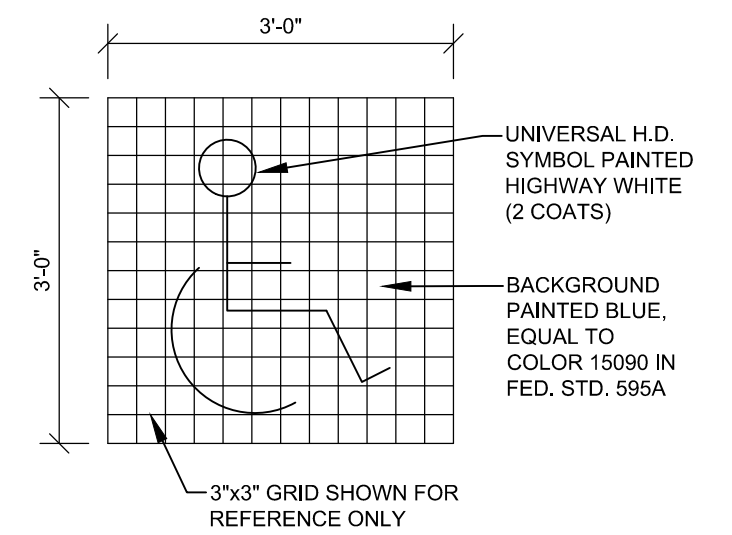


NOTES:
1. COMPACT GRAVEL SUBBASE, BASE COURSE TO 92% OF MAXIMUM DENSITY USING
HEAVY ROLLER COMPACTION.
2. CONTRACTOR SHALL SET GRADE STAKES MARKING SUBBASE AND FINISH GRADE
ELEVATIONS FOR CONSTRUCTION REFERENCE.
3. CONTRACTOR MAY REPLACE BITUMINOUS PAVING SECTION WITH TWO (2) 1-3/4" LIFTS OF
12.5mm SUPERPAVE MIX. SUBMIT PAVEMENT MIX DESIGN PRIOR TO CONSTRUCTION.

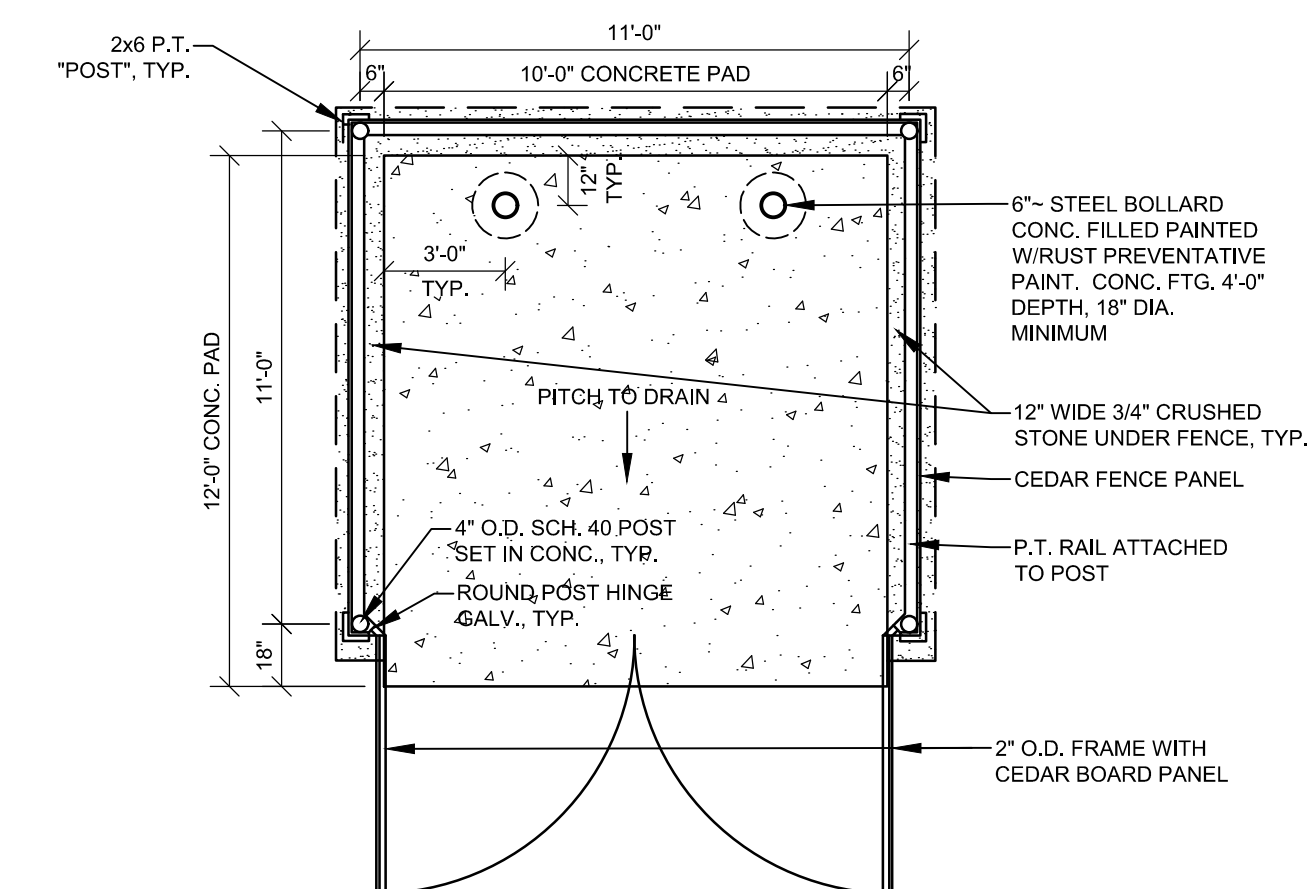
PAVED PARKING LOT SECTION
NOT TO SCALE



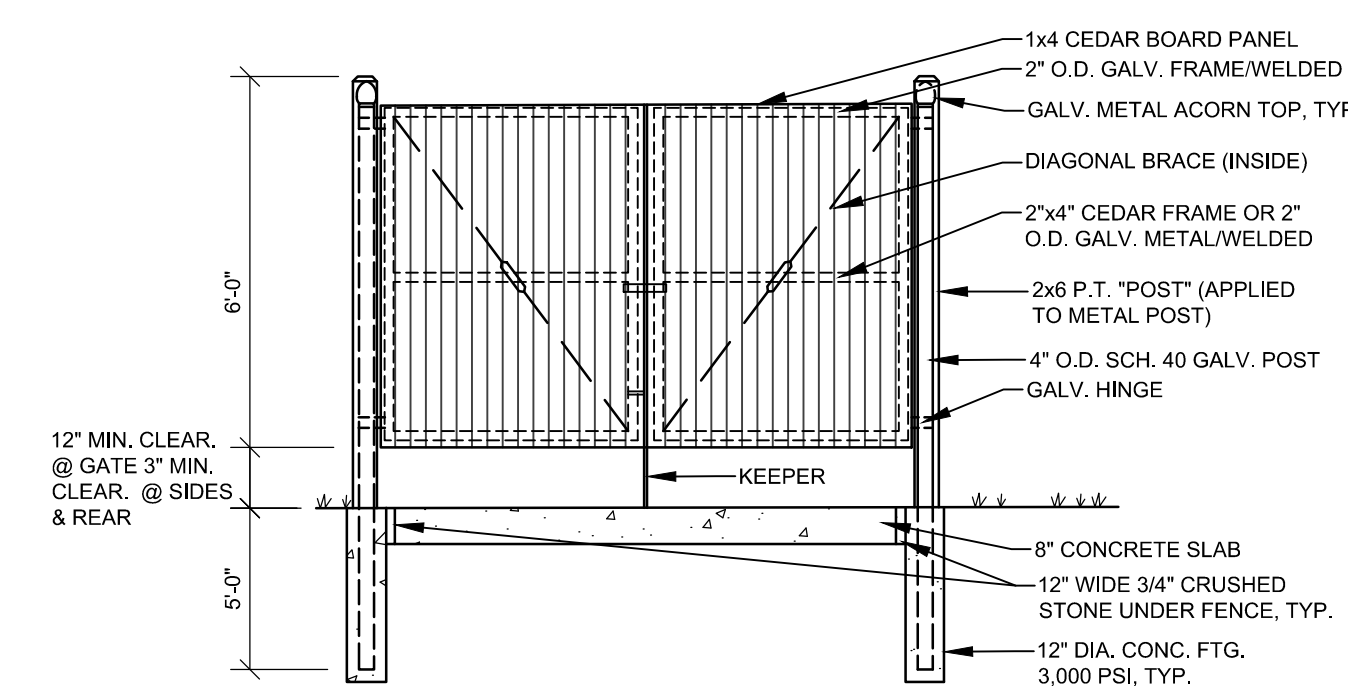
STRAIGHT ADA RAMP
NOT TO SCALE



ADA ACCESSIBLE SIGNS
NOT TO SCALE

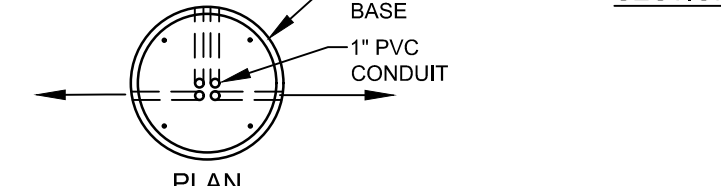
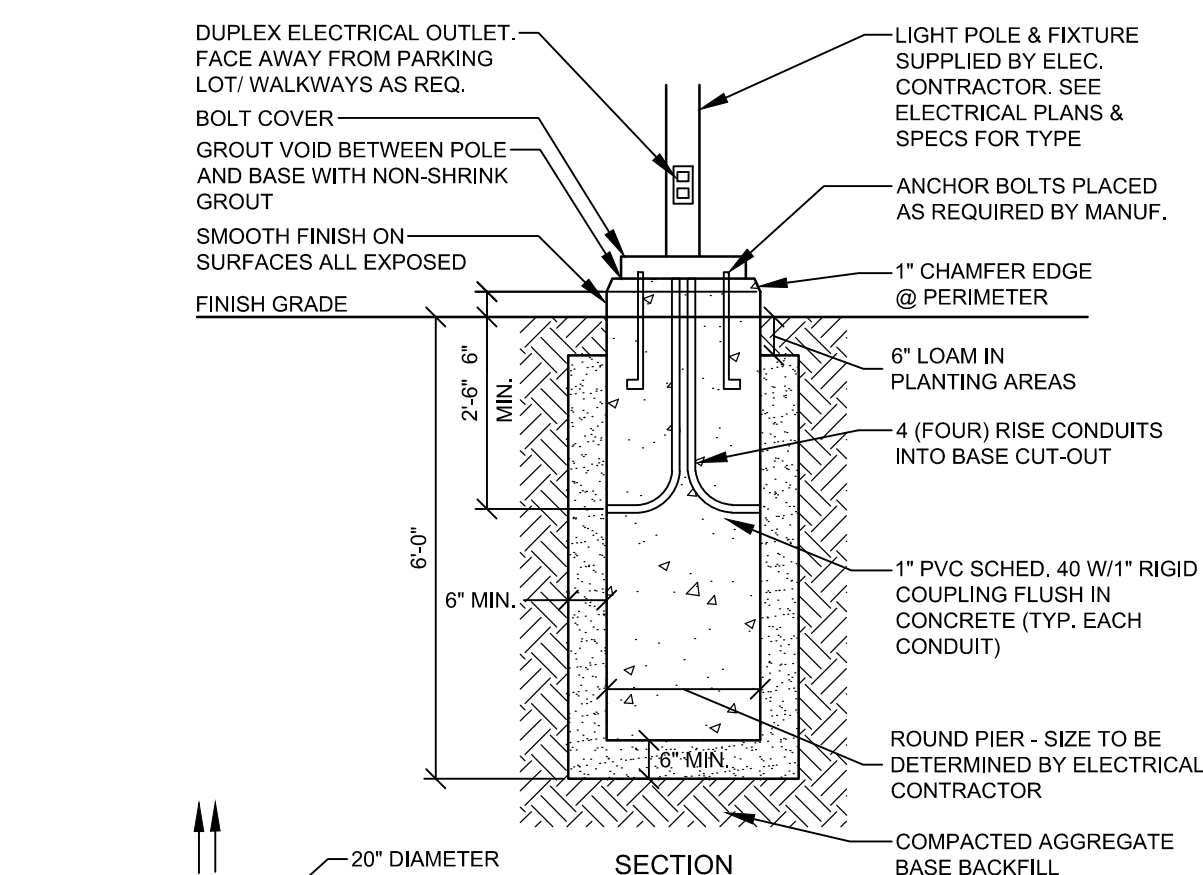


DUMPSTER ENCLOSURE
NOT TO SCALE



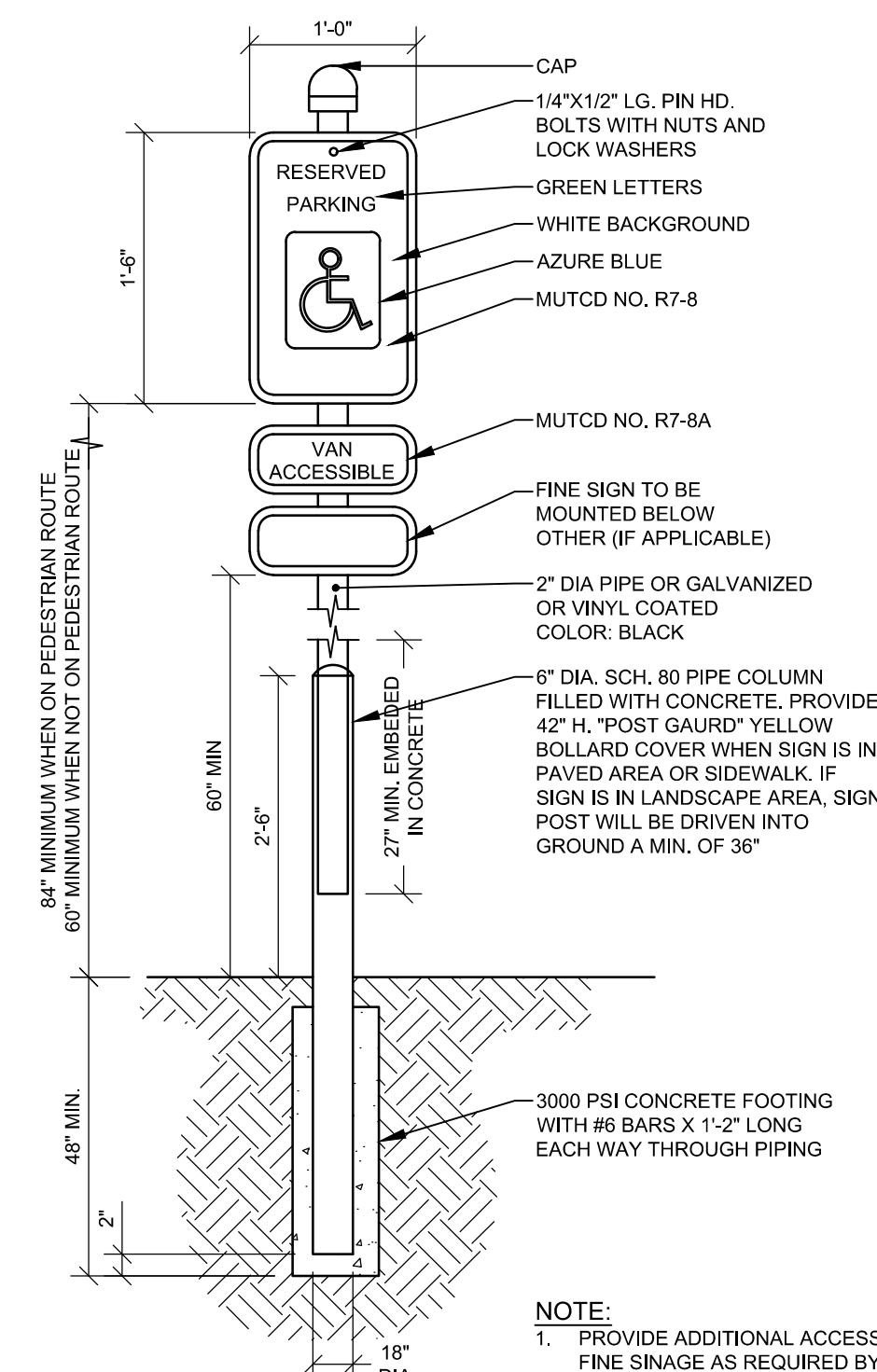
NOTES:
1. CONCRETE Fc=5000 psi. @ 28 DAYS WITH STEEL REINFORCEMENT
2. CONDUIT AND ANCHOR BOLTS PLACED AS REQUIRED PROVIDED BY ELECTRICAL CONTRACTOR
3. PROVIDE 2 COATS BITUMINOUS DAMPROOFING FOR ALL CONCRETE BELOW GRADE.
4. INSTALL BASE 3'-0" ABOVE FINISH GRADE IN LOCATIONS WHERE POLES ARE IN PARKING LOT PAVEMENT.
5. LIGHT POLE BASE AS MANUFACTURED BY SUPERIOR CONCRETE OR APPROVED EQUAL

20" ROUND LIGHT POLE BASE
NOT TO SCALE



NOTES:
1. CONCRETE Fc=5000 psi. @ 28 DAYS WITH STEEL REINFORCEMENT
2. CONDUIT AND ANCHOR BOLTS PLACED AS REQUIRED PROVIDED BY ELECTRICAL CONTRACTOR
3. PROVIDE 2 COATS BITUMINOUS DAMPROOFING FOR ALL CONCRETE BELOW GRADE.
4. INSTALL BASE 3'-0" ABOVE FINISH GRADE IN LOCATIONS WHERE POLES ARE IN PARKING LOT PAVEMENT.
5. LIGHT POLE BASE AS MANUFACTURED BY SUPERIOR CONCRETE OR APPROVED EQUAL

20" ROUND LIGHT POLE BASE
NOT TO SCALE



RESERVED PARKING SIGN (POLE MOUNTED)
NOT TO SCALE



R7-8 STANDARD PARKING SIGN

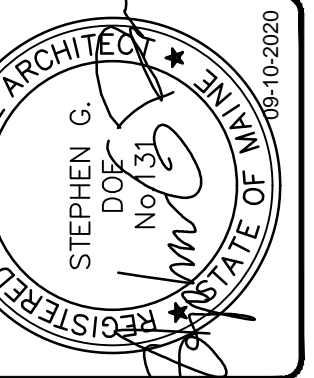


R7-8A STANDARD VAN ACCESS SIGN

A LOCAL MUNICIPALITY MAY HAVE MORE STRINGENT REGULATIONS BUT THEY CANNOT BE LESS STRINGENT THAN ADA STANDARDS OR STATE LAW.

NOTE:
1. PROVIDE ADDITIONAL ACCESSIBLE
FINE SIGNAGE AS REQUIRED BY STATE
AND LOCAL REGULATIONS.
2. PROVIDE ONE "VAN ACCESSIBLE"
SIGN PER 25 ADA SPACES REQUIRED.

NOT FOR
CONSTRUCTION



ISSUED FOR FINAL SITE PLAN APPROVAL	ISSUED FOR DD	REVISED FOR VALUE ENGINEER	REVISED PER STAFF AND CLIENT COMMENTS	ISSUED FOR SITE PLAN REVIEW	ISSUED FOR CLIENT USE
SGD 08/10/2020	SGD 08/14/2020	SGD 07/23/2020	SGD 03/12/2020	SGD 02/06/2020	SGD 08/27/2019
F	E	D	C	B	A
REV. BY:	DATE:	STATUS:			

THIS PLAN SHALL NOT BE MODIFIED WITHOUT WRITTEN PERMISSION FROM SEBAGO TECHNIQS, INC. ANY ALTERATIONS, AUTHORIZED OR OTHERWISE, SHALL BE AT THE USER'S SOLE RISK AND WITHOUT LIABILITY TO SEBAGO TECHNIQS, INC.

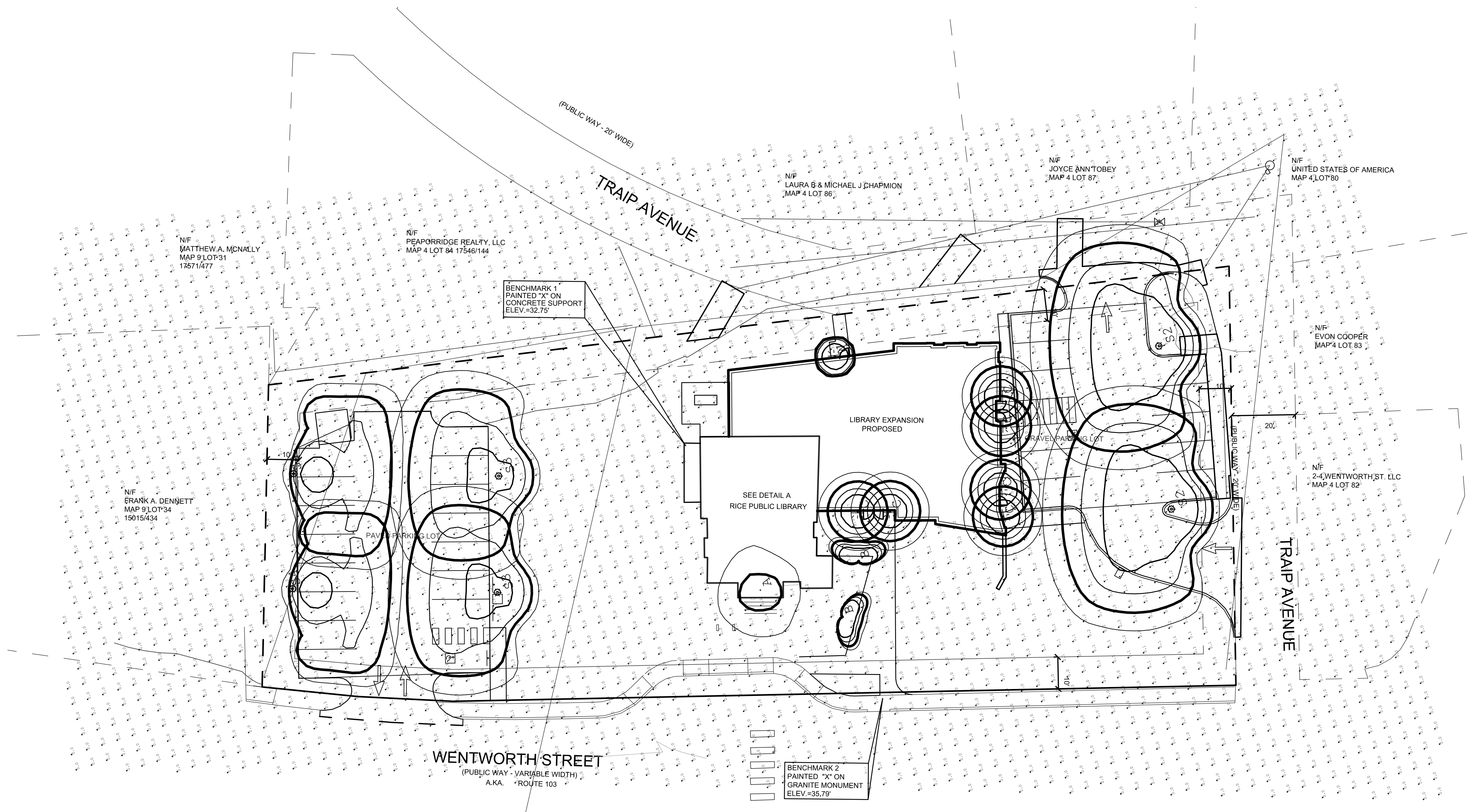


DETAILS
OF
RICE PUBLIC LIBRARY
8 WENTWORTH STREET
KITTERY, MAINE 03904
FOR:
LASSEL ARCHITECTS
P.O. BOX 370, 370 MAIN STREET
SOUTH BERWICK, MAINE 03908

SCOTT SIMONS ARCHITECTS
75 YORK STREET
PORTLAND, MAINE 04101

DESIGNED	SGD
DRAWN	SRC
CHECKED	SGD
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SCALE	1" = 20'
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NOT FOR CONSTRUCTION



StatArea_1 NORTHERN REMOTE PARKING LOT Illuminance (Fc) Average = 1.35 Maximum = 2.9 Minimum = 0.4 Avg/Min Ratio = 3.38 Max/Min Ratio = 7.25	StatArea_2 SOUTH PARKING LOT Illuminance (Fc) Average = 1.77 Maximum = 4.7 Minimum = 0.2 Avg/Min Ratio = 8.85 Max/Min Ratio = 23.50
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Symbol	Qty	Label	Arrangement	Description	[MANUFAC]
⊖	1	A	SINGLE	PENDENT LIGHT WITH LED BULB	Verbatim Americas
⊙	2	B	SINGLE	FLINDT 31.5 15W LED/4000K 120-277 NPA POST W/ANCHORAGE UNIT DIM 0-10V	Louis Poulsen Lighting
⊙	6	C	SINGLE	55943	BEGA Converted by LUMCat V 19.09.2014 / H.R.
⊙	2	S2	SINGLE	ICS-E02-LED-E1-T4-XX/RSS4A12S-1NX (12' POLE)	EATON - INVUE (FORMER COOPER LIGHTING)
⊙	2	S3	SINGLE	ICS-E01-LED-E1-T4-XX/ RSS4A12S-N1X (12' POLE)	EATON - INVUE (FORMER COOPER LIGHTING)
⊙	2	S4	SINGLE	ICS-E01-LED-E1-SL4-HSS-XX/RSS4A12S-N1X (12' POLE)	EATON - INVUE (FORMER COOPER LIGHTING)
⊖	1	W	SINGLE	33580	BEGA Converted by LUMCat V 22.04.2016 / H.R.

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SEBAGO
TECHNIQS
www.sebagotechniqs.com
75 John Roberts Rd.
Sullivan, IA
South Portland, ME 04106
Tel: 207-200-2100

PHOTOMETRIC PLAN - BY OTHERS
OF
RICE PUBLIC LIBRARY
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FOR:
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DESIGNED	BY OTHERS
DRAWN	BY OTHERS
CHECKED	BY OTHERS
DATE	09/10/2020
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NOTE: LIGHTING PLAN DONE BY CHARRON INC.