

TOWN OF KITTERY, MAINE TOWN PLANNING AND DEVELOPMENT DEPARTMENT

200 Rogers Road, Kittery, Maine 03904 PHONE: (207) 475-1323 - FAX: (207) 439-6806 www.kittery.org

APPLICATION: SITE PLAN REVIEW

555.50		\$300. 00 PLUS THE GREATER OF:		□ \$5	60/USE C	OF UNIT; OR		\$5.00/100 SQ FT OF GROSS FLOOR AREA			Application Fee Paid: \$ Date:		
FEE FO SITE PLA REVIEW	AN '			\$0.50/LINEAR FOOT OF DOCK, SLIP & FLOAT; OR			PROV		0/ Unit intended to vide overnight sleeping omodations		ASA Fee Paid: (TITLE 3.3 TOWN CODE) \$ Date:		
PROPERTY DESCRIPTION		Parcel ID	Мар	61	Lot	27A		Zone: Base: Overlay: MS4:		R-RL ———————————————————————————————————		al Land Area uare Feet)	483,516
		Physical Address	460	U.S. R	oute 1	Kittery, \	⁄ork	County,	M	E 03909			
		Name	PigP	enPar	ners								
PROPERTY OWNER'S	•	Phone	207-	363-0612		Mailing		40 Brickyard Court York County					
INFORMATION		Fax					Add	dress	ME 03909				
		Email	asie	asiegel@maine.rr.com									
		Name	Neil J. Rapoza, PE				me of iness	CI	VIL CONSULTAN	ΓS			
APPLICAN' AGENT	T'S	Phone		207-384-2550			N. 4 - 111		P.O. Box 100 South Berwick Maine 03908				
INFORMAT	ΓΙΟΝ	Fax	207-3	-384-2112			Mailing Address						
		Email	neil@	l@civcon.com									
	Existing	Use: The	exist	ing sit	e, ow	ned by P	Pig F	Pen Part	ne	ers, is the location	on c	of the Whe	n Pigs Fly
	Restaurant & Pizzeria. The lot utilizes an entrance and exit on to U.S. Route 1, as well as a												
	parking lot of 89 parking spaces.												
N O													
ESCR	Project Name: When Pigs Fly Restaurant & Pizzeria Proposed Use: Pig Pen Partners is proposing to modify the site of the existing When Pigs Fly												
CT D							<u>_</u>		_	and convert inte			
PROJECT DESCRIPTI										I continue to uti			
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WAIVER REQUEST

	Ordinance Section	Describe why this request is being made.					
	EXAMPLE 16.32.560 (B)- OFFSTREET PARKING.	***EXAMPLE*** Requesting a waiver of this ordinance since the proposed professional offices have a written agreement with the abutting Church owned property to share parking.					
Z							
IPTIO							
DESCRIPTION							
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Related Kittery Land Use Code concerning waivers and modifications:

16.10.8.2.5 Conditions or Waivers.

Conditions required by the Planning Board at the final plan review phase must have been met before the final plan may be given final approval unless so specified in the condition or specifically waived, upon written request by the applicant, by formal Planning Board action wherein the character and extent of such waivers which may have been requested are such that they may be waived without jeopardy to the public health, safety and general welfare.

16.7.4.1 Objectives Met. In granting modifications or waivers, the Planning Board must require such conditions as will, in its judgment, substantially meet the objectives of the requirements so waived or modified.

I certify that, to the best of my knowledge, the information provided in this application is true and correct and will not deviate from						
the plans submitted without notifying the Kittery Planning Department of any changes.						
Applicant's	27.32	Owner's	27.32			
Signature: Date:	3/8/2022	Signature: Date:	3/8/2022			

COMPLETED BY OFFICE STAFF

ASA CHARGE		AMOUNT	ASA CHARGE	AMOUNT
REVIEW			SERVICES	
LEGAL FEES (TBD)		RECORDER	\$35
ENGINEERS REVIEW	TBD)		FACT FINDING (TBD)	
ABUTTER NOTICES			3 RD PARTY INSPECTIONS (TBD)	
Postage		\$20	OTHER PROFESSIONAL SERVICES	\$50
LEGAL NOTICES			PERSONNEL	
ADVERTISING		\$300	SALARY CHARGES IN EXCESS OF 20 HOURS	
SUPPLIES				
OFFICE	·	\$5		
SUB	TOTAL		SUB TOTAL	
			TOTAL ASA REVIEW FEES	

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Minimum Submission Requirements					
	15 COPIES OF THIS APPLICATION				
	15 COPIES OF THE PROPOSED SITE PLAN – 12 REDUCED SIZE AT 11"X17"AND 3 FULL SIZE AT 24"X 36"				
	1 PDF OF THE SITE PLAN SHOWING GPS COORDINATES				
SUBN	SUBMITTALS THE TOWN PLANNER DEEMS SUFFICIENTLY LACKING IN CONTENT WILL NOT BE SCHEDULED FOR PLANNING BOARD REVIEW.				

Related Ordinances: Kittery Land Use Code- Title 16

16.10.5.2 Planner Review and Confirmation of Submittal Content - Preliminary Plan.

A completed application must include on the plan or attached thereto, the following items, unless upon the applicant's written request, the Planning Board, by formal action, waives or defers any requirement(s) for submission.

A. A minimum of fifteen (15) paper copies of the application form, plan and all attachments thereto plus if applicable, five (5) paper copies of the 24 x 36 inches size plan sheets.

B. Plan must include:

- 1. Plan sheets drawn on a reproducible medium and must measure no less than eleven (11) inches by seventeen (17) inches and no larger than twenty-four (24) inches by thirty-six (36) inches; with a:
- 2. Scale of the drawings no greater than one inch equals thirty (30) feet for developments less than ten (10) acres, and one inch equals fifty (50) feet for all others;
- 3. Code block in the lower right-hand corner. The block must contain:
 - a. Name(s) and address(es) of the applicant and owner,
 - b. Name of the project.
 - c. Name and address of the preparer of the plan, with professional seal, if applicable,
 - d. Date of plan preparation/revision, and a unique ID number for the plan and any revisions;
- 4. Standard boundary survey conducted by a surveyor licensed in the state of Maine, in the manner recommended by the State Board of Registration for Land Surveyors;
- 5. An arrow showing true north and the magnetic declination, a graphic scale, and signature blocks for the owner(s) and members of the Planning Board;
- 6. Locus map showing the property in relation to surrounding roads, within two thousand (2,000) feet of any property line of the development,
- 7. Surveyed acreage of the total parcel, of rights-of-way, wetlands, and area to be disturbed and amount of street frontage;
- 8. Names and addresses of all owners of record of property abutting the development, including those across a street;
- 9. Locations of essential physical features such as watercourses, forest cover, and outcroppings
- 10. Proposed development area conditions including, but not limited to:
 - a. Structures; their location and description including signs, to be placed on the site, floor plan of exterior walls and accesses located within one hundred (100) feet of the property line;
 - b. Utilities proposed including power, water, sewer, holding tanks, bridges, culverts and drainage ways;

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- c. Sewage facilities type and placement. Test pit locations, at least two of which must meet the State of Maine Plumbing Code requirements, must be shown;
- d. Domestic water source;
- e. Parks, open space, or conservation easement locations;
- f. Lot lines, interior and exterior, right-of-way, and street alignments;
- g. Road and other paved ways plans, profiles and typical sections including all relevant data;
- Setbacks Existing and proposed;
- i. Machinery permanently installed locations likely to cause appreciable noise at the lot lines;
- j. Raw, finished or waste materials to be stored outside the buildings, and any stored material of a toxic or hazardous nature;
- k. Topographic contours of existing contours and finished grade elevations within the development;
- I. Sidewalks, curbs, driveways, fences, retaining walls and other artificial features locations and dimensions proposed;;
- m. Landscaping required including size and type of plant material;
- n. Temporary markers locations adequate to enable the Planning Board to readily locate and appraise the layout of the development;
- o. Land proposed to be dedicated to public use and the conditions of such dedication;
- p. Natural features or site elements to be preserved.

C. Supporting documentation must include:

- 1. Vicinity map and aerial photograph showing the property in relation to surrounding properties, roads, geographic, natural resource (wetland, etc.), historic sites, applicable comprehensive plan features such as proposed park locations, land uses, zones, and other features within five hundred (500) feet from any boundary of the proposed development:
- 2. Existing Development Area Conditions including but not limited to:
 - a. Location and description of all structures, including signs, existing on the site, together with accesses located within one hundred (100) feet of the property line;
 - b. Essential physical features such as watercourses, wetlands, flood plains, wildlife habitat areas, forest cover, and outcroppings;
 - c. Utilities existing, including power, water, sewer, holding tanks, bridges, culverts and drainage ways;
- 3. Legal interest documents showing legal interest of the applicant in the property to be developed. Such documents must contain the description upon which the survey was based;
- 4. Property encumbrances currently affecting the property, as well as any proposed encumbrances;
- 5. Water District approval letter, if public water is used, indicating there is adequate supply and pressure to be provided to the development;

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- 6. Erosion and sedimentation control plan endorsed by the York County soil and water conservation district;
- 7. Stormwater management plan for stormwater and other surface water drainage prepared by a registered professional engineer including a Maintenance Plan and Agreement that defines maintenance responsibilities, responsible parties, shared costs, and schedule. Where applicable, a Maintenance Agreement must be included in the Document of Covenants, Homeowners Documents and/or as riders to the individual deed and recorded with the York County Registry of Deeds.
- 8. Soil survey for York County covering the development. Where the soil survey shows soils with severe restrictions for development, a high intensity Class "A" soil survey must be provided;
- 9. Vehicular traffic report estimating the amount and type of vehicular traffic that will be generated by the development on a daily basis and for peak hours.
- 10. Traffic impact analysis in accordance with subsection (E)(2) for developments involving forty (40) or more parking spaces or which are projected to generate more than four hundred (400) vehicle trips per day;
- 11. Test pit(s) analysis prepared by a licensed site evaluator when sewage disposal is to be accomplished by subsurface disposal, pits, prepared by a licensed site evaluator;
- 12. Town Sewage Department or community system authority letter, when sewage disposal is to be through a public or community system, approving the connection and its location;
 - a. Additional submissions as may be required by other sections of this Code such as for clustered development, mobile home parks, or junkyards must be provided.
 - b. Letters of evaluation of the development by the Chief of Police, Fire Chief, Commissioner of Public Works, and, for residential applications, the superintendent of schools, must be collected and provided by the Town Planner.
 - c. Additional Requirements. In its consideration of an application/plan, the Planning Board may at any point in the review, require the applicant to submit additional materials, studies, analyses, and agreement proposals as it may deem necessary for complete understanding of the application.
- 1. Such materials may include:
- 1. Traffic impact study, including the following data:
 - An executive summary outlining the study findings and recommendations.
 - b. A physical description of the project site and study area encompassed by the report with a diagram of the site and its relationship to existing and proposed development sites within the study area.
 - c. A complete description of the proposed uses for the project site (in cases where specific uses have not been identified, the highest traffic generators within the category best fitting the proposed development must be used to estimate traffic generators).
 - d. Existing land uses and zone(s) in the vicinity of the site must be described. Any proposals for the development of vacant parcels or redevelopment of parcels within the study area of which the municipality makes the applicant aware, must be included in the description.
 - e. Roadway geometry and existing traffic control devices on all major streets and intersections affected by the anticipated traffic generated.
 - f. Trip generation must be calculated for the proposed project and other proposed new projects and redevelopment projects within the study area using the most recent data available from the Institute of Transportation Engineers' (ITE) Trip Generation Guide, and/or actual field data collected from a comparable trip generator (i.e., comparable in size, location and setting). This data will be presented in a summary table

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such that assumptions on trip generation and rates arrived at by the engineer are fully understandable to the Planning Board.

- g. The anticipated trip distribution of vehicles entering and exiting the proposed site during the appropriate peak hour(s) must be described and diagrammed.
- h. Trip assignment, the anticipated utilization of study area roadways by traffic generated by the proposed project, must be described and diagrammed.
- i. Existing traffic conditions in the study area will be identified and analyzed based upon actual field counts and/or recent available machine counts.
- j. Existing traffic conditions in the study area will be described and diagrammed, specifically AADT, appropriate peak design hour(s), traffic volumes, roadway and intersection capacities, and levels of service.
- k. Existing safety conditions must be evaluated based upon the traffic accident data available for the most current three years and described including link and node critical rate factors (CRF).
- I. Future traffic conditions on the roadway system will be estimated based on existing volumes, projected traffic growth in the general study area, projected traffic from approved development, and traffic generated by the proposed project, specifically AADT traffic, appropriate peak hour(s) traffic volumes, roadway and intersection capacity, roadway and intersection levels of service will be analyzed. When other projects are being proposed within the impact area of the project, the Planning Board may require these projects to be incorporated into the analysis.
- m. When the analysis of the proposed project's impact on traffic indicates unsatisfactory CRF, levels of service or operating capacity on study area roadways and intersections, a description of proposed improvements to remedy identified deficiencies must be included.
- n. The base data collected and analyzed during the course of the traffic impact study must be made available upon request of the Planning Board.
- o. If a development that requires a traffic impact study is within five hundred (500) feet of York or Eliot, Maine or if the study identifies impacts on segments of Route 1 or Route 236 or on their intersections located in York or Eliot, Maine, the applicant must provide evidence that a copy of the impact study has been given to the impacted municipality's chief administrative officer;
- 3. Environmental Analysis. An analysis of the effects that the development may have upon surrounding lands and resources, including intensive study of groundwater, ecosystems, or pollution control systems, as the Planning Board, upon review and recommendation by the Conservation Commission, may deem necessary;
- 4. Hydrologic Analysis. When required, an analysis of the effects that the development may have on groundwater must be conducted in accordance with Section 16.32.520. This analysis is always required for mobile home park proposals.
- 5. Wireless Communication Services Facilities (WCSF) Analysis.
 - a. A visual impact analysis prepared by a landscape architect or other qualified professional acceptable to the Town that quantifies the amount of visual impact on properties located within five hundred (500) feet, within two thousand five hundred (2,500) feet and within two miles of the WCSF. This analysis will include recommendations to mitigate adverse visual impacts on such properties;
 - b. An analysis prepared by a qualified professional acceptable to the Town that describes why this site and structure is critical to the operation for which it is proposed. The analysis must address, at a minimum: existing and proposed service area; how this WCSF is integrated with other company operations, particularly other structures in Kittery and surrounding communities; future expansion needs in the area; the effect on company operations if this structure is not constructed in this location; other sites evaluated for location of this

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structure and how such sites compare to the proposed site; other options, if any, which could be used to deliver similar services, particularly if the proposed equipment can be co-located (shared use) on an existing structure; and an analysis to the projected life cycle of this structure and location;

- Certification by a structural engineer that construction of the structure satisfies all federal, state and local building code requirements as well as the requirement of maximum permitted co-location at the site as approved by the Planning Board / Town Planner;
- d. Payment of all required performance guarantees as a condition of plan approval, with a note on the plan so stating;
- e. Payment of the Planning Board application fees;
- f. And all other requirements per Section 16.10.

16.10.7.2 Final Plan Application Submittal Content.

- A. A complete final plan application must fulfill all the requirements of a preliminary plan as indicated in subsection 16.36.??? of this section and must show the following items, unless the Planning Board, by formal action, upon the applicant's written request, waives or defers any requirement(s) for submission. If no changes occurred to the preliminary plan it also may be considered to be the final plan.
- B. Preliminary plan information including vicinity map and any amendments thereto suggested or required by the Planning Board, or other required reviewing agency;
- C. Street names and lines, pedestrian ways, lots, easements, and areas to be reserved for or dedicated to public use;
- D. Street length of all straight lines, the deflection angles, radii, lengths of curves and central angles of all curves, tangent distances and tangent bearings;
- E. Lots and blocks within a subdivision numbered in accordance with local practice;
- F. Markers/permanent reference monuments: Their location, source references, and where required, constructed in accordance with specifications herein;
- G. Structures; their location and description including signs, to be placed on the site, floor plans and elevations of principal structures as well as detail of all structures showing building materials and colors, and accesses located within one hundred (100) feet of the property line;
- H. Outdoor lighting and signage plan; if the
- 1. Lighting plan, if the application involves the construction of more than five thousand (5,000) square feet of nonresidential floor area, or the creation of more than twenty thousand (20,000) square feet of impervious area, or the creation of three or more dwelling units in a building; prepared by a qualified lighting professional, showing at least the following at the same scale as the site plan:
 - a. All buildings, parking areas, driveways, service areas, pedestrian areas, landscaping, and proposed exterior lighting fixtures;
 - b. All proposed lighting fixture specifications and illustrations including photometric data, designation as "cut-off" fixtures, color rendering index (CRI) of all lamps (bulbs), and other descriptive information on the fixtures;
 - c. Mounting height of all exterior lighting fixtures;
 - d. Lighting analyses and luminance level diagrams or photometric point by point diagrams on a twenty (20) foot grid showing that the proposed installation conforms to the lighting level standards of the ordinance codified in this Section together with statistical summaries documenting the average luminance, maximum luminance, minimum luminance, average to minimum uniformity ratio, and maximum to minimum uniformity ratio for each parking area, drive, canopy, and sales or storage area;

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- e. Drawings of all relevant building elevations showing the fixtures, the portions of the walls to be illuminated, the luminance levels of the walls, and the aiming points for any remote light fixtures; and
- f. A narrative that describes the hierarchy of site lighting hierarchy and how the lighting will be used to provides safety, security, and aesthetic effects.
- I. Machinery permanently installed locations likely to cause appreciable noise at the lot lines;
- J. Materials (raw, finished or waste) storage areas, their types and location; and any stored toxic or hazardous materials, their types and locations;
- K. Fences, retaining walls and other artificial features locations and dimensions proposed;
- Landscaping plan including location, size, and type of plant material;
- M. Boundary markers for protected land areas permanently marked using Town environmental boundary markers, their location and type. The five boundary markers are: (1) Conservation Land, (2) Protected Wetland, (3) Protected Vernal Pool, (4) Wildlife Habitat, and (5) Wetlands. Depending on the proposed development the required markers(s), number of markers, placement and spacing, and the method of mounting.
- N. Municipal impact analysis of the relationship of the revenues to the Town from the development and the costs of additional publicly funded resources including;
- Review for impacts. A list of the construction items that will be completed by the developer prior to the sale of lots.
- 2. Municipal construction and maintenance items. A list of construction and maintenance items that must be borne by the municipality, which must include, but not be limited to:
- a.. Schools, including busing;
- b. Road maintenance and snow removal;
- c. Police and fire protection;
- d. Solid waste disposal;
- e. Recreation facilities:
- f. Runoff water disposal drainage ways and/or storm sewer enlargement with sediment traps
- 3. Municipal costs and revenues. Cost estimates to the Town for the above services and the expected tax revenue of the development.
- O. Open Space Land Cession Offers. Written offers of cession to the municipality of all public open space shown on the plan, and copies of agreements, or other documents showing the manner in which space(s), Code to which is reserved by the subdivider, are to be maintained.
- P. Open Space Land Cession Offers Acknowledgement by Town. Written evidence that the municipal officers are satisfied with the legal sufficiency of the documents referred to in subsection (C)(2)(a) of this section. Such written evidence does not constitute an acceptance by the municipality of any public open space referred to in subsection (C)(2)(a) of this section.
- Q. Performance Guaranty and Town Acceptance to secure completion of all improvements required by the Planning Board and written evidence the Town manager is satisfied with the sufficiency of such guaranty.
- 1. Where improvements for the common use of lessees or the general public have been approved, the Planning Board must require a performance guaranty of amount sufficient to pay for said improvements as a part of the agreement.
- 2. Process. Prior to the issue of a building permit, the applicant must, in an amount and form acceptable to the Town manager, file with the municipal treasurer an instrument to cover the full cost of the required improvements. A period of one year (or such other period as the Planning Board may determine appropriate, not to exceed three years) is the guaranty time within which required improvements must be completed. The performance guaranty must include an amount required for recreation land or improvements as specified.

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- R. Maintenance Plan and Agreement defining maintenance responsibilities, responsible parties, shared costs, and schedule. Where applicable, a Maintenance Agreement must be included in the Document of Covenants, Homeowners Documents and/or as riders to the individual deed.
- S. Phasing Plan. Where, upon applicant's request, the Planning Board may permit phasing of the plans where it can be demonstrated to the Planning Board's satisfaction that such phasing would result in a safe and orderly development of the plan.
- 1. The applicant may file a section of the approved plan with the municipal officials and the York County registry of deeds if said section constitutes at least twenty-five percent (25%) of the total number of lots, or for plans including buildings, twenty-five percent (25%) of the gross area, contained in the approved plan. In all circumstances, plan approval of the remaining sections of the plan will remain in effect for three years unless the applicant requests and the Planning Board grants extensions of time equivalent to the requirements for approved plans in Section 16.36.050(E).
- 2. Phasing is subject to any conditions deemed necessary to assure a reasonable mixture of uses is completed within each separate phase of the plan.
- 3. Where projects are to be constructed in phases, phasing of stormwater management, water mains and streets are part of the review process.
- 4. Portions of both the developed and undeveloped site, impacted by interim infrastructure conditions such as un-looped water systems, stormwater runoff from unfinished areas onto finished areas and vice versa, dead end streets, etc., must be clearly defined and shown on the plans.
- 5. The Planning Board may permit construction of phases "out of order" only when the storm drainage plan and the water plan, etc. have been reviewed and it has been demonstrated that the impact on both the developed and undeveloped sections is negligible.
- T. Right-of-Way Plan.
- 1. A completed application for a Planning Board approved right-of-way must include the requirements of Section 16.36.060 with the following modifications:
- a. The following submission requirements are not necessary for Right-of-Way review: subsections (B)(2)(I), (m), (p), (r)—(w) and (z); (B)(3)(c)—(h); (B)(4); and (B)(5) of this section.
- Subsection (B)(2) of this section modified so floor plans and elevations of principal structures are not required;
- Include the size of the parcel minus the area in the ROW, and the street frontage excluding the ROW;
- d. Only need to show and locate on the plan the names and addresses of all owners of record of contiguous property, including those across a street;
- e. Include required front yards from the R.O.W. on the plan.

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neil@civcon.com

From: outlook_5860CF8220E6E208@outlook.com <asiegel@maine.rr.com>

Sent: Tuesday, March 8, 2022 10:34 AM

To: neil@civcon.com
Subject: When Pigs Fly

"To whom it may concern

I authorize Civil Consultants to represent Christopher Hagan for Federal, State and Local permit applications pertaining to the proposed work at Town of Kittery Tax Map 61 Lot 27A, 460 US Route 1, Kittery, ME.

Andrew Siegel
Pig Pen Partners "

Sent from Mail for Windows



Google Ea Kittery,	•	PREPARED 40 Brick FOR:	ig Pen Partners yard Court, York, ME 03909 tion: 460 U.S. Route 1, E 03909
JOB NO: 09-185.04 NTS		DATE: January 2022	

 $\label{lem:components} J:\ aaa\ 2021\ 2123200\ PLANNING\ BOARD\ COMPONENTS\ Google Image.doc$



Doc# 2010044264 Bk 15952 Ps 341 - 344 Received York SS 10/04/2010 9:41AM Debra L. Anderson Resister of Deeds

WARRANTY DEED

KNOW ALL PERSONS BY THESE PRESENTS, That I, ANN E. BLAKE, Successor Trustee of Trust A of The HAROLD L. DURGIN FAMILY TRUST, and ANN E. BLAKE, Trustee of Trust B of The HAROLD L. DURGIN FAMILY TRUST, a trust created and existing under the laws of the State of Maine by a certain Revocable Trust Agreement dated February 25, 1993, for consideration paid, grant to PIG PEN PARTNERS, a Maine general partnership with a place of business at York, County of York and State of Maine, and whose mailing address is 40 Brickyard Court, York, Maine 03909, with WARRANTY COVENANTS, a certain lot or parcel of land, together with the buildings and improvements located thereon, situated at 460 U.S. Route 1, in the Town of Kittery, County of York and State of Maine, and being more specifically bounded and described as follows:

See attached EXHIBIT A for a more specific description of the premises herein conveyed, which description is hereby incorporated herein by this reference.

The premises described on **EXHIBIT A** are shown as "PROPOSED LOT "A"" on a certain plan entitled "**DIVISION OF LAND** (TAX MAP 61, LOT 27) FOR **TRUST A & TRUST B** OF **THE HAROLD L. DURGIN FAMILY TRUST (HAROLD L. DURGIN & ANN E. BLAKE, TRUSTEES)** 450 U.S. ROUTE 1 KITTERY, MAINE", dated May 23, 2006, by Doucet Survey, Inc., and recorded in the York County Registry of Deeds at Plan Book 311, Page 46.

The premises described on EXHIBIT A are conveyed subject to the following easements:

- 1. Easement from Harold L. Durgin and Sarah S. Durgin to the State of Maine as described in the fourth paragraph of the deed dated March 12, 1952 and recorded in the York County Registry of Deeds at Book 1204, Page 323; and,
- Warranty Easement Deed from Ann E. Blake, successor Trustee of Trust A of the Harold L. Durgin Family Trust dated February 25, 1993, and Ann E. Blake, Trustee of Trust B of the Harold L. Durgin Family Trust dated February 25, 1993 to Kittery Commons, LLC dated June 9, 2006 and recorded in the York County Registry of Deeds at Book 14863, Page 0434.

Meaning and intending to convey and hereby conveying the remaining portion of the premises conveyed by (1) corrective quitclaim deed with covenants dated January 12, 2000 from Harold L. Durgin, Trustee of The Harold L. Durgin Family Trust Dated February 25, 1993 to Harold L. Durgin, Trustee of Trust A of The Harold L. Durgin Family Trust Dated February 25, 1993, and recorded in the York County Registry of Deeds at Book 10050, Page 256, and (2) corrective quitclaim deed with covenants dated January 12, 2000 from Harold L.Durgin, Trustee of The Harold L. Durgin Family Trust Dated February 25, 1993 to Ann E. Blake as Trustee of Trust B of The Harold L. Durgin Family Trust Dated February 25, 1993, and recorded in the York County Registry of Deeds at Book 10050, Page 259, and (3) by release deed dated April 19, 2000 from Flagship Management, Inc. to Ann E. Blake, Successor Trustee of Trust A of The Harold L. Durgin Family Trust Dated February 25, 1993 and Ann E. Blake, Trustee of Trust B of The Harold L. Durgin Family Trust Dated February 25, 1993 and recorded in the York County Registry of Deeds at Book 10279, Page 61. Pursuant to Article II of The Harold L. Durgin Family Trust Dated February 25, 1993, said Ann E. Blake became the Successor Trustee of Trust A of The Harold L. Durgin Family Trust upon the death of the surviving trustee Harold L. Durgin. Harold L. Durgin died on February 2, 2001.

IN WITNESS WHEREOF, the said Ann E. Blake, in her capacity as Successor Trustee of Trust A and Trustee of Trust B of The Harold L. Durgin Family Trust Dated February 25, 1993, has caused this instrument to be executed this 29¹⁴ day of September, 2010.

> TRUST A of The HAROLD L. DURGIN FAMILY TRUST Dated February 25, 1993

TRUST B of The HAROLD L. DURGIN FAMILY TRUST Dated February 25, 1993

STATE OF MAINE

COUNTY OF YORK

September 29, 2010

Personally appeared the above-named Ann E. Blake, Successor Trustee of Trust A and Trustee of Trust B of The Harold L. Durgin Family Trust Dated February 25, 1993 and acknowledged the foregoing instrument to be her voluntary act and deed in her said capacities, and the voluntary act and deed of said Trust A and Trust B of The Harold L. Durgin Family Trust Dated February 25, 1993.

Before me,

Notary Public

Print Name:

Lynn A. Keisker

Notary Public

My Commission Expires 05-14-2017

Instrument prepared by: DEAN K. BOUFFARD, P.C. 74 State Road, Suite 205 Post Office Box 30 Kittery, Maine 03904-0030 (207) 439-6377

SEAL

EXHIBIT A

A certain lot or parcel of land, together with the buildings and improvements located thereon, situated on U.S. Route 1 and known as 460 U.S. Route 1, in Kittery, County of York and State of Maine, and being more specifically bounded and described as follows:

Beginning at a 5/8" rebar w/ID cap set in the easterly right-of-way line of Lewis Road and the southerly right-of-way line of U.S. Route 1 and thence proceeding North 66° 51' 32" East a distance of 1155.28 feet along the southerly right-of-way line of said U.S. Route 1 to a 2" iron pipe set at the end of a stone wall and land now or formerly of Donald C. and Barbara J. West;

Thence turning and proceeding along said stone wall the following courses and distances:

South 07° 39' 52" East a distance of 231.97 feet to an iron pipe in said stone wall; South 07° 01' 24" East a distance of 17.66 feet to a fence post with barbed wire found in said stone wall:

South 15° 55' 22" East a distance of 119.30 feet to a 2" iron pipe is said stone wall; and,

South 15° 38' 55" East a distance of 45.23 feet to a point;

Thence turning and proceeding South 43° 00' 00" West a distance of 135.35 feet along land now or formerly of Kittery Commons, LLC to a 5/8" rebar w/ID cap;

Thence turning and proceeding South 71° 06' 08" West a distance of 519.91 feet along said land now or formerly of Kittery Commons, LLC to a 5/8" rebar w/ID cap;

Thence proceeding South 64° 57' 32" West a distance of 407.72 feet still along said land now or formerly of Kittery Commons, LLC to a point in the easterly right-of-way line of said Lewis Road;

Thence turning and proceeding North 31° 20' 43" West a distance of 438.00 feet along the easterly right-of-way line of said Lewis Road to a 5/8" rebar w/ID cap set in the easterly right-of-way line of Lewis Road and the southerly right-of-way line of U.S. Route 1, and being the place of beginning.

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Warranty Easement Deed

Ann E. Blake, successor Trustee of Trust A of the Harold L. Durgin Family Trust dated February 25, 1993, and Ann E. Blake, Trustee of Trust B of the Harold L. Durgin Family Trust dated February 25, 1993 (hereafter collectively referred to as "Grantor", which expression shall include their successors and assigns forever), owner of a certain lot or parcel of land situated at the intersection of US Route 1 and Lewis Road in Kittery, Cumberland County, Maine, evidenced by certain deeds dated (a) January 12, 2000 recorded in the York County Registry of Deeds in Book 10050, Page 256; (b) January 12, 2000 recorded in said Registry in Book 10050, Page 259; (c) April 19, 2000 recorded in said Registry in Book 10279, Page 61, for good and valuable consideration, hereby grants to Kittery Commons, LLC, a Maine Limited Liability Company with a place of business in Scarborough, Cumberland County, Maine (hereinafter referred to as "Grantee", which expression shall include its successors and assigns forever), a certain exclusive and perpetual easement, running with the land, for the purposes more particularly described hereinafter. That portion of Grantor's aforesaid real estate which is subject to the easement rights herein granted is more particularly bounded and described as follows:

To reach the point of beginning for the easement described herein, commence at an iron rod marking the intersection of the southerly right-of-way line of US Route 1 and the easterly right-of-way line of Lewis Road in Kittery, York County, Maine, and thence running North 66° 51' 32" East along said southerly right-of-way line of US Route 1, 44.68 feet to a 6" square Maine Highway Monument, thence continuing North 66° 51' 32" East along said southerly right-of-way line of US Route 1 a distance of 425.32 feet to an iron rod with cap set flush, and then continuing North 66° 51' 32" East along said southerly right-of-way line of US Route 1 a distance of 111.04 feet to the point of beginning for the easement herein described and thereafter by the following bearing courses and distance:

South 05° 13' 26" East a distance of 83.52 feet to a point;

South 83° 09' 37" East a distance of 177.71 feet to a point;

South 38° 10' 22" East a distance of 220.59 feet to a point;

South 55° 28' 39" East a distance of 75.36 feet to a point on the northerly line of land to be conveyed to Kittery Commons, LLC by deed of near or even date herewith to be recorded in the York County Registry of Deeds;

South 71° 06' 08" West along a portion of the northerly line of said Kittery Commons, LLC land a distance of 49.81 feet to a point;

North 55° 28' 39" West a distance of 51.76 feet to a point;

North 38° 10' 22" West a distance of 210.11 feet to a point;

North 83° 09' 37" West a distance of 193.50 feet to a point;

North 05° 13' 26" West a distance of 102.94 feet to a point on the southerly right-of-way line of US Route 1;

North 66° 51' 32" East along said southerly right-of-way line of US Route 1 a distance of 42.04 feet to the point of beginning for the easement herein described.

The express purposes for which Grantee may utilize the above described portion of Grantor's land (hereinafter referred to as the "Easement Corridor") are as follows:

- A. Installation, inspection, examination, testing, maintenance, replacement, repair, removal, upgrade and restoration of potable water lines and related equipment, subsurface sewer lines and related equipment, natural gas lines and related equipment, and any and all other like utilities and/or services and equipment;
- B. Right to remove vegetation, materials, and any other matter determined by Grantee to be adverse to the performance or condition of the services to be located within the Easement Corridor as noted above, subject to those terms and provisions hereinafter set forth; and
- C. Right to access the Easement Corridor at all times and without notice for all purposes hereinabove stated in this Warranty Easement Deed.

Grantee hereby expressly warrants and covenants to Grantor that all work to be performed by Grantee pursuant to the rights granted herein shall be so performed in a good and workmanlike manner and shall be at sole expense of Grantee. Grantee further warrants and covenants to Grantor that during any installation of utilities permitted hereunder, should Grantee's permitted activity during installation extend beyond the Easement Corridor, said activity shall be temporarily permitted by Grantor provided Grantee shall at its own cost and expense restore Grantor's surrounding real estate outside the Easement Corridor to a condition at least equal in quality to that which existed prior to the commencement of the work. Grantee further warrants and covenants to Grantor that Grantee shall not utilize the Easement Corridor or any portion thereof for any purpose not specifically set forth herein. Grantee further warrants and covenants to Grantor that any damage to Grantor's real estate resulting from any malfunction of those permitted facilities installed within the Easement Corridor by Grantee shall be restored by Grantee at Grantee's sole cost and expense.

Grantor hereby warrants and covenants to Grantee that Grantor shall, neither directly nor indirectly, construct, erect, place, store or otherwise locate within the Easement Corridor any buildings, structures, equipment, inventory, goods, vehicles of any type or any other like structure or property, either permanently or temporarily, so as to prevent any damage, destruction, impediment, interference or other negative impact upon those facilities installed by Grantee within the Easement Corridor. Grantor further warrants and covenants to Grantee that in the event Grantor engages in any activity on Grantor's

retained land which damages, destroys, impedes, restricts or otherwise negatively affects any of the utility services installed by Grantee within the Easement Corridor, Grantor shall, at its sole cost and expense, repair, replace, restore and otherwise remedy said damage to Grantee's reasonable satisfaction. Grantor further warrants and covenants to Grantee that in the event any excavation activity is performed on Grantor's retained land, Grantor shall abide by all Federal, State and Local rules and regulations relating to such excavation in order to preserve and protect the integrity of the facilities contained within the Easement Corridor, including but not limited to compliance with the "Dig-Safe" requirements in effect at the time of such excavation.

Notwithstanding the foregoing two (2) paragraphs, to the extent that the Easement Corridor extends over the existing driveway to the property, Grantee expressly warrants that all work shall be performed in a good and workmanlike manner and that the driveway shall be restored following such work to a condition at least equal in quality to that which existed prior to commencement of the work. Grantee further warrants that all work affecting the driveway portion of the Easement Corridor shall be completed expeditiously and with the minimum loss of use or disruption to Grantor.

In Witness Whereof, the Grantors have set their hands and seals on this 9th day of June, 2006.

Trust A of the Harold L. Durgin Family Trust dated February 25, 1993

Witness

Ann E. Blake, Successor Trustee

Trust B of the Harold L. Durgin Family Trust dated February 25, 1993

Car & Blake Frustre

\V/itmoss

Ann L. Diake, Trustee

Kittery Commons, LLC

\V/:

JOSEPH F. A

June 9, 2006

Then personally appeared the above-named Ann E. Blake, Trustee of Trust A of the Harold L. Durgin Family Trust dated February 25, 1993, in his/her said capacity, and the free act and deed of said Trust A of the Harold L. Durgin Family Trust dated February 25, 1993. My Commission Expires: Fullofiute State of Maine York, SS. June 9, 2006 Then personally appeared the above-named Ann E. Blake, successor Trustee of Trust B of the Harold L. Durgin Family Trust dated February 25, 1993 in his/her said capacity, and the free act and deed of said Trust B of the Harold L. Durgin Family Trust dated February 25, 1993. Before me, Notary Public Dean K. Boulfand, Astro My Commission Expires: Indefinite State of Maine York, SS. June 9, 2006 Then personally appeared the above-named Joseph F. How, duly authorized Nemec, of Kittery Commons, LLC and acknowledged the foregoing instrument to be his/her free act and deed, in his/her said capacity, and the free act and deed of said Kittery Commons, LLC. Before me,

odh: H:\CLIENTS\Kittery Commons, LLC\US Route 1 - Lewis Road\Warranty Fasement Deed

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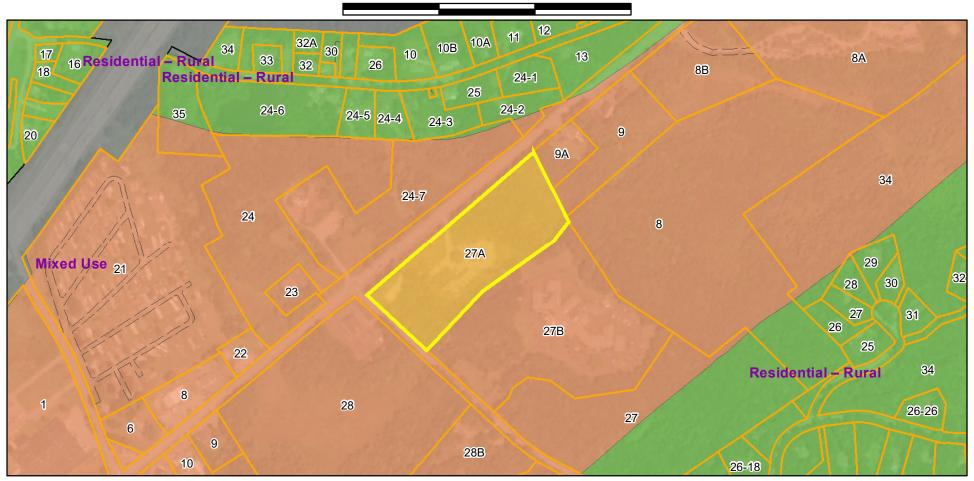
Kittery, ME

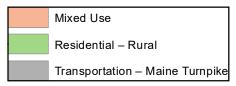


February 16, 2022

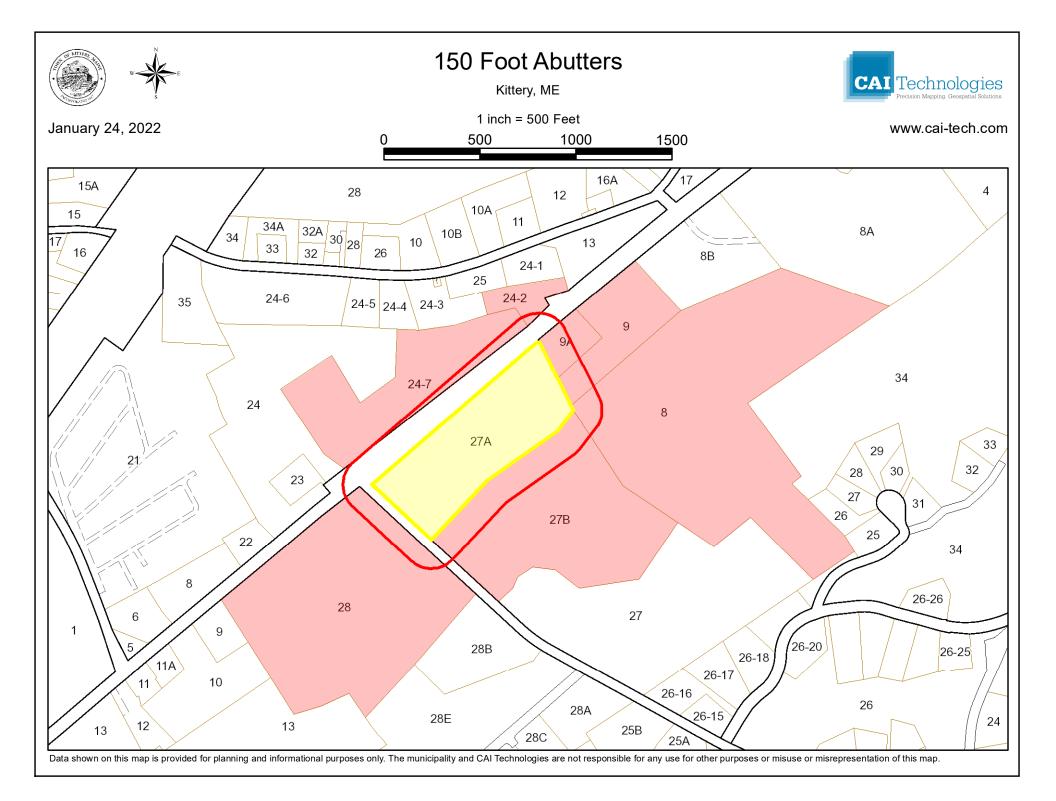
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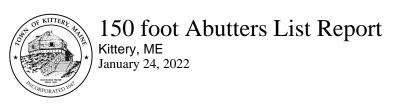
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Subject Property:

Parcel Number: 61-27A CAMA Number: 61-27A

Property Address: 460 US ROUTE 1

Mailing Address: PIG PEN PARTNERS

40 BRICKYARD COURT

YORK. ME 03909

Abutters:

Parcel Number: 60-24-2

CAMA Number: 60-24-2

Property Address: 9 ADAMS ROAD

Parcel Number: 60-24-7

CAMA Number: 60-24-7

Property Address: US ROUTE 1

Parcel Number: 61-27B

CAMA Number: 61-27B Property Address: 9 LEWIS ROAD

Parcel Number: 61-28

CAMA Number: 61-28

Property Address: 450 US ROUTE 1

Parcel Number: 67-8

CAMA Number: 67-8

Property Address: US ROUTE 1

Parcel Number: 67-9

CAMA Number: 67-9

Property Address: 484 US ROUTE 1

Parcel Number: 67-9A

CAMA Number:

1/24/2022

Property Address: 480 US ROUTE 1

67-9A

Mailing Address: MOST FANTASTIC YOUNG FAMILY REV

TRUST OF 2021 YOUNG, KATHERINE S

& JOSEPH L 9 ADAMS ROAD KITTERY, ME 03904

Mailing Address: JAI SHREE DURGA LAXMI LLC

736 SOUTHBRIDGE STREET

AUBURN, MA 01501

Mailing Address: KITTERY COMMONS LLC

250 GODDARD ROAD LEWISTON, ME 04240

Mailing Address: KITTERYDOM, LLC

100 CONIFER HILL DRIVE SUITE #402

DANVERS, MA 01923

Mailing Address: COBALT PROPERTIES LLC

PO BOX 868

CALAIS, ME 04619

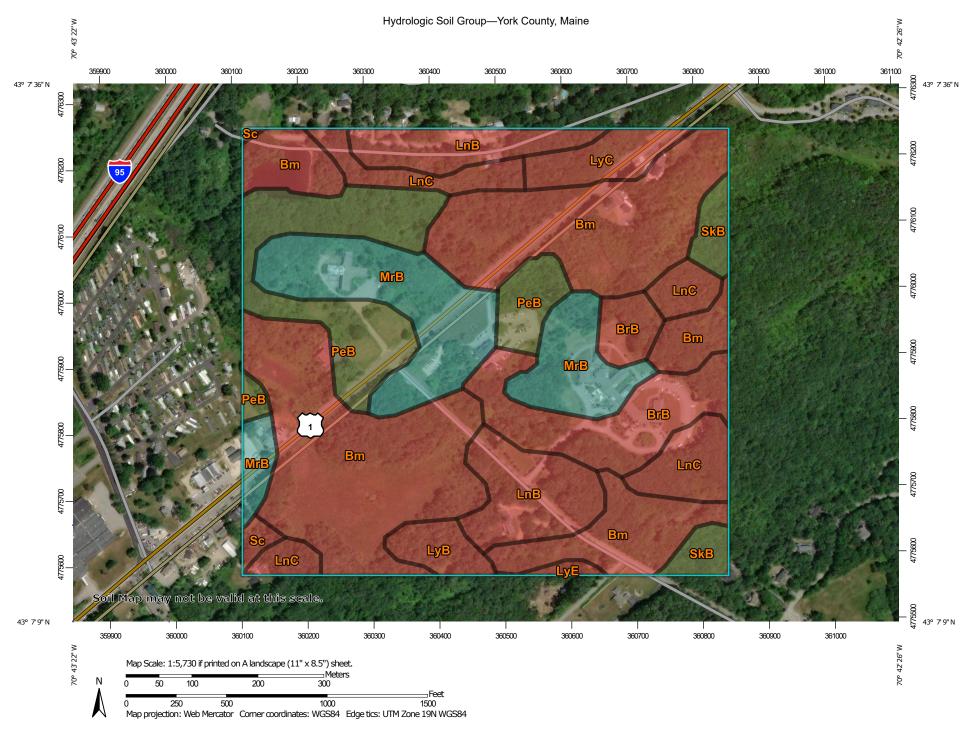
Mailing Address: CAPE HOUSE MANAGEMENT LLC

484 US ROUTE 1 KITTERY, ME 03904

PORTSMOUTH HOLDINGS, LLC Mailing Address:

PO BOX 657

PORTSMOUTH, NH 03802



MAP LEGEND MAP INFORMATION The soil surveys that comprise your AOI were mapped at Area of Interest (AOI) С 1:20.000. Area of Interest (AOI) C/D Soils Warning: Soil Map may not be valid at this scale. D Soil Rating Polygons Enlargement of maps beyond the scale of mapping can cause Not rated or not available Α misunderstanding of the detail of mapping and accuracy of soil **Water Features** line placement. The maps do not show the small areas of A/D Streams and Canals contrasting soils that could have been shown at a more detailed Transportation B/D Rails ---Please rely on the bar scale on each map sheet for map measurements. Interstate Highways C/D Source of Map: Natural Resources Conservation Service **US Routes** Web Soil Survey URL: D Major Roads Coordinate System: Web Mercator (EPSG:3857) Not rated or not available -Local Roads Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts Soil Rating Lines Background distance and area. A projection that preserves area, such as the Aerial Photography Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below. Soil Survey Area: York County, Maine Survey Area Data: Version 20, Aug 31, 2021 Soil map units are labeled (as space allows) for map scales 1:50.000 or larger. Not rated or not available Date(s) aerial images were photographed: Dec 31, 2009—Sep 9. 2017 **Soil Rating Points** The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background A/D imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident. B/D

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
Bm	Biddeford mucky peat, 0 to 3 percent slopes	D	48.3	39.0%
BrB	Brayton and Westbury fine sandy loams, 0 to 8 percent slopes	D	11.1	8.9%
LnB	Lyman loam, 3 to 8 percent slopes, rocky	D	11.3	9.2%
LnC	Lyman loam, 8 to 15 percent slopes, rocky	D	9.4	7.6%
LyB	Lyman-Rock outcrop complex, 3 to 8 percent slopes	D	2.4	1.9%
LyC	Lyman-Rock outcrop complex, 8 to 15 percent slopes	D	3.5	2.8%
LyE	Lyman-Rock outcrop complex, 15 to 80 percent slopes	D	0.5	0.4%
MrB	Marlow fine sandy loam, 3 to 8 percent slopes	С	18.9	15.3%
PeB	Peru fine sandy loam, 3 to 8 percent slopes	C/D	14.5	11.7%
Sc	Scantic silt loam, 0 to 3 percent slopes	D	0.8	0.6%
SkB	Skerry fine sandy loam, 0 to 8 percent slopes	C/D	3.1	2.5%
Totals for Area of Inter	rest	123.8	100.0%	

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher





NATURAL RESOURCES

Kittery, ME

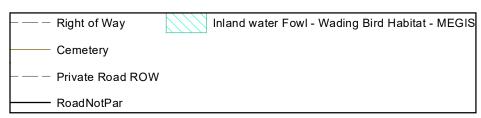


February 16, 2022

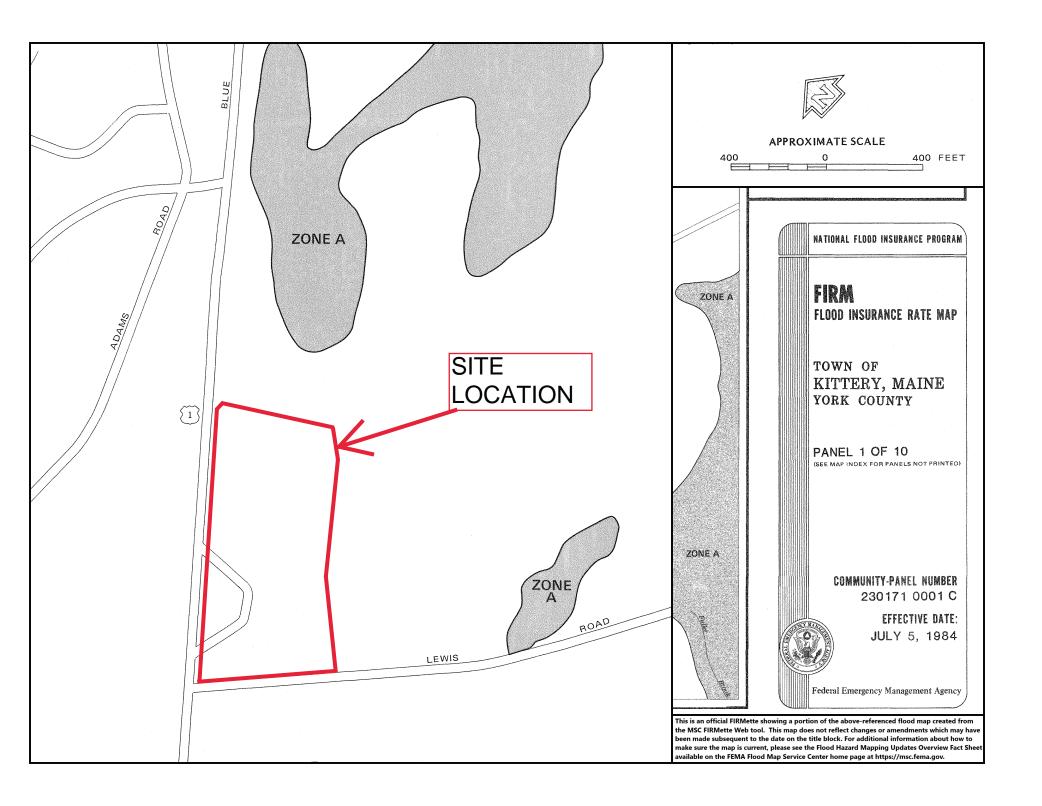
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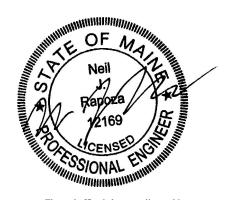
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The seal affixed above applies to this report, Appendices A through F (*from previous submission*), and Pre- & Post-Development Drainage Plans D1-D2.

Town of Kittery Site Plan Application Stormwater Management Plan

"WHEN PIGS FLY" 460 ROUTE 1, KITTERY, ME

Prepared for

Andrew Siegel 40 Brickyard Court York, ME 03909

October 2010 (*Revised March 2022*)



CIVIL CONSULTANTS

Engineers

Planners

Surveyors

Stormwater Management Narrative

TOWN OF KITTERY STORMWATER MANAGEMENT PLAN

"WHEN PIGS FLY PIZZERIA" 460 US Route 1 Kittery, Maine

Prepared for:

Andrew Siegel 40 Brickyard Ct York, ME 03909

October 2010
Revised March 2022

PROJECT DESCRIPTION:

The proposed commercial development is located on Map 61, Lot 27A, which is situated on Route 1 in Kittery, ME. The project will consist of a new restaurant and food retail store, installation of new associated parking, and demolition of an existing building. Wherever possible, the intention is to re-use existing pavement and building foundations.

Of the site's 11.1 acres, approximately 2.2 acres will be considered developed upon completion of the proposed site improvements. This includes areas of buildings, driveway, parking, and drainage facilities, along with associated utilities, improved existing developed areas, and general site grading.

It is proposed that a 10,000 sf area will also be designated for outdoor dining. This area includes a 180 sf

outdoor bar and associated stonedust/hardpack access path.

Of the disturbed area, approximately 0.86 0.89 acres is proposed new impervious (parking, drive, and roofs). The existing impervious cover on the site is approximately 0.42 acres (prior to 1985).

Much of the new development will consist of clearing vegetation and re-grading the areas adjacent to the proposed expanded building to allow for new parking. It also includes the disturbance necessary for the installation of the proposed level spreaders and other stormwater conveyance systems, as well as various utilities.

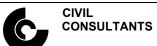
Control of peak stormwater flows from the site will be achieved through the use of a 40 ft stone bermed level lip spreader outletting to a grassed receiving area and a 100 ft stone bermed level lip spreader outletting to a forested receiving area.

The outlet control structures, culverts and other BMP's used to treat developed areas have been designed to the standards presented in "Stormwater Management for Maine – Vol III BMPs Technical Design Manual, January 2006" Rev April 2007.

The project will not create more than 3 acres of developed area or 1 acre of impervious area. Therefore, it will require only a Permit by Rule from the Maine DEP. The PBR Notification will be filed upon approval this site plan application, prior to the start of work.

EXISTING DRAINAGE CONDITIONS:

The site features currently include a 2,353 sf single family residence, and a 1,070 sf shed. There is 14,708 sf of



existing impervious parking/driveway area. The site is located directly on US Route 1 and access is gained through two existing one-way entrances.

The site is bound on the west by Lewis Road, which drains to a wetland feeding Wilson Creek. The receiving wetland to the east drains to Johnson Brook.

The slopes on the site vary between 1% and as much as 14%. Slopes average approximately 6% over the majority of the site.

The project drains west to the Wilson Creek. Flows leaving the site to the east drain to Johnson Brook. These are not listed as Urban Impaired Streams nor do they flow to any bodies of water listed as Lakes Most at Risk from New Development. (See Appendix A for a copy of the applicable USGS map).

Soils in the watershed are hydrologic type C & D. Medium Intensity Soils Mapping for the site was obtained using the USGS Web Soil Survey.

A designated one hundred (100) year flood area is located adjacent to the site with flood location shown on the attached flood map. No development or disturbance is planned within this area (See Appendix E for a copy of the applicable FEMA map).

PROPOSED DRAINAGE:

The majority of the proposed development will flow over land to one of two stone berm level lip spreaders.

Runoff from the eastern portion of the site will be directed to a 40' level spreader that will flow to wetlands associated with Johnson Brook.

Runoff from the western portion of the site will be directed to a 100' level spreader that will flow to wetlands associated with Wilson Creek.

The level spreader has been sized to dissipate flow volume and velocity, as outlined in the Maine DEP BMP Technical Design Manual.

METHODOLOGY:

All runoff calculations were performed using methods based on USDA–SCS Technical Release No. 20 (also known as TR-20). The two-, ten-, and twenty-five-year, twenty-four-hour storm events (Type III rainfall distribution) were used for the site-specific analysis to determine pre- and post-development peak discharge rates and required stormwater conveyance systems.

Runoff curve numbers (CN) and times of concentration (Tc) were determined by the methods outlined in USDA-SCS Technical Release No. 55 (better known as TR-55). On site watershed areas were determined using one-foot contour data provided by previously compiled topography plans. The applicable USGS Quadrangles were used to determine the extent of off-site drainage areas.

The detailed analysis for this project was performed by computer utilizing "HYDROCAD." The computer printouts are attached. Analysis parameters were updated to align with current practices, including increasing the timeframe of the analysis and decreasing the increment of data points, in an effort to produce a more accurate evaluation of the runoff. This resulted in minor changes to both the pre- and post-development runoff and did not have an effect on

<u>sizing or capacity of the facilities in place.</u>

As previously noted, USGS Web Soil Survey was used to determine the hydraulic designation for on- and off-site areas and areas that contribute to the development. The general distribution of soils is shown on the attached drainage plans.

The attached Pre- and Post Development plans (D1 & D2) show subcatchment boundaries, hydraulic flow lines, existing and proposed roads, and drainage features and facilities. Land cover type boundaries used in the model for on-site areas are also shown on the plan (i.e. tree lines, wetlands, etc).

ANALYSIS:

The overall perimeter of the watershed remained the same for both Pre- and Post Development.

There were five subcatchments identified for the Pre-Development analysis and six subcatchments identified for Post Development analysis. The modified areas were used to evaluate stormwater management facilities and modifications to the site.

The eastern discharge point (identified as OUT 1 in the calculations) includes flow from the site that outlets east to Johnson Brook. A portion of this area includes the proposed outdoor bar and access path, along with the outdoor seating area which has been analyzed as having a surface area of 50-75% grass cover to account for the lawn damage due to patron foot traffic in the area.

The northwestern discharge point (identified as OUT 2 in the calculations) includes runoff which converges at the

culvert at the intersection of Lewis Road and US Route 1. Flow from this culvert feeds a wetland draining to Wilson Creek. A portion of this subcatchment also includes some outdoor dining area, and has been analyzed similarly to comparable areas that drain to OUT 1 (see above).

The western discharge point (identified as OUT 3 in the calculations) includes runoff which converges to the culvert at the Lewis Road crossing. Flow from this culvert feeds a wetland draining to Wilson Creek. A portion of this area was temporarily cleared to allow for overflow gravel parking. This has since been abandoned and allowed to return to natural land cover. This has been conservatively analyzed as 50-75% grass cover for this study.

The post-development discharge point captures the same flows as described above for the post analysis and are also labeled OUT 1, matching the pre designation.

For further details regarding subcatchment determination, refer to sheets D1 & D2 included in the appendix of this report.

RESULTS:

In order to meet the Town requirement that downstream waterways and abutters are not adversely affected, we have attempted to hold Post-Development flows at or below Pre-Development levels.

The following tables summarize the results of the analyses for the 2-, 10-, and 25-year events:

TWO-YEAR EVENT -

Discharge Poir	ıt		
Designation	Peak	Runoff(in cfs)	Change
Pre/Post	Pre	Post	(cfs)
			<u> </u>
OUT 1	7.6		-0.29
OUT 2	2.24	<u>=</u>	N/C
OUT 3	8.5	<u>8.51</u>	N/C

TEN-YEAR EVENT -

Discharge Poi	nt		
Designation	Peak Runc	off(in cfs)	Change
Pre/Post	Pre	Post	(cfs)
· <u></u>	•		
OUT 1	<u>15.00</u>	14.24	-0.76
OUT 2	3.79	3.79	N/C
OUT 3	1 <u>6.62</u>	1 <u>6.29</u>	- 0.3 2

TWENTY-FIVE YEAR EVENT -

Discharge Poin Designation		off(in cfs)	Change
Pre/Post	Pre	Post	(cfs)
OUT 1 OUT 2 OUT 3	20.00 4.80 22.15	18.89 4.80 21.55	-1.11 <u>N/C</u> -0.60

As shown in the tables, peak flows are maintained or reduced in all storm events. The reduction in flows in the larger events is expected to have a positive impact on the adjacent wetlands. The change in land cover due to outdoor dining and modified model parameters in the revised analysis and did not raise runoff rates beyond the previously existing flows in any storm event.

CONCLUSIONS:

The proposed development will utilize new stormwater BMP's to generally reduce or maintain Post-Development peak runoff rates to Pre-Development rates by increasing the time of concentration of the contributing flows.

It is our opinion that there will be no adverse downstream impacts as a result of this project.



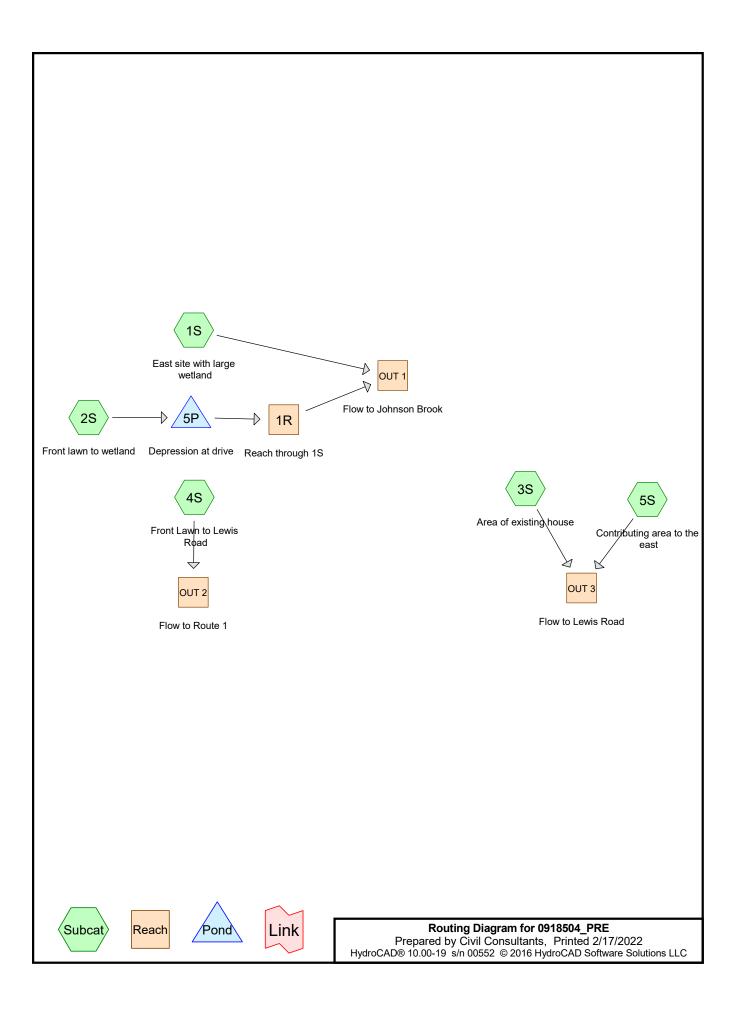
CIVIL CONSULTANTS

Engineers

Planners

Surveyors

Pre-Development Calculations



Area Listing (all nodes)

	Area	CN	Description
(a	acres)		(subcatchment-numbers)
	2.067	79	50-75% Grass cover, Fair, HSG C (1S, 3S, 4S)
	0.557	74	>75% Grass cover, Good, HSG C (2S)
	0.288	98	Paved parking, HSG C (2S, 4S)
	0.675	92	Paved roads w/open ditches, 50% imp, HSG C (2S, 3S, 4S)
	1.405	93	Paved roads w/open ditches, 50% imp, HSG D (1S, 2S, 3S, 5S)
	0.202	98	Roofs, HSG C (3S)
1	0.898	73	Woods, Fair, HSG C (1S, 3S, 5S)
	4.666	79	Woods, Fair, HSG D (1S, 3S, 5S)
2	20.758	78	TOTAL AREA

Printed 2/17/2022 Page 3

Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
0.000	HSG B	
14.687	HSG C	1S, 2S, 3S, 4S, 5S
6.071	HSG D	1S, 2S, 3S, 5S
0.000	Other	
20.758		TOTAL AREA

Prepared by Civil Consultants
HydroCAD® 10.00-19 s/n 00552 © 2016 HydroCAD Software Solutions LLC

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Page 4

Subcatc Numbers

Ground Covers (all nodes)

HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground
 (acres)	(acres)	(acres)	(acres)	(acres)	(acres)	Cover
0.000	0.000	2.067	0.000	0.000	2.067	50-75% Grass cover, Fair
0.000	0.000	0.557	0.000	0.000	0.557	>75% Grass cover, Good
0.000	0.000	0.288	0.000	0.000	0.288	Paved parking
0.000	0.000	0.675	1.405	0.000	2.080	Paved roads w/open ditches, 50%
						imp
0.000	0.000	0.202	0.000	0.000	0.202	Roofs
0.000	0.000	10.898	4.666	0.000	15.564	Woods, Fair
0.000	0.000	14.687	6.071	0.000	20.758	TOTAL AREA

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Time span=1.00-48.00 hrs, dt=0.01 hrs, 4701 points x 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: East site with largeRunoff Area=332,681 sf 3.67% Impervious Runoff Depth=1.11"
Flow Length=610' Tc=14.9 min CN=77 Runoff=7.26 cfs 0.708 af

Subcatchment 2S: Front lawn to wetlandRunoff Area=0.870 ac 27.07% Impervious Runoff Depth=1.43"
Flow Length=218' Tc=7.2 min CN=82 Runoff=1.39 cfs 0.103 af

Subcatchment 3S: Area of existing house Runoff Area=5.519 ac 6.83% Impervious Runoff Depth=1.11" Flow Length=600' Tc=23.8 min CN=77 Runoff=4.35 cfs 0.512 af

Subcatchment 4S: Front Lawn to Lewis

Runoff Area=1.248 ac 26.52% Impervious Runoff Depth=1.64"

Flow Length=425' Tc=8.0 min CN=85 Runoff=2.24 cfs 0.170 af

Subcatchment 5S: Contributing area to the Runoff Area=5.484 ac 5.59% Impervious Runoff Depth=1.11" Flow Length=783' Tc=25.6 min CN=77 Runoff=4.18 cfs 0.509 af

Reach 1R: Reach through 1SAvg. Flow Depth=0.04' Max Vel=0.23 fps Inflow=1.31 cfs 0.103 af n=0.050 L=760.0' S=0.0053 '/' Capacity=51.83 cfs Outflow=0.45 cfs 0.103 af

Reach OUT 1: Flow to Johnson Brook

Inflow=7.61 cfs 0.812 af
Outflow=7.61 cfs 0.812 af

Reach OUT 2: Flow to Route 1 Inflow=2.24 cfs 0.170 af
Outflow=2.24 cfs 0.170 af

Reach OUT 3: Flow to Lewis Road Inflow=8.51 cfs 1.020 af Outflow=8.51 cfs 1.020 af

Pond 5P: Depression at drive Peak Elev=58.20' Storage=99 cf Inflow=1.39 cfs 0.103 af 12.0" Round Culvert n=0.013 L=58.0' S=0.0603 '/' Outflow=1.31 cfs 0.103 af

Total Runoff Area = 20.758 ac Runoff Volume = 2.002 af Average Runoff Depth = 1.16" 92.63% Pervious = 19.228 ac 7.37% Impervious = 1.530 ac

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Time span=1.00-48.00 hrs, dt=0.01 hrs, 4701 points x 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: East site with largeRunoff Area=332,681 sf 3.67% Impervious Runoff Depth=2.11"
Flow Length=610' Tc=14.9 min CN=77 Runoff=14.23 cfs 1.340 af

Subcatchment 2S: Front lawn to wetlandRunoff Area=0.870 ac 27.07% Impervious Runoff Depth=2.52"
Flow Length=218' Tc=7.2 min CN=82 Runoff=2.47 cfs 0.183 af

Subcatchment 3S: Area of existing house Runoff Area=5.519 ac 6.83% Impervious Runoff Depth=2.11" Flow Length=600' Tc=23.8 min CN=77 Runoff=8.48 cfs 0.968 af

Subcatchment 4S: Front Lawn to Lewis

Runoff Area=1.248 ac 26.52% Impervious Runoff Depth=2.79"

Flow Length=425' Tc=8.0 min CN=85 Runoff=3.79 cfs 0.290 af

Subcatchment 5S: Contributing area to the Runoff Area=5.484 ac 5.59% Impervious Runoff Depth=2.11" Flow Length=783' Tc=25.6 min CN=77 Runoff=8.17 cfs 0.962 af

Reach 1R: Reach through 1SAvg. Flow Depth=0.06' Max Vel=0.30 fps Inflow=2.11 cfs 0.183 af n=0.050 L=760.0' S=0.0053 '/' Capacity=51.83 cfs Outflow=1.00 cfs 0.183 af

Reach OUT 1: Flow to Johnson Brook Inflow=15.00 cfs 1.523 af
Outflow=15.00 cfs 1.523 af

Reach OUT 2: Flow to Route 1 Inflow=3.79 cfs 0.290 af Outflow=3.79 cfs 0.290 af

Reach OUT 3: Flow to Lewis Road Inflow=16.62 cfs 1.931 af Outflow=16.62 cfs 1.931 af

Pond 5P: Depression at drive Peak Elev=58.50' Storage=286 cf Inflow=2.47 cfs 0.183 af 12.0" Round Culvert n=0.013 L=58.0' S=0.0603 '/' Outflow=2.11 cfs 0.183 af

Total Runoff Area = 20.758 ac Runoff Volume = 3.744 af Average Runoff Depth = 2.16" 92.63% Pervious = 19.228 ac 7.37% Impervious = 1.530 ac

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Time span=1.00-48.00 hrs, dt=0.01 hrs, 4701 points x 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: East site with largeRunoff Area=332,681 sf 3.67% Impervious Runoff Depth=2.79"
Flow Length=610' Tc=14.9 min CN=77 Runoff=18.97 cfs 1.776 af

Subcatchment 2S: Front lawn to wetlandRunoff Area=0.870 ac 27.07% Impervious Runoff Depth=3.26"
Flow Length=218' Tc=7.2 min CN=82 Runoff=3.18 cfs 0.236 af

Subcatchment 3S: Area of existing house Runoff Area=5.519 ac 6.83% Impervious Runoff Depth=2.79" Flow Length=600' Tc=23.8 min CN=77 Runoff=11.29 cfs 1.284 af

Subcatchment 4S: Front Lawn to Lewis

Runoff Area=1.248 ac 26.52% Impervious Runoff Depth=3.55"

Flow Length=425' Tc=8.0 min CN=85 Runoff=4.80 cfs 0.370 af

Subcatchment 5S: Contributing area to the Runoff Area=5.484 ac 5.59% Impervious Runoff Depth=2.79" Flow Length=783' Tc=25.6 min CN=77 Runoff=10.88 cfs 1.276 af

Reach 1R: Reach through 1SAvg. Flow Depth=0.07' Max Vel=0.34 fps Inflow=2.51 cfs 0.236 af n=0.050 L=760.0' S=0.0053 '/' Capacity=51.83 cfs Outflow=1.39 cfs 0.236 af

Reach OUT 1: Flow to Johnson Brook

Inflow=20.00 cfs 2.013 af
Outflow=20.00 cfs 2.013 af

Reach OUT 2: Flow to Route 1 Inflow=4.80 cfs 0.370 af Outflow=4.80 cfs 0.370 af

Reach OUT 3: Flow to Lewis Road Inflow=22.15 cfs 2.559 af Outflow=22.15 cfs 2.559 af

Pond 5P: Depression at drive Peak Elev=58.71' Storage=489 cf Inflow=3.18 cfs 0.236 af 12.0" Round Culvert n=0.013 L=58.0' S=0.0603 '/' Outflow=2.51 cfs 0.236 af

Total Runoff Area = 20.758 ac Runoff Volume = 4.941 af Average Runoff Depth = 2.86" 92.63% Pervious = 19.228 ac 7.37% Impervious = 1.530 ac

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Summary for Subcatchment 1S: East site with large wetland

Runoff = 18.97 cfs @ 12.20 hrs, Volume= 1.776 af, Depth= 2.79"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25 yr Rainfall=5.20"

	Α	rea (sf)	CN E	Description					
	1	13,178	79 V	79 Woods, Fair, HSG D					
	1	80,503	73 V	Voods, Fai	r, HSG C				
		24,400			•	itches, 50% imp, HSG D			
_		14,600	79 5	0-75% Gra	ass cover, F	Fair, HSG C			
	3	32,681	77 V	Veighted A	verage				
	3	20,481	_		vious Area				
		12,200	3	3.67% Impe	ervious Area	a			
	_		01			B 1.0			
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	8.1	100	0.0900	0.21		Sheet Flow, 1.1			
						Grass: Dense n= 0.240 P2= 3.00"			
	0.4	45	0.0600	1.71		Shallow Concentrated Flow, 1.2			
						Short Grass Pasture Kv= 7.0 fps			
	1.6	125	0.0720	1.34		Shallow Concentrated Flow, 1.3			
						Woodland Kv= 5.0 fps			
	4.8	340	0.0150	1.19	22.36	Trap/Vee/Rect Channel Flow, 1.4			
						Bot.W=50.00' D=0.25' Z= 100.0 '/' Top.W=100.00'			
_						n= 0.050 Scattered brush, heavy weeds			
	14.9	610	Total						

Summary for Subcatchment 2S: Front lawn to wetland

Runoff = 3.18 cfs @ 12.10 hrs, Volume= 0.236 af, Depth= 3.26"

A	rea (ac)	CN	Description
	0.158	98	Paved parking, HSG C
	0.120	92	Paved roads w/open ditches, 50% imp, HSG C
	0.557	74	>75% Grass cover, Good, HSG C
	0.035	93	Paved roads w/open ditches, 50% imp, HSG D
	0.870	82	Weighted Average
	0.634		72.93% Pervious Area
	0.235		27.07% Impervious Area

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	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	4.9	50	0.0310	0.17		Sheet Flow, 2.1
						Grass: Short n= 0.150 P2= 3.00"
	2.3	168	0.0300	1.21		Shallow Concentrated Flow, 2.2
_						Short Grass Pasture Kv= 7.0 fps
	7.2	218	Total			

Summary for Subcatchment 3S: Area of existing house

Runoff = 11.29 cfs @ 12.34 hrs, Volume= 1.284 af, Depth= 2.79"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25 yr Rainfall=5.20"

Area	(ac) C	N Des	cription					
3.	3.516 73 Woods, Fair, HSG C							
0.435 79 Woods, Fair, HSG D								
0.	153	92 Pave	ed roads w	/open ditch	ies, 50% imp, HSG C			
0.	197	93 Pave	ed roads w	/open ditch	es, 50% imp, HSG D			
0.	202	98 Roo	fs, HSG C					
1.	016	79 50-7	5% Grass	cover, Fair	, HSG C			
5.	519	77 Wei	ghted Aver	age				
	142	93.1	7% Pervio	us Area				
0.	377	6.83	% Impervi	ous Area				
_								
Tc	Length	Slope	Velocity		Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
16.2	75	0.0247	0.08		Sheet Flow, 3.1			
					Woods: Light underbrush n= 0.400 P2= 3.00"			
3.7	170	0.0240	0.77		Shallow Concentrated Flow, 3.2			
0.0	400	0.0440	4.05		Woodland Kv= 5.0 fps			
2.9	180	0.0440	1.05		Shallow Concentrated Flow, 3.3			
4.0	475	0.0450	2.07	00.00	Woodland Kv= 5.0 fps			
1.0	175	0.0150	2.97	88.26	Trap/Vee/Rect Channel Flow, 3.5			
					Bot.W=50.00' D=0.50' Z= 8.0 & 30.0 '/' Top.W=69.00'			
	000	T.4.1			n= 0.035 Scattered brush, heavy weeds			
23.8	600	Total						

Summary for Subcatchment 4S: Front Lawn to Lewis Road

Runoff = 4.80 cfs @ 12.11 hrs, Volume= 0.370 af, Depth= 3.55"

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		<i>(</i>) 0						
_	Area	(ac) C	N Des	cription				
	0.402 92 Paved roads w/open ditches, 50% imp, HSG C							
	0.	130	98 Pave	ed parking	. HSG C			
					cover, Fair	· HSG C		
_				ghted Aver		, 1.00 0		
			,					
	_	917		8% Pervio				
	0.	331	26.5	2% Imper\	/ious Area			
	Tc	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•		
_	4.9	50	0.0310	0.17	, ,	Sheet Flow, 4.1		
		00	0.00.0	0		Grass: Short n= 0.150 P2= 3.00"		
	2.1	155	0.0310	1.23		Shallow Concentrated Flow, 4.2		
	۷.۱	133	0.0310	1.23		•		
	4.0	000		0.50	0.00	Short Grass Pasture Kv= 7.0 fps		
	1.0	220	0.0270	3.52	2.86	Trap/Vee/Rect Channel Flow, 4.3		
						Bot.W=2.00' D=0.25' Z= 5.0 '/' Top.W=4.50'		
						n= 0.022 Earth, clean & straight		
	8.0	425	Total			<u> </u>		

Summary for Subcatchment 5S: Contributing area to the east

10.88 cfs @ 12.37 hrs, Volume= 1.276 af, Depth= 2.79" Runoff

_	Area	(ac) C	N Desc	cription		
	3.	238 7	'3 Woo	ds, Fair, F	ISG C	
	1.			ods, Fair, F		
_	0.	613 9	3 Pave	ed roads w	/open ditch	nes, 50% imp, HSG D
		_	,	ghted Aver	•	
		178	_	1% Pervio		
	0.	306	5.59	% Impervi	ous Area	
	Тс	Longth	Slope	Volocity	Congoity	Description
		Length	Slope (ft/ft)	Velocity (ft/sec)	Capacity	Description
_	(min)	(feet)			(cfs)	01 (51 04
	18.9	100	0.0300	0.09		Sheet Flow, 3.1
						Woods: Light underbrush n= 0.400 P2= 3.00"
	3.1	145	0.0240	0.77		Shallow Concentrated Flow, 3.2
						Woodland Kv= 5.0 fps
	0.2	30	0.0330	2.92		Shallow Concentrated Flow, 3.3
						Unpaved Kv= 16.1 fps
	1.8	142	0.0700	1.32		Shallow Concentrated Flow, 3.4
						Woodland Kv= 5.0 fps
	1.6	366	0.0164	3.92	10.29	Trap/Vee/Rect Channel Flow, 3.5
						Bot.W=2.00' D=0.50' Z= 3.0 & 10.0 '/' Top.W=8.50'
_						n= 0.022 Earth, clean & straight
	25.6	783	Total			

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Summary for Reach 1R: Reach through 1S

Inflow Area = 0.870 ac, 27.07% Impervious, Inflow Depth = 3.26" for 25 yr event

Inflow = 2.51 cfs @ 12.17 hrs, Volume= 0.236 af

Outflow = 1.39 cfs @ 12.40 hrs, Volume= 0.236 af, Atten= 45%, Lag= 14.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs / 3

Max. Velocity= 0.34 fps, Min. Travel Time= 37.0 min Avg. Velocity = 0.09 fps, Avg. Travel Time= 138.2 min

Peak Storage= 3,086 cf @ 12.40 hrs Average Depth at Peak Storage= 0.07'

Bank-Full Depth= 0.50' Flow Area= 50.0 sf, Capacity= 51.83 cfs

50.00' x 0.50' deep channel, n= 0.050 Scattered brush, heavy weeds

Side Slope Z-value= 100.0 '/' Top Width= 150.00'

Length= 760.0' Slope= 0.0053 '/'

Inlet Invert= 48.00', Outlet Invert= 44.00'



Summary for Reach OUT 1: Flow to Johnson Brook

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 8.507 ac, 6.06% Impervious, Inflow Depth = 2.84" for 25 yr event

Inflow = 20.00 cfs @ 12.21 hrs, Volume= 2.013 af

Outflow = 20.00 cfs @ 12.21 hrs, Volume= 2.013 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs / 3

Summary for Reach OUT 2: Flow to Route 1

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 1.248 ac, 26.52% Impervious, Inflow Depth = 3.55" for 25 yr event

Inflow = 4.80 cfs @ 12.11 hrs, Volume= 0.370 af

Outflow = 4.80 cfs @ 12.11 hrs, Volume= 0.370 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs / 3

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Summary for Reach OUT 3: Flow to Lewis Road

[40] Hint: Not Described (Outflow=Inflow)

11.003 ac, 6.21% Impervious, Inflow Depth = 2.79" for 25 yr event Inflow Area =

Inflow 22.15 cfs @ 12.35 hrs, Volume= 2.559 af

Outflow 2.559 af, Atten= 0%, Lag= 0.0 min 22.15 cfs @ 12.35 hrs, Volume=

Routing by Dyn-Stor-Ind method, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs / 3

Summary for Pond 5P: Depression at drive

Inflow Area = 0.870 ac, 27.07% Impervious, Inflow Depth = 3.26" for 25 yr event

Inflow 3.18 cfs @ 12.10 hrs, Volume= 0.236 af

Outflow 2.51 cfs @ 12.17 hrs, Volume= 0.236 af, Atten= 21%, Lag= 4.1 min

Primary = 2.51 cfs @ 12.17 hrs, Volume= 0.236 af

Routing by Dyn-Stor-Ind method, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 58.71' @ 12.17 hrs Surf.Area= 1,108 sf Storage= 489 cf

Plug-Flow detention time= 1.5 min calculated for 0.236 af (100% of inflow)

Center-of-Mass det. time= 1.5 min (817.3 - 815.8)

Volume	ln۱	vert Avai	l.Storage	Storage	Description	
#1	57.	50'	3,818 cf	Custom	Stage Data (Pr	ismatic) Listed below (Recalc)
Elevatio		Surf.Area (sq-ft)		c.Store ic-feet)	Cum.Store (cubic-feet)	
57.5	50	20		0	0	
58.0	00	150		43	43	
59.0	00	1,500		825	868	
60.0	00	4,400		2,950	3,818	
Device	Routing	In	vert Out	let Device	S	
#1	Primary	57	7.50' 12. 0	0" Round	Culvert	

L= 58.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 57.50' / 54.00' S= 0.0603 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.51 cfs @ 12.17 hrs HW=58.71' TW=48.06' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.51 cfs @ 3.20 fps)



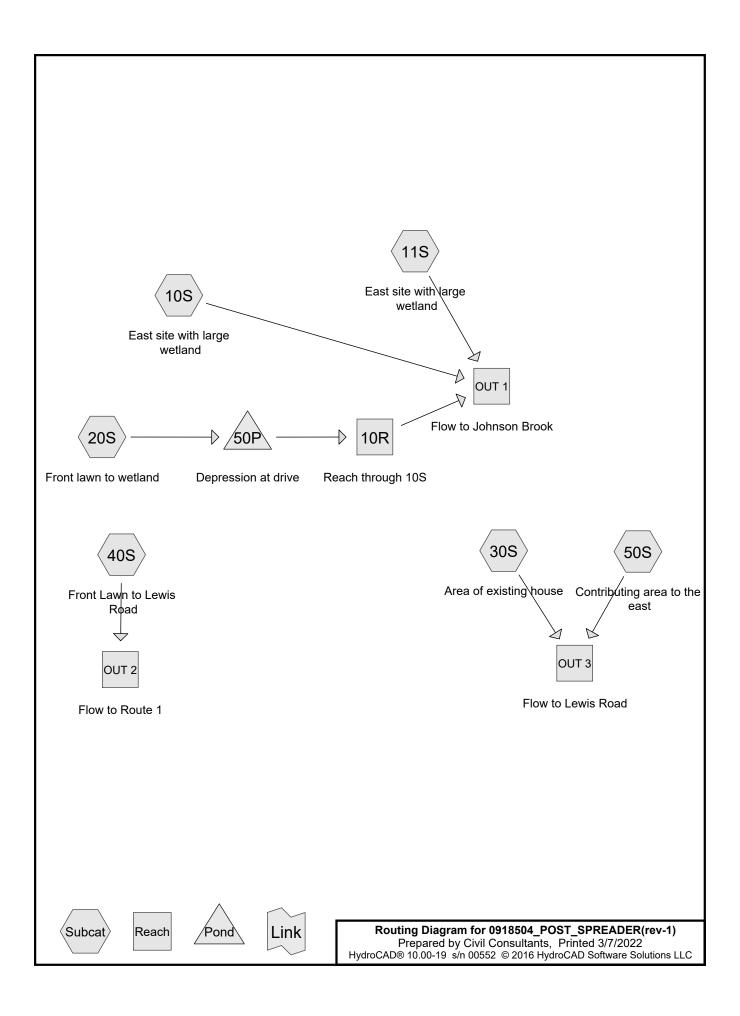
CIVIL CONSULTANTS

Engineers

Planners

Surveyors

Post-Development Calculations



Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
1.815	79	50-75% Grass cover, Fair, HSG C (30S, 40S)
0.137	79	50-75% Grass cover, Fair, HSG C (OUTDOOR DINING) (20S)
0.783	74	>75% Grass cover, Good, HSG C (10S, 20S)
0.122	70	Brush, Fair, HSG C (RECLAIMED GRAVEL PARKING) (50S)
0.013	96	Gravel surface, HSG C (WALKWAY TO BAR) (20S)
0.649	98	Paved parking, HSG C (10S, 20S, 40S)
0.675	92	Paved roads w/open ditches, 50% imp, HSG C (20S, 30S, 40S)
1.405	93	Paved roads w/open ditches, 50% imp, HSG D (10S, 20S, 30S, 50S)
0.728	98	Roofs, HSG C (30S)
0.004	98	Roofs, HSG C (OUTDOOR BAR BLDG) (20S)
9.764	73	Woods, Fair, HSG C (10S, 11S, 30S, 50S)
4.666	79	Woods, Fair, HSG D (10S, 11S, 30S, 50S)
20.761	79	TOTAL AREA

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Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
0.000	HSG B	
14.690	HSG C	10S, 11S, 20S, 30S, 40S, 50S
6.071	HSG D	10S, 11S, 20S, 30S, 50S
0.000	Other	
20.761		TOTAL AREA

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Subcatchi Numbers

Ground Covers (all nodes)

HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground
(acres)	(acres)	(acres)	(acres)	(acres)	(acres)	Cover
0.000	0.000	1.952	0.000	0.000	1.952	50-75% Grass cover, Fair
0.000	0.000	0.783	0.000	0.000	0.783	>75% Grass cover, Good
0.000	0.000	0.122	0.000	0.000	0.122	Brush, Fair
0.000	0.000	0.013	0.000	0.000	0.013	Gravel surface
0.000	0.000	0.649	0.000	0.000	0.649	Paved parking
0.000	0.000	0.675	1.405	0.000	2.080	Paved roads w/open ditches, 50%
						imp
0.000	0.000	0.732	0.000	0.000	0.732	Roofs
0.000	0.000	9.764	4.666	0.000	14.430	Woods, Fair
0.000	0.000	14.690	6.071	0.000	20.761	TOTAL AREA

Type III 24-hr 2 yr Rainfall=3.06" Printed 3/7/2022

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Time span=1.00-48.00 hrs, dt=0.01 hrs, 4701 points x 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 10S: East site with large Runoff Area=5.366 ac 11.68% Impervious Runoff Depth=1.23" Flow Length=630' Tc=18.7 min CN=79 Runoff=5.24 cfs 0.551 af

Subcatchment 11S: East site with large Runoff Area=2.181 ac 0.00% Impervious Runoff Depth=0.95" Flow Length=610' Tc=14.9 min CN=74 Runoff=1.71 cfs 0.172 af

Subcatchment 20S: Front lawn to wetland Runoff Area=37,897 sf 29.15% Impervious Runoff Depth=1.50" Flow Length=218' Tc=7.2 min CN=83 Runoff=1.46 cfs 0.108 af

Subcatchment 30S: Area of existing house Runoff Area=5.612 ac 16.09% Impervious Runoff Depth=1.23" Flow Length=750' Tc=31.1 min CN=79 Runoff=4.41 cfs 0.576 af

Subcatchment 40S: Front Lawn to Lewis Runoff Area=54,363 sf 26.52% Impervious Runoff Depth=1.64" Flow Length=425' Tc=8.0 min CN=85 Runoff=2.24 cfs 0.170 af

Subcatchment 50S: Contributing area to the Runoff Area=5.484 ac 5.59% Impervious Runoff Depth=1.11" Flow Length=783' Tc=25.6 min CN=77 Runoff=4.18 cfs 0.509 af

Reach 10R: Reach through 10S Avg. Flow Depth=0.04' Max Vel=0.24 fps Inflow=1.37 cfs 0.108 af n=0.050 L=760.0' S=0.0053 '/' Capacity=51.83 cfs Outflow=0.49 cfs 0.108 af

Reach OUT 1: Flow to Johnson BrookInflow=7.32 cfs 0.832 af
Outflow=7.32 cfs 0.832 af

Reach OUT 2: Flow to Route 1 Inflow=2.24 cfs 0.170 af
Outflow=2.24 cfs 0.170 af

Reach OUT 3: Flow to Lewis Road Inflow=8.51 cfs 1.085 af Outflow=8.51 cfs 1.085 af

Pond 50P: Depression at drive Peak Elev=58.22' Storage=107 cf Inflow=1.46 cfs 0.108 af 12.0" Round Culvert n=0.013 L=58.0' S=0.0603 '/' Outflow=1.37 cfs 0.108 af

Total Runoff Area = 20.761 ac Runoff Volume = 2.087 af Average Runoff Depth = 1.21" 88.34% Pervious = 18.340 ac 11.66% Impervious = 2.421 ac

Type III 24-hr 10 yr Rainfall=4.37"

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Time span=1.00-48.00 hrs, dt=0.01 hrs, 4701 points x 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment10S: East site with large Runoff Area=5.366 ac 11.68% Impervious Runoff Depth=2.27"

Flow Length=630' Tc=18.7 min CN=79 Runoff=9.86 cfs 1.014 af

Subcatchment 11S: East site with large Runoff Area=2.181 ac 0.00% Impervious Runoff Depth=1.87"

Flow Length=610' Tc=14.9 min CN=74 Runoff=3.57 cfs 0.340 af

Subcatchment 20S: Front lawn to wetland Runoff Area=37,897 sf 29.15% Impervious Runoff Depth=2.61" Flow Length=218' Tc=7.2 min CN=83 Runoff=2.55 cfs 0.189 af

Subcatchment 30S: Area of existing house Runoff Area=5.612 ac 16.09% Impervious Runoff Depth=2.27" Flow Length=750' Tc=31.1 min CN=79 Runoff=8.28 cfs 1.061 af

Subcatchment 40S: Front Lawn to Lewis

Runoff Area=54,363 sf 26.52% Impervious Runoff Depth=2.79"

Flow Length=425' Tc=8.0 min CN=85 Runoff=3.79 cfs 0.290 af

Subcatchment 50S: Contributing area to the Runoff Area=5.484 ac 5.59% Impervious Runoff Depth=2.11" Flow Length=783' Tc=25.6 min CN=77 Runoff=8.17 cfs 0.962 af

Reach 10R: Reach through 10S Avg. Flow Depth=0.06' Max Vel=0.31 fps Inflow=2.16 cfs 0.189 af

n=0.050 L=760.0' S=0.0053 '/' Capacity=51.83 cfs Outflow=1.05 cfs 0.189 af

Reach OUT 1: Flow to Johnson Brook Inflow=14.24 cfs 1.544 af

Outflow=14.24 cfs 1.544 af

Reach OUT 2: Flow to Route 1 Inflow=3.79 cfs 0.290 af

Outflow=3.79 cfs 0.290 af

Reach OUT 3: Flow to Lewis Road Inflow=16.29 cfs 2.023 af

Outflow=16.29 cfs 2.023 af

Pond 50P: Depression at drive Peak Elev=58.53' Storage=308 cf Inflow=2.55 cfs 0.189 af

12.0" Round Culvert $\,$ n=0.013 L=58.0' S=0.0603 '/' Outflow=2.16 cfs $\,$ 0.189 af

Total Runoff Area = 20.761 ac Runoff Volume = 3.857 af Average Runoff Depth = 2.23" 88.34% Pervious = 18.340 ac 11.66% Impervious = 2.421 ac

Type III 24-hr 25 yr Rainfall=5.20"

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Time span=1.00-48.00 hrs, dt=0.01 hrs, 4701 points x 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 10S: East site with large Runoff Area=5.366 ac 11.68% Impervious Runoff Depth=2.97" Flow Length=630' Tc=18.7 min CN=79 Runoff=12.97 cfs 1.330 af

Subcatchment 11S: East site with large Runoff Area=2.181 ac 0.00% Impervious Runoff Depth=2.52" Flow Length=610' Tc=14.9 min CN=74 Runoff=4.87 cfs 0.459 af

Subcatchment 20S: Front lawn to wetland Runoff Area=37,897 sf 29.15% Impervious Runoff Depth=3.36" Flow Length=218' Tc=7.2 min CN=83 Runoff=3.26 cfs 0.243 af

Subcatchment 30S: Area of existing house Runoff Area=5.612 ac 16.09% Impervious Runoff Depth=2.97" Flow Length=750' Tc=31.1 min CN=79 Runoff=10.87 cfs 1.391 af

Subcatchment 40S: Front Lawn to Lewis

Runoff Area=54,363 sf 26.52% Impervious Runoff Depth=3.55"

Flow Length=425' Tc=8.0 min CN=85 Runoff=4.80 cfs 0.370 af

Subcatchment 50S: Contributing area to the Runoff Area=5.484 ac 5.59% Impervious Runoff Depth=2.79" Flow Length=783' Tc=25.6 min CN=77 Runoff=10.88 cfs 1.276 af

Reach 10R: Reach through 10SAvg. Flow Depth=0.07' Max Vel=0.35 fps Inflow=2.56 cfs 0.243 af n=0.050 L=760.0' S=0.0053 '/' Capacity=51.83 cfs Outflow=1.44 cfs 0.243 af

Reach OUT 1: Flow to Johnson Brook Inflow=18.89 cfs 2.032 af Outflow=18.89 cfs 2.032 af

Reach OUT 2: Flow to Route 1 Inflow=4.80 cfs 0.370 af
Outflow=4.80 cfs 0.370 af

Reach OUT 3: Flow to Lewis Road Inflow=21.55 cfs 2.667 af Outflow=21.55 cfs 2.667 af

Pond 50P: Depression at drive Peak Elev=58.73' Storage=517 cf Inflow=3.26 cfs 0.243 af 12.0" Round Culvert n=0.013 L=58.0' S=0.0603 '/' Outflow=2.56 cfs 0.243 af

Total Runoff Area = 20.761 ac Runoff Volume = 5.069 af Average Runoff Depth = 2.93" 88.34% Pervious = 18.340 ac 11.66% Impervious = 2.421 ac

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Summary for Subcatchment 10S: East site with large wetland

Runoff = 12.97 cfs @ 12.26 hrs, Volume= 1.330 af, Depth= 2.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25 yr Rainfall=5.20"

Area	(ac) C	N Desc	cription		
2.	070 7	79 Woo	ds, Fair, H	ISG D	
1.995 73 Woods, Fair, HSG C					
0.	560	93 Pave	ed roads w	/open ditch	ies, 50% imp, HSG D
0.	394	74 >75°	% Grass co	over, Good,	, HSG C
			ed parking		
0.	041	Pave	ed parking	, HSG C	
5.	366 7	79 Weig	ghted Aver	age	
4.	739	88.3	2% Pervio	us Area	
0.	627	11.6	8% Imper\	/ious Area	
_				_	
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
5.5	50	0.0600	0.15		Sheet Flow, 10.1
					Grass: Dense n= 0.240 P2= 3.00"
0.9	75	0.0400	1.40		Shallow Concentrated Flow, 10.2
•					Short Grass Pasture Kv= 7.0 fps
6.3	100	0.0500	0.27		Sheet Flow, 10.3
4.0	0.5	0.0000	0.00		Range n= 0.130 P2= 3.00"
1.2	65	0.0308	0.88		Shallow Concentrated Flow, 10.4
4.0	240	0.0450	1 10	20.26	Woodland Kv= 5.0 fps
4.8	340	0.0150	1.19	22.36	Trap/Vee/Rect Channel Flow, 10.5
					Bot.W=50.00' D=0.25' Z= 100.0 '/' Top.W=100.00' n= 0.050 Scattered brush, heavy weeds
40.7	000	T-4-1			n- 0.000 Scattered brush, neavy weeds
18.7	630	Total			

Summary for Subcatchment 11S: East site with large wetland

Runoff = 4.87 cfs @ 12.20 hrs, Volume= 0.459 af, Depth= 2.52"

Area (ac)	CN	Description
0.528	79	Woods, Fair, HSG D
1.653	73	Woods, Fair, HSG C
 2.181	74	Weighted Average
2.181		100.00% Pervious Area

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	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	8.1	100	0.0900	0.21		Sheet Flow, 11.1
						Grass: Dense n= 0.240 P2= 3.00"
	0.4	45	0.0600	1.71		Shallow Concentrated Flow, 11.2
						Short Grass Pasture Kv= 7.0 fps
	1.6	125	0.0720	1.34		Shallow Concentrated Flow, 11.3
						Woodland Kv= 5.0 fps
	4.8	340	0.0150	1.19	22.36	Trap/Vee/Rect Channel Flow, 11.4
						Bot.W=50.00' D=0.25' Z= 100.0 '/' Top.W=100.00'
_						n= 0.050 Scattered brush, heavy weeds
	14.9	610	Total			

Summary for Subcatchment 20S: Front lawn to wetland

Runoff = 3.26 cfs @ 12.10 hrs, Volume= 0.243 af, Depth= 3.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25 yr Rainfall=5.20"

	Α	rea (sf)	CN I	Description						
		7,492	98 I	Paved park	ing, HSG C					
		5,227	92 I	Paved road	s w/open d	itches, 50% imp, HSG C				
		16,953	74	>75% Grass cover, Good, HSG C						
		1,525	93 I	Paved road	s w/open d	itches, 50% imp, HSG D				
*		5,955	79	50-75% Gra	ass cover, F	Fair, HSG C (OUTDOOR DINING)				
*		565	96	Gravel surfa	ace, <mark>HSG C</mark>	C (WALKWAY TO BAR)				
*		180	98 I	Roofs, HSG	C (OUTD)	OOR BAR BLDG)				
_		37,897	83 \	Neighted A	verage					
		26,849	-	70.85% Per	vious Area					
		11,048		29.15% Imp	pervious Are	ea				
	Tc	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	4.9	50	0.0310	0.17		Sheet Flow, 20.1				
						Grass: Short n= 0.150 P2= 3.00"				
	2.3	168	0.0300	1.21		Shallow Concentrated Flow, 20.2				
						Short Grass Pasture Kv= 7.0 fps				
	7.2	218	Total							

Summary for Subcatchment 30S: Area of existing house

Runoff = 10.87 cfs @ 12.43 hrs, Volume= 1.391 af, Depth= 2.97"

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Area	(ac) C	N Desc	cription		
3.	000 7	73 Woo	ds, Fair, H	SG C	
0.	, HSG C				
0.					
0.	153				ies, 50% imp, HSG C
0.	197 9	3 Pave	ed roads w	open ditch	ies, 50% imp, HSG D
			s, HSG C		
0.	660 7	79 50-7	5% Grass	cover, Fair	HSG C
5.	612 7		hted Aver	•	
	709		1% Pervio		
0.	903	16.0	9% Imperv	ious Area	
_					
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
3.3	35	0.0400	0.18		Sheet Flow, 30.1
					Grass: Short n= 0.150 P2= 3.00"
0.5	115	0.0430	4.21		Shallow Concentrated Flow, 30.2
00.0	400	0.0050	0.00		Paved Kv= 20.3 fps
20.3	100	0.0250	0.08		Sheet Flow, 30.3
2.4	115	0.0040	0.77		Woods: Light underbrush n= 0.400 P2= 3.00"
3.1	145	0.0240	0.77		Shallow Concentrated Flow, 30.4
2.9	180	0.0440	1.05		Woodland Kv= 5.0 fps
2.9	100	0.0440	1.03		Shallow Concentrated Flow, 30.5 Woodland Kv= 5.0 fps
1.0	175	0.0150	2.97	88.26	Trap/Vee/Rect Channel Flow, 30.6
1.0	173	0.0100	2.01	00.20	Bot.W=50.00' D=0.50' Z= 8.0 & 30.0 '/' Top.W=69.00'
					n= 0.035 Scattered brush, heavy weeds
31.1	750	Total			titteame.ou a.ue,eu,eu.

Summary for Subcatchment 40S: Front Lawn to Lewis Road

Runoff = 4.80 cfs @ 12.11 hrs, Volume= 0.370 af, Depth= 3.55"

Area (sf)	CN	Description
17,511	92	Paved roads w/open ditches, 50% imp, HSG C
5,663	98	Paved parking, HSG C
31,189	79	50-75% Grass cover, Fair, HSG C
54,363	85	Weighted Average
39,945		73.48% Pervious Area
14,419		26.52% Impervious Area

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_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	4.9	50	0.0310	0.17		Sheet Flow, 40.1
	2.1	155	0.0310	1.23		Grass: Short n= 0.150 P2= 3.00" Shallow Concentrated Flow, 40.2 Short Grass Pasture Kv= 7.0 fps
	1.0	220	0.0270	3.52	2.86	Trap/Vee/Rect Channel Flow, 40.3
						Bot.W=2.00' D=0.25' Z= 5.0 '/' Top.W=4.50' n= 0.022 Earth, clean & straight
-	8.0	425	Total			·

Summary for Subcatchment 50S: Contributing area to the east

Runoff = 10.88 cfs @ 12.37 hrs, Volume= 1.276 af, Depth= 2.79"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25 yr Rainfall=5.20"

	Area	(ac) C	N Des	cription		
	3.	116	73 Woo	ds, Fair, H	SG C	
	1.	633	79 Woo	ds, Fair, H	SG D	
	0.	613	93 Pav	ed roads w	open ditch	ies, 50% imp, HSG D
*	0.	122				LAIMED GRAVEL PARKING)
	5.	484	77 Wei	ghted Aver	age	<u> </u>
		178		1% Pervio	•	
		306		% Impervi		
	•		0.00	70		
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•
	18.9	100	0.0300	0.09		Sheet Flow, 50.1
						Woods: Light underbrush n= 0.400 P2= 3.00"
	3.1	145	0.0240	0.77		Shallow Concentrated Flow, 50.2
						Woodland Kv= 5.0 fps
	0.2	30	0.0330	2.92		Shallow Concentrated Flow, 50.3
						Unpaved Kv= 16.1 fps
	1.8	142	0.0700	1.32		Shallow Concentrated Flow, 50.4
						Woodland Kv= 5.0 fps
	1.6	366	0.0164	3.92	10.29	Trap/Vee/Rect Channel Flow, 50.5
						Bot.W=2.00' D=0.50' Z= 3.0 & 10.0 '/' Top.W=8.50'
						n= 0.022 Earth, clean & straight
	25.6	783	Total			

Summary for Reach 10R: Reach through 10S

Inflow Area = 0.870 ac, 29.15% Impervious, Inflow Depth = 3.36" for 25 yr event

Inflow = 2.56 cfs @ 12.17 hrs, Volume= 0.243 af

Outflow = 1.44 cfs @ 12.40 hrs, Volume= 0.243 af, Atten= 44%, Lag= 14.0 min

Type III 24-hr 25 yr Rainfall=5.20"

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Routing by Dyn-Stor-Ind method, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs / 3

Max. Velocity = 0.35 fps, Min. Travel Time = 36.5 min Avg. Velocity = 0.09 fps, Avg. Travel Time = 137.7 min

Peak Storage= 3,161 cf @ 12.40 hrs Average Depth at Peak Storage= 0.07'

Bank-Full Depth= 0.50' Flow Area= 50.0 sf, Capacity= 51.83 cfs

50.00' x 0.50' deep channel, n= 0.050 Scattered brush, heavy weeds

Side Slope Z-value= 100.0 '/' Top Width= 150.00'

Length= 760.0' Slope= 0.0053 '/'

Inlet Invert= 48.00', Outlet Invert= 44.00'



Summary for Reach OUT 1: Flow to Johnson Brook

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 8.417 ac, 10.46% Impervious, Inflow Depth = 2.90" for 25 yr event

Inflow = 18.89 cfs @ 12.24 hrs, Volume= 2.032 af

Outflow = 18.89 cfs @ 12.24 hrs, Volume= 2.032 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs / 3

Summary for Reach OUT 2: Flow to Route 1

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 1.248 ac, 26.52% Impervious, Inflow Depth = 3.55" for 25 yr event

Inflow = 4.80 cfs @ 12.11 hrs, Volume= 0.370 af

Outflow = 4.80 cfs @ 12.11 hrs, Volume= 0.370 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs / 3

Summary for Reach OUT 3: Flow to Lewis Road

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 11.096 ac, 10.90% Impervious, Inflow Depth = 2.88" for 25 yr event

Inflow = 21.55 cfs @ 12.40 hrs, Volume= 2.667 af

Outflow = 21.55 cfs @ 12.40 hrs, Volume= 2.667 af, Atten= 0%, Lag= 0.0 min

Type III 24-hr 25 yr Rainfall=5.20"

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Routing by Dyn-Stor-Ind method, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs / 3

Summary for Pond 50P: Depression at drive

Inflow Area = 0.870 ac, 29.15% Impervious, Inflow Depth = 3.36" for 25 yr event

Inflow = 3.26 cfs @ 12.10 hrs, Volume= 0.243 af

Outflow = 2.56 cfs @ 12.17 hrs, Volume= 0.243 af, Atten= 22%, Lag= 4.1 min

Primary = 2.56 cfs @ 12.17 hrs, Volume= 0.243 af

Routing by Dyn-Stor-Ind method, Time Span= 1.00-48.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 58.73' @ 12.17 hrs Surf.Area= 1,142 sf Storage= 517 cf

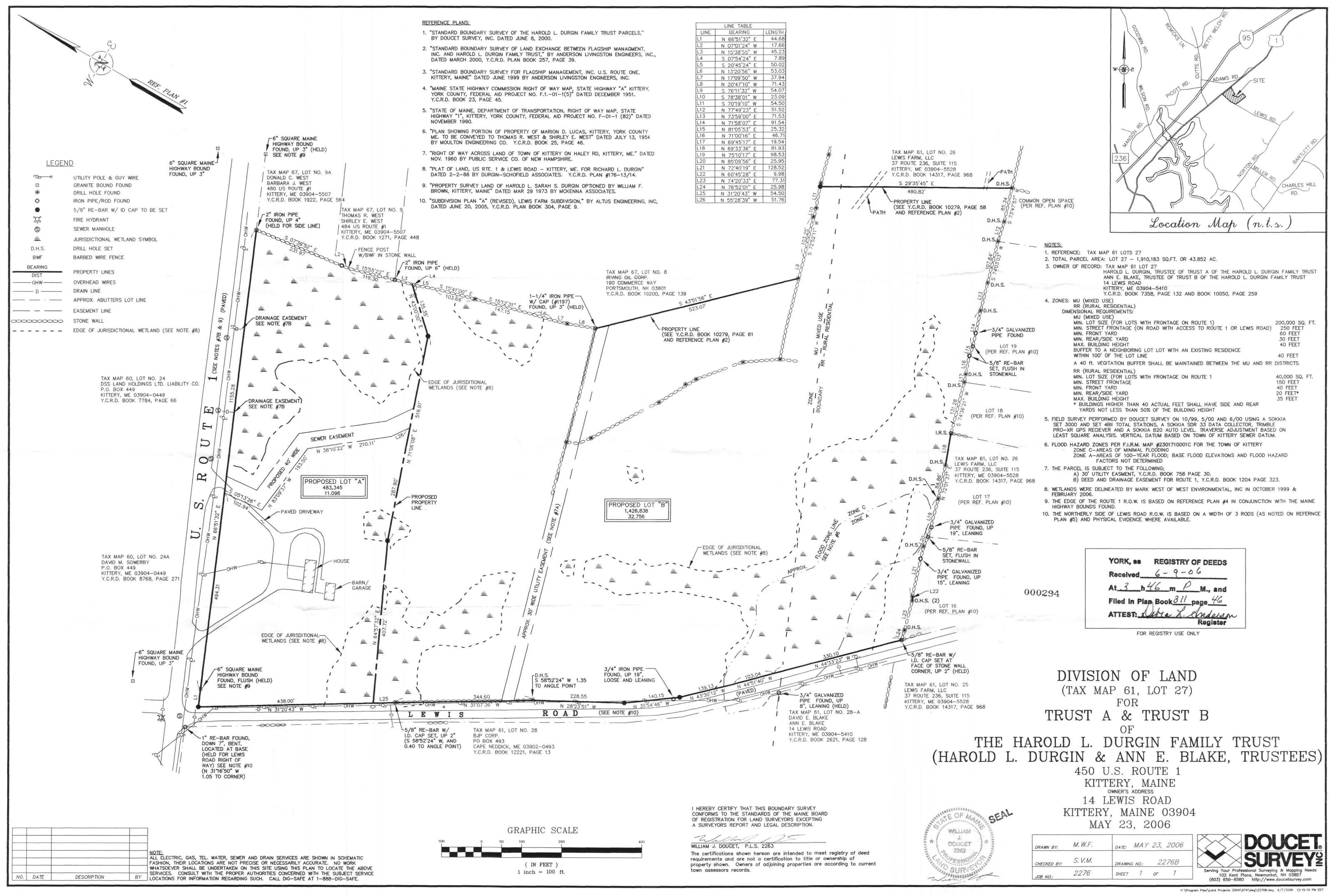
Plug-Flow detention time= 1.7 min calculated for 0.243 af (100% of inflow)

Center-of-Mass det. time= 1.6 min (814.5 - 813.0)

Volume	ln۱	∕ert Avai	I.Storage	Storage	Description	
#1	57.	.50'	3,818 cf	Custom	Stage Data (Pi	rismatic)Listed below (Recalc)
Elevatio		Surf.Area (sq-ft)		:.Store c-feet)	Cum.Store (cubic-feet)	
57.5		20	(OGDI	0	0	
58.0	00	150		43	43	
59.0	00	1,500		825	868	
60.0	00	4,400		2,950	3,818	
Device	Routing	In	vert Outl	et Device	S	
#1	Primary	57	.50' 12.0	" Round	Culvert	

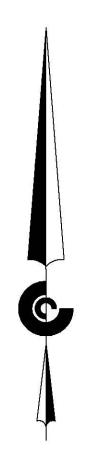
L= 58.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 57.50' / 54.00' S= 0.0603 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.56 cfs @ 12.17 hrs HW=58.73' TW=48.06' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.56 cfs @ 3.26 fps)



WHEN PIGS FLY 460 U.S. ROUTE 1 KITTERY, MAINE FINAL SITE PLAN APPLICATION





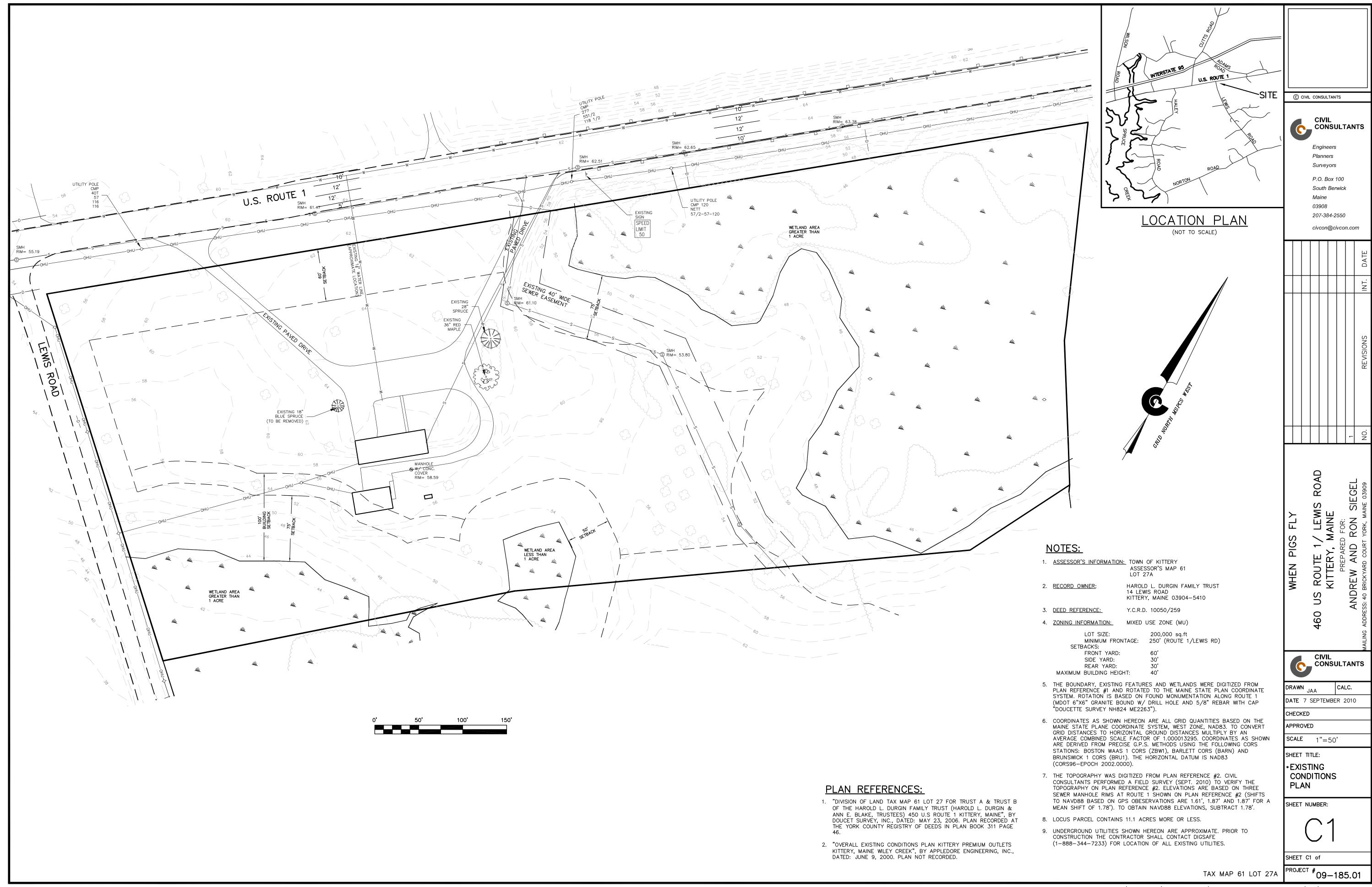
SHEET NUMBER	SHEET TITLE
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C2	OVERALL SITE PLAN
C3	ENLARGED SITE PLAN
C4	CONSTRUCTION DETAILS
C5	MAINTENANCE NOTES
EP	POINT BY POINT LIGHTING STUD

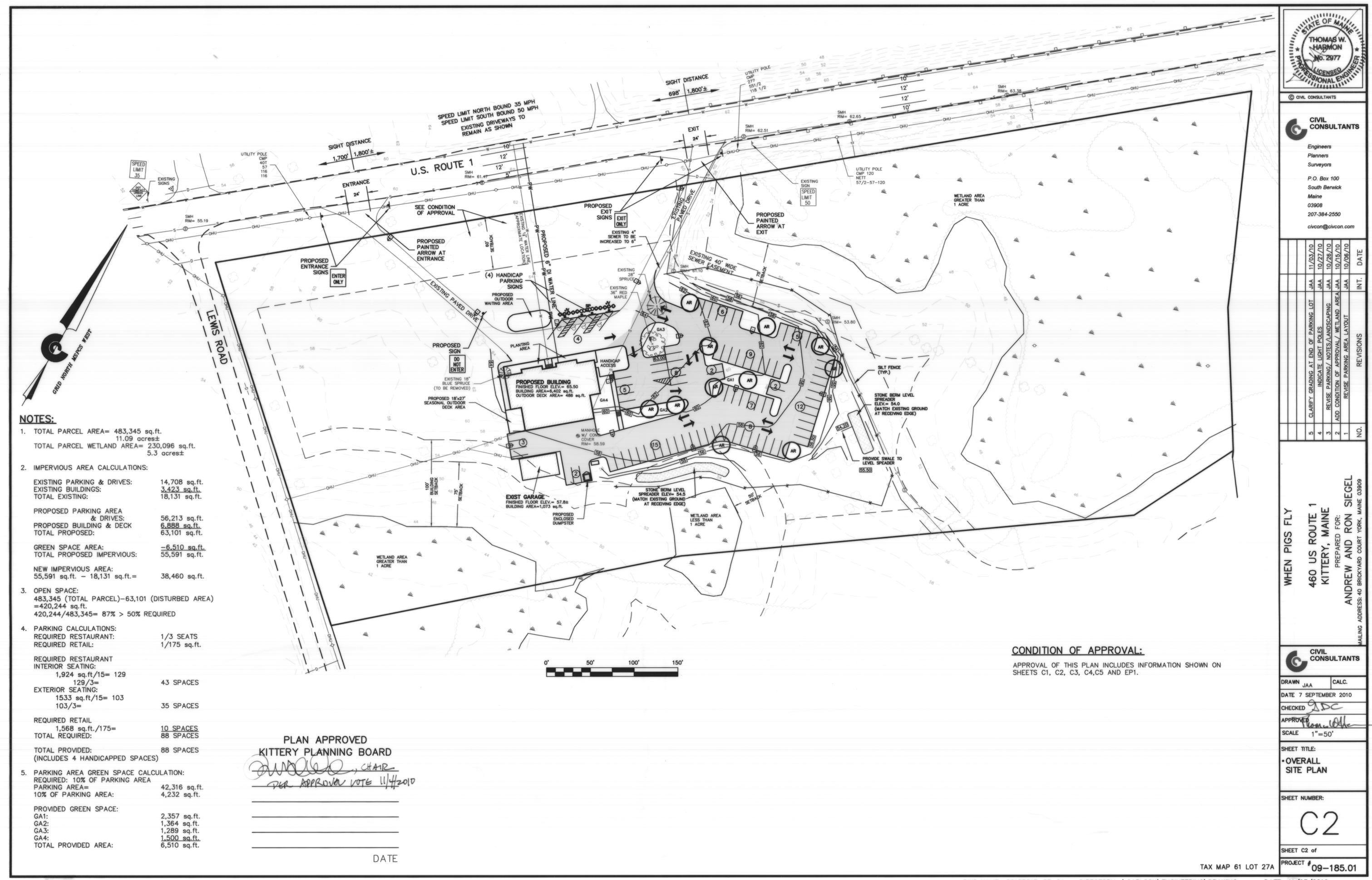
LOCATION PLAN
NOT TO SCALE

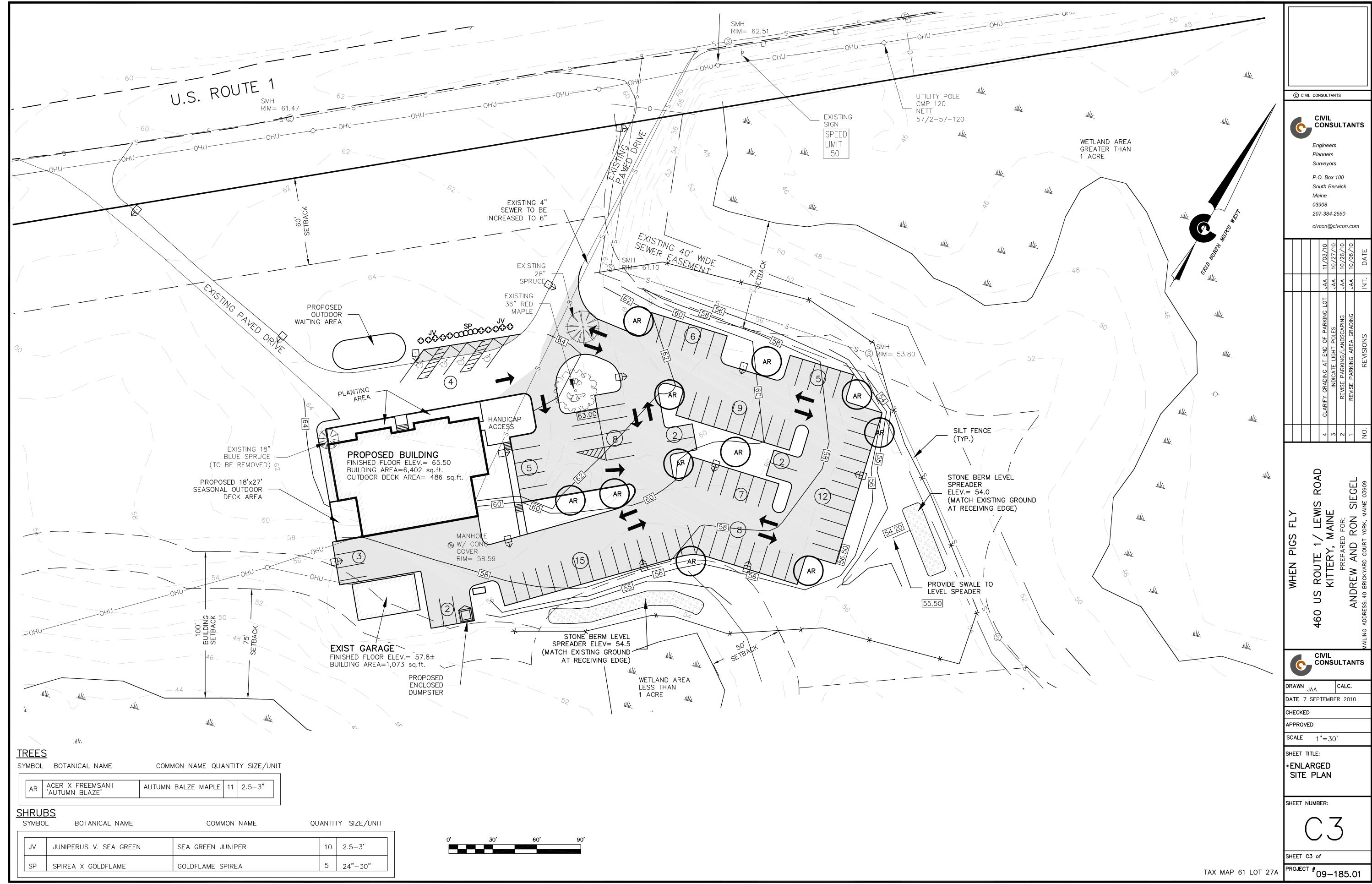


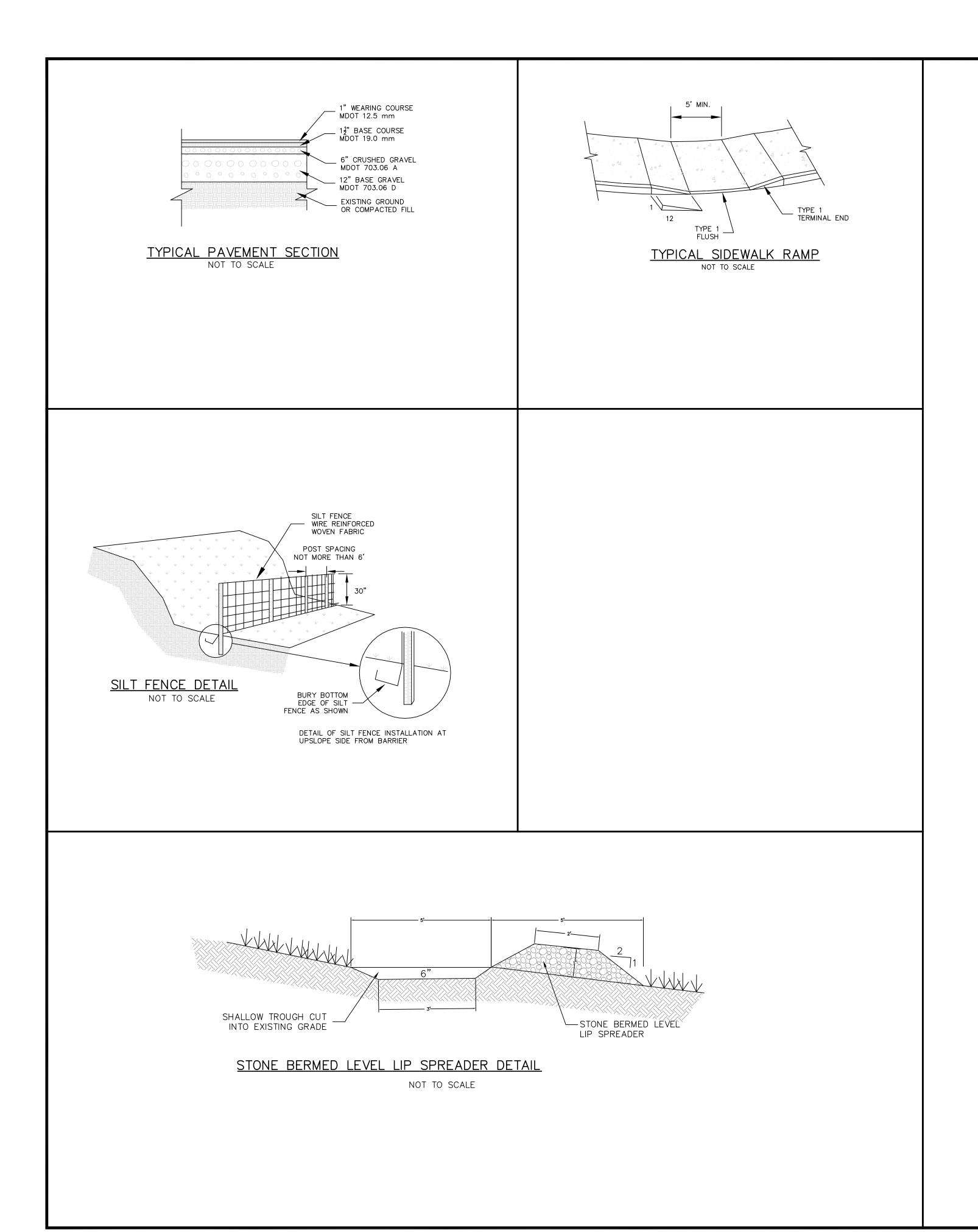
PREPARED FOR:

ANDREW AND RON SIEGEL









EROSION AND SEDIMENT CONTROL PRACTICES

- NO SOIL SHALL BE DISTURBED DURING THE PERIOD OF MARCH 1 THROUGH APRIL 15, NOR DURING ANY OTHER PERIOD WHEN SOILS ARE SATURATED DUE TO RAIN OR SNOW MELT.
- 2. DISTURBED SOILS SHALL BE STABILIZED WITHIN ONE (1) WEEK FROM THE TIME IT WAS LAST ACTIVELY WORKED USING TEMPORARY OR PERMANENT MEASURES SUCH AS PLACEMENT OF RIPRAP, MULCH OR OTHER EROSION CONTROL BLANKET, OR OTHER COMPARABLE MEASURES.
- 3. HAY OR STRAW MULCH, WHERE USED, SHALL BE APPLIED AT A RATE OF AT LEAST ONE (1) BALE PER 500 SQUARE FEET (1-2 TONS PER ACRE).
- 4. IF MULCH IS LIKELY TO BE REMOVED DUE TO TO STEEP SLOPES OR WIND, IT SHALL BE ANCHORED WITH NETTING, PEG OR TWINE, OR OTHER SUITABLE METHOD AND SHALL BE MAINTAINED UNTIL A CATCH OF VEGETATION IS ESTABLISHED OVER THE ENTIRE DISTURBED AREA.
- 5. IN ADDITION TO PLACEMENT OF RIPRAP, MULCH OR EROSION CONTROL BLANKETS, ADDITIONAL STEPS SHALL BE TAKEN WHERE NECESSARY IN ORDER TO PREVENT SEDIMENTATION OF THE WATER. EVIDENCE OF SEDIMENTATION INCLUDES VISIBLE GULLY EROSION, DISCOLORATION OF WATER BY SUSPENDED PARTICLES AND SLUMPING OF BANKS, SILT FENCES, STAKED HAY BALES AND OTHER SEDIMENTATION CONTROL MEASURES, WHERE PLANNED FOR, SHALL BE IN PLACE PRIOR TO COMMENCEMENT OF WORK, BUT SHALL ALSO BE INSTALLED WHEREVER NECESSARY DUE TO SEDIMENTATION.
- 6. MULCH OR OTHER TEMPORARY MEASURES SHALL BE MAINTAINED UNTIL THE SITE IS PERMANENTLY STABILIZED WITH VEGETATION OR OTHER PERMANENT CONTROL MEASURES AFTER WHICH TEMPORARY MEASURES WILL BE REMOVED.
- 7. PERMANENT RE-VEGETATION OF ALL DISTURBED AREAS, USING NATIVE PLANT MATERIAL WHEN POSSIBLE, SHALL OCCUR WITHIN 30 DAYS FROM THE TIME THE AREAS WERE LAST ACTIVELY WORKED, OR FOR FALL AND WINTER ACTIVITIES, BY JUNE 15, EXCEPT WHERE PRECLUDED BY THE TYPE OF ACTIVITY (E.G.RIRRAP, ROAD SURFACES, ETC.). THE VEGETATIVE COVER SHALL BE MAINTAINED.
- 8. DISPOSAL OF COLLECTED DEBRIS MUST BE IN CONFORMANCE WITH MAINE SOLID WASTE LAW, TITLE 38 MRSA SECTION 1301 ET. SEQ.
- 9. LIME AND FERTILIZER APPLICATION RATES SHALL NOT EXCEED THE FOLLOWING:

GROUND LIMESTONE: 3 TONS/ACRE (130 LBS./1000 S.F.)

FERTILIZER, 10-10-10 OF EQUIVALENT: 600 LBS./ACRE (14 LBS./1000 S.F.)

FERTILIZER SHALL NOT BE APPLIED BEFORE START OF THE GROWING SEASON NOR AFTER SEPTEMBER 30 FERTILIZED AREAS SHALL BE MULICHED TO

FERTILIZER SHALL NOT BE APPLIED BEFORE START OF THE GROWING SEASONOR AFTER SEPTEMBER 30. FERTILIZED AREAS SHALL BE MULCHED TO REDUCE OFF—SITE TRANSPORT OF NUTRIENTS UNTIL USED BY VEGETATIVE GROWTH.

SEEDING MIXTURE AND SCHEDULE:

SPREAD TOPSOIL UNIFORMLY 6" DEEP OVER AREAS TO BE RECLAIMED. THE FOLLOWING SEED MIXTURE SHALL BE USED:

LAWNS:

KENTUCKY BLUEGRASS 0.46 LBS./1000 S.F.
CREEPING RED FESCUE
PERENNIAL RYE GRASS
TOTAL 0.46 LBS./1000 S.F.
0.46 LBS./1000 S.F.
1.03 LBS./1000 S.F.

APPLY LIME AND FERTILIZER AS SPECIFIED UNDER THE EROSION AND SEDIMENTATION CONTROL NOTES. WORK INTO THE TOP (4) INCHES OF SOIL PRIOR TO SEEDING. AFTER SEEDING, APPLY MULCH HAY AS SPECIFIED. ON FLAT AREAS AND NOT EXPOSED TO WIND, THE MULCH WILL BE ANCHORED BY WETTING DOWN. IN OTHER AREAS, JUTE NETTING SHALL BE USED FOR ANCHORAGE. THE ABOVE SEEDING SCHEDULE IS APPLICABLE IF SEEDING DURING THE GROWING SEASON (APRIL 15 TO JUNE 15 AND AUGUST 30 TO SEPTEMBER 30). BETWEEN JUNE 15 AND AUGUST 30, SEEDING WILL BE DELAYED UNTIL AUGUST 30. IF SOIL IS DISTURBED BETWEEN OCTOBER 1 AND NOVEMBER 1, DELAY SEEDING UNTIL NOVEMBER 1. AFTER NOVEMBER 1 AND BEFORE A SNOW COVER FORMS, THE SAME PROCEDURE WILL BE FOLLOWED EXCEPT THE SEED RATE WILL BE DOUBLED. AFTER SNOW COVER AND BEFORE APRIL 15, SEEDING WILL BE DELAYED UNTIL APRIL 15. HAY MULCH WILL BE APPLIED AT A RATE OF 150 LBS./1000 SQUARE FEET. THIS WILL BE ANCHORED BY NON—ASPHALTIC TACKIFIER SPRAYED ON LAWNS AND JUTE NETTING IN DRAINAGE WAYS AND OTHER AREAS.



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1 ADD LEVEL SPREADER DETAIL NJR 10/06/10 NO. REVISIONS INT. DATE

WHEN PIGS FLY ROUTE 1/ LEWIS ROA

O US ROUTE . KITTERY,



DATE 7 SEPTEMBER 2010
CHECKED

APPROVED

SCALE AS NOTED

SHEET TITLE:
• CONSTRUCTION

DETAILS

SHEET C4 of

SHEET NUMBER:

TAX MAP 61 LOT 27A PROJECT #09-185.01

MAINTENANCE PROCEDURES

THE FOLLOWING PROCEDURES WILL BE FOLLOWED FOR INITIAL AND LONG TERM MAINTENANCE OF THE STORMWATER MANAGEMENT FACILITIES AT THIS SITE. NOTE: FOR THE PURPOSES OF THESE PROCEDURES, A MAJOR STORM EVENT IS CLSSIFIED AS A RAINFALL EXCEEDING 3.0 INCHES. A SIGNIFICANT RAINFALL IS 1/2" IN A 24 HOUR PERIOD.

MAINTENANCE LO

THE RESPONSIBLE PARTY SHALL ESTABLISH A MAINTENANCE LOG/PLAN FOR USE IN RECORDING MAINTENANCE ACTIVITIES. AS A MINIMUM, THE LOG SHALL INCLUDE THE DATE(S) OF ACTIVITIES, WHO PERFORMED THE DUTIES, WHAT WAS DONE (I.E. LOOKED AT DETENTION BERMS, CLEANED DROP INLETS, ETC.), THE RESULTS OF THE ACTIVITY (I.E. ALL STRUCTURES WERE IN GOOD SHAPE, OR, POND #44 NEEDS TO BE REPAIRED). IF ANY ITEM NEEDS TO BE REPAIRED, A FOLLOW-UP ENTRY SHALL SHOW THE DATE THAT REPAIRS WERE COMPLETED.

DETENTION BASINS (INITIAL AND LONG TERM)

MAINTENANCE IS NECESSARY IF DETENTION BASINS ARE TO CONTINUE TO FUNCTION AS ORIGINALLY DESIGNED. THE RESPONSIBLE PARTY SHALL DESIGNATE AN INDIVIDUAL (OR COMPANY) TO MAINTAIN THE STRUCTURES AND THE BASIN AREA.

THE FOLLOWING MAINTENANCE SCHEDULE ARE ITEMS WHICH SHOULD BE CONSIDERED IN FORMULATING A MAINTENANCE

1. EMBANKMENT — EMBANKMENTS SHOULD BE INSPECTED ANNUALLY TO DETERMINE IF RODENT BURROWS, WET AREAS, OR EROSION OF THE FILL IS TAKING PLACE.

2. VEGETATION — THE VEGETATED AREAS OF STRUCTURE DEVICES SHOULD BE PROTECTED FROM DAMAGE BY FIRE, GRAZING, TRAFFIC, AND DENSE WEED GROWTH. LIME AND FERTILIZER SHOULD BE APPLIED AS NECESSARY AS DETERMINED BY SOIL TESTS. TREES AND SHRUBS SHOULD BE KEPT OFF THE EMBANKMENT AND EMERGENCY SPILLWAY AREAS.

3. INLETS — PIPE INLETS AND SPILLWAY STRUCTURES SHOULD BE INSPECTED ANNUALLY AND AFTER EVERY MAJOR STORM. ACCUMULATED DEBRIS AND SEDIMENT SHOULD BE REMOVED. IF PIPES ARE COATED, THE COATING SHOULD BE CHECKED AND REPAIRED AS NECESSARY.

4. OUTLETS — PIPE OUTLETS SHOULD BE INSPECTED ANNUALLY AND AFTER EVERY MAJOR STORM. THE CONDITION OF THE PIPES SHOULD BE NOTED AND REPAIRS MADE AS NECESSARY. IF EROSION IS TAKING PLACE THEN MEASURES SHOULD BE TAKEN TO STABILIZE AND PROTECT. THE AFFECTED AREA OF THE OUTLET.

5. SEDIMENT — SEDIMENT SHOULD BE CONTINUALLY CHECKED IN THE BASIN. WHEN SEDIMENT ACCUMULATIONS REACH THE PREDETERMINED DESIGN ELEVATION, THEN THE SEDIMENT SHOULD BE REMOVED AND PROPERLY DISPOSED OF.
6. SAFETY INSPECTIONS — ALL BERMS OVER 2' IN HEIGHT SHALL BE INSPECTED BY A QUALIFIED PROFESSIONAL ENGINEER EVERY 5 YEARS. THE DESIGNATED INDIVIDUAL SHOULD ALSO MAKE INSPECTIONS AFTER EVERY MAJOR STORM EVENT.

LAND GRADING AND SLOPE STABILIZATION

ALL SLOPES SHOULD BE CHECKED PERIODICALLY TO SEE THAT VEGETATION IS IN GOOD CONDITION. ANY RILLS OR DAMAGE FROM EROSION AND ANIMAL BURROWING SHOULD BE REPAIRED IMMEDIATELY TO AVOID FURTHER DAMAGE. IF SEEPS DEVELOP ON THE SLOPES, THE AREA SHOULD BE EVALUATED TO DETERMINE IF THE SEEP WILL CAUSE AN UNSTABLE CONDITION. SUBSURFACE DRAINS OR GRAVEL MULCHING MAY BE REQUIRED TO SOLVE SEEP PROBLEMS. DIVERSIONS, BERMS, AND WATERWAYS IN THE LAND GRADING AREA SHOULD BE CHECKED TO SEE THAT THEY ARE FUNCTIONING PROPERLY. PROBLEMS FOUND DURING THE INSPECTIONS SHOULD BE REPAIRED. SLOPES AND ASSOCIATED PRACTICES UTILIZING VEGETATION SHOULD BE LIMED AND FERTILIZED AS NECESSARY TO KEEP THE VEGETATION HEALTHY. ENCROACHMENT OF UNDESIRABLE VEGETATION SUCH AS WEEDS AND WOODY GROWTH THAT IS NOT PLANNED SHOULD BE CONTROLLED TO AVOID PROBLEMS OF BANK STABILITY IN THE FUTURE.

LEVEL SPREADE

LEVEL SPREADERS SHOULD BE CHECKED PERIODICALLY AND AFTER EVERY MAJOR STORM TO DETERMINE IF THE LIP HAS BEEN DAMAGED AND THAT THE DESIGN CONDITIONS HAVE NOT CHANGED. ANY SEDIMENT ACCUMULATION SHOULD BE REMOVED. DAMAGE SHOULD BE REPAIRED AND RE-VEGETATED. THE VEGETATION SHOULD BE MOWED OCCASIONALLY TO CONTROL WEEDS AND THE ENCROACHMENT OF WOODY VEGETATION. CLIPPINGS SHOULD BE REMOVED AND DISPOSED OF OUTSIDE THE SPREADER AND AWAY FROM THE OUTLET AREA. FERTILIZATION SHOULD BE DONE AS NECESSARY TO KEEP THE VEGETATION HEALTHY AND DENSE.

DUTLET PROTECTION

OUTLET PROTECTION SHOULD BE CHECKED AT LEAST ANNUALLY AND AFTER EVERY MAJOR STORM. IF THE RIPRAP HAS BEEN DISPLACED, UNDERMINED OR DAMAGED, IT SHOULD BE REPAIRED. THE CHANNEL IMMEDIATELY BELOW THE OUTLET SHOULD BE CHECKED TO SEE THAT EROSION IS NOT OCCURRING. THE DOWNSTREAM CHANNEL SHOULD BE KEPT CLEAR OF OBSTRUCTIONS SUCH AS FALLEN TREES, DEBRIS, AND SEDIMENT THAT COULD CHANGE FLOW PATTERNS AND/OR TAILWATER DEPTHS ON THE PIPES. REPAIRS MUST BE CARRIED OUT TO AVOID ADDITIONAL DAMAGE TO THE OUTLET PROTECTION APRON.

ROCK RIPRAP (INITIAL & LONG TERM)

ROCK RIPRAP SHOULD BE CHECKED AT LEAST ANNUALLY AND AFTER EVERY MAJOR STORM TO DETERMINE IF THE RIPRAP HAS BEEN DISPLACED, UNDERMINED OR DAMAGED. WOODY VEGETATION SHOULD BE REMOVED FROM THE ROCK RIPRAP ANNUALLY. IF THE RIPRAP IS ON A CHANNEL BANK, THE STREAM SHOULD BE KEPT CLEAR OF OBSTRUCTIONS. IF DAMAGE HAS OCCURED, REPAIRS MUST BE CARRIED OUT IMMEDIATELY TO AVOID ADDITIONAL DAMAGE TO THE RIPRAP.

STORM DRAIN INLET PROTECTION

ALL STRUCTURES SHALL BE INSPECTED AFTER SIGNIFICANT RAIN EVENTS AND REPAIRED AS NEEDED.

SEDIMENT SHALL BE REMOVED AND THE STORMDRAIN SEDIMENT BARRIER RESTORED TO ITS ORIGINAL DIMENSIONS WHEN THE SEDIMENT HAS ACCUMULATED TO 1/2 THE DESIGN DEPTH OF THE TRAP. REMOVED SEDIMENT SHALL BE DEPOSITED IN A SUITABLE AREA AND IN SUCH A MANNER THAT IT WILL NOT ERODE.

STRUCTURES SHALL BE REMOVED AND THE AREA STABILIZED WHEN THE REMAINING DRAINAGE AREA HAS BEEN PROPERLY STABILIZED.

ALL CATCHBASINS AND STORMDRAIN INLETS SHALL BE CLEANED AT THE END OF CONSTRUCTION AND AFTER THE SITE HAS BEEN FULLY STABILIZED.

STRAW OR HAY BALE BARRIER, SILT FENCE AND FILTER BERM

HAY BALE BARRIERS, SILT FENCES AND FILTER BERMS SHALL BE INSPECTED AFTER EACH RAINFALL AND AT LEAST DAILY DURING PROLONGED RAINFALL. THEY SHALL BE REPAIRED IF THERE ARE ANY SIGNS OF EROSION OR SEDIMENTATION BELOW THEM. IF THERE ARE SIGNS OF UNDERCUTTING AT THE CENTER OF THE EDGES OF THE BARRIER, OR IMPOUNDING OF LARGE VOLUMES OF WATER BEHIND THEM, SEDIMENT BARRIERS SHALL BE REPLACED WITH A TEMPORARY CHECK DAM.

SHOULD THE FABRIC ON A SILT FENCE OR FILTER BARRIER DECOMPOSE OR BECOME INEFFECTIVE PRIOR TO THE END OF THE EXPECTED USABLE LIFE AND THE BARRIER IS STILL NECESSARY, THE FABRIC SHALL BE REPLACED.

SEDIMENT DEPOSITS SHOULD BE REMOVED WHEN THE DEPOSITS REACH APPROXIMATELY ONE—HALF OF THE HEIGHT OF THE BARRIER.

FILTER BERMS SHOULD BE RESHAPED AS NEEDED.

SEDIMENT DEPOSITS REMAINING IN PLACE AFTER THE SILT FENCE OR FILTER BARRIER IS NO LONGER REQUIRED SHOULD BE DRESSED TO CONFORM TO THE EXISTING GRADE, PREPARED AND SEEDED.

TEMPORARY CHECK DAMS

REGULAR INSPECTIONS MUST BE MADE TO ENSURE THAT THE CENTER OF THE CHECK DAM IS LOWER THAT THE EDGES. EROSION CAUSED BY HIGH FLOWS AROUND THE EDGES OF THE CHECK DAM MUST BE CORRECTED. IF EVIDENCE OF SILTATION IN THE WATER IS APPARENT DOWNSTREAM OF THE CHECK DAM, THE CHECK DAM MUST BE INSPECTED AND ADJUSTED.

CHECK DAMS MUST BE CHECKED FOR SEDIMENT ACCUMULATION AFTER EACH SIGNIFICANT RAINFALL. SEDIMENT MUST BE REMOVED WHEN IT REACHES ONE HALF THE ORIGINAL HEIGHT OF BEFORE.

IF IT POSSIBLE, LEAVE THE CHECK DAM IN PLACE PERMANENTLY. IN TEMPORARY DITCHES AND SWALES, CHECK DAMS MUST BE REMOVED WHEN A PERMANENT LINING HAS BEEN ESTABLISHED. IF A CHECK DAM MUST BE REMOVED FROM A GRASS LINED DITCH, WAIT UNTIL THE GRASS HAS MATURED TO PROTECT THE DITCH OR SWALE. THE AREA BENEATH THE CHECK DAM MUST BE SEEDED AND MULCHED REMOVAL.

STABILIZED CONSTRUCTION ENTRANCE (ANTI-TRACKING PAD)

EXITS SHALL BE MAINTAINED IN A CONDITION THAT WILL PREVENT TRACKING OF SEDIMENT ONTO PUBLIC RIGHTS—OF—WAY. WHEN THE CONTROL PAD BECOMES INEFFECTIVE, THE STONE SHALL BE REMOVED ALONG WITH THE COLLECTED SOIL MATERIAL AND REDISTRIBUTED ON SITE IN A STABLE MANNER AND THE ENTRANCE RECONSTRUCTED. THE CONTRACTOR SHALL SWEEP OR WASH PAVEMENT AT EXITS, WHICH HAVE EXPERIENCED MUD—TRACKING ONTO THE PAVEMENT OR TRAVELED WAY. WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH AGGREGATE, WHICH DRAINS INTO AN APPROVED SEDIMENT TRAPPING DEVICE. ALL SEDIMENT SHALL BE PREVENTED FROM ENTERING STORM DRAINS, DITCHES OR WATERWAYS.

CUL VERTS

CULVERTS MUST BE MAINTAINED BY KEEPING INLETS, TRASH GUARDS, AND COLLECTION BOXES AND STRUCTURES CLEAN AND FREE OF MATERIALS THAT CAN REDUCE THE FLOW. ALL LEAKS SHALL BE REPAIRED TO ENSURE PROPER FUNCTIONING OF THE CULVERT. ANIMAL GUARDS MUST BE INSPECTED AND MAINTAINED IN PROPER WORKING ORDER.

ROAD DITCH TURN OUT

AFTER CONSTRUCTION, DITCH TURNOUTS SHALL BE CAREFULLY INSPECTED AND REPAIRED IF SIGNS OF CHANNELIZATION APPEAR. IT WILL BE NECESSARY TO REMOVE SEDIMENT FROM THE DITCH TURNOUT TRENCH WHEN THE SWALE IS FULL AND THE STRUCTURE IS NO LONGER FUNCTIONING PROPERLY.

VECETATED SWALE

TIMELY MAINTENANCE IS IMPORTANT TO KEEP THE VEGETATION IN THE SWALE IN GOOD CONDITION. MOWING SHOULD BE DONE FREQUENTLY ENOUGH TO KEEP THE VEGETATION IN VIGOROUS CONDITION AND TO CONTROL ENCROACHMENT OF WEEDS AND WOODY VEGETATION, HOWEVER IT SHOULD NOT BE MOWED TOO CLOSELY SO AS TO REDUCE THE FILTERING EFFECT. FERTILIZE ON AN "AS NEEDED" BASIS TO KEEP THE GRASS HEALTHY. OVER FERTILIZATION CAN RESULT IN THE SWALE BECOMING A SOURCE OF POLLUTION.

THE SWALE SHOULD BE INSPECTED PERIODICALLY AND AFTER EVERY MAJOR STORM TO DETERMINE THE CONDITION OF THE SWALE. RILLS AND DAMAGED AREAS SHOULD BE PROMPTLY REPAIRED AS NECESSARY TO PREVENT FURTHER DETERIORATION.

OVERWINTER CONSTRUCTION

MAINE EROSION AND SEDIMENT CONTROL BMP (3/2003)

THE WINTER CONSTRUCTION PERIOD IS FROM NOVEMBER 1 THROUGH APRIL 15. IF THE CONSTRUCTION SITE IS NOT STABILIZED WITH PAVEMENT, A ROAD GRAVEL BASE, 75% MATURE VEGETATION COVER OR RIPRAP BY NOVEMBER 15 THEN THE SITE NEEDS TO BE PROTECTED WITH OVER-WINTER STABILIZATION. AN AREA CONSIDERED OPEN IS ANY AREA NOT STABILIZED WITH PAVEMENT, VEGETATION, MULCHING, EROSION CONTROL MATS, RIPRAP OR GRAVEL BASE ON A ROAD.

WINTER EXCAVATION AND EARTHWORK SHALL BE COMPLETED SUCH THAT NO MORE THAN 1 ACRE OF THE SITE IS WITHOUT STABILIZATION AT ANY TIME. LIMIT THE EXPOSED AREA TO THOSE AREAS IN WHICH WORK IS EXPECTED TO BE UNDER TAKEN DURING THE PROCEEDING 15 DAYS AND THAT CAN BE MULCHED IN ONE DAY PRIOR TO ANY SNOW EVENT. ALL AREA SHALL BE CONSIDERED TO BE DENUDED UNTIL THE SUBBASE GRAVEL IS INSTALLED IN ROADWAY AREAS OR THE AREAS OF FUTURE LOAM AND SEED HAVE BEEN LOAMED, SEEDED AND MULCHED. A COVER OF EROSION CONTROL MIX PERFORMS THE BEST.

ANY ADDED MEASURES, WHICH MAY BE NECESSARY TO CONTROL EROSION/SEDIMENTATION, MUST BE BE INSTALLED. THESE MAY BE DEPENDENT UPON SITE AND WEATHER CONDITIONS AND THE ACTUAL SITE SIZE. TO MINIMIZE AREAS WITHOUT EROSION CONTROL PROTECTION, CONTINUATION OF EARTHWORK OPERATIONS ON ADDITIONAL AREAS SHALL NOT BEGIN UNTIL THE EXPOSED SOIL SURFACE ON THE AREA BEING WORKED HAS BEEN STABILIZED.

1. NATURAL RESOURCES PROTECTION

ANY AREAS WITHIN 100 FEET FROM ANY NATURAL RESOURCE, IF NOT STABILIZED WITH A MINIMUM OF 75% MATURE VEGETATION CATCH, SHALL BE MULCHED BY DECEMBER 1 AND ANCHORED WITH PLASTIC NETTING OR PROTECTED WITH AN EROSION CONTROL COVER.

DURING WINTER CONSTRUCTION, A DOUBLE ROW OF SEDIMENT BARRIERS (I.E. SILT FENCE BACKED WITH HAY BALES OR EROSION CONTROL MIX) WILL BE PLACED BETWEEN ANY NATURAL RESOURCE AND THE DISTURBED AREA. PROJECTS CROSSING THE NATURAL RESOURCE SHALL BE PROTECTED A MINIMUM DISTANCE OF 100 FEET ON EITHER SIDE FROM THE RESOURCE. EXISTING PROJECTS NOT STABILIZED BY DECEMBER 1 SHALL BE PROTECTED WITH THE SECOND LINE OF SEDIMENT BARRIER TO ENSURE FUNCTIONALITY DURING THE SPRING THAW AND RAINS.

2. SEDIMENT BARRIERS

DURING FROZEN CONDITIONS, SEDIMENT BARRIERS MAY CONSIST OF EROSION CONTROL MIX BERMS OR ANY OTHER RECOGNIZED SEDIMENT BARRIERS AS FROZEN SOIL PREVENTS THE PROPER INSTALLATION OF HAY BALES AND SEDIMENT SILT FENCES.

3. MULCHING

ALL AREA SHALL BE CONSIDERED TO BE DENUDED UNTIL SEEDED AND MULCHED. HAY AND STRAW MULCH SHALL BE APPLIED AT A RATE OF 150 LB. PER 1,000 SQUARE FEET OR 3 TONS/ACRE (TWICE THE NORMAL ACCEPTED RATE OF 75 LBS./1,000 S.F. OR 1.5 TONS/ACRE) AND SHALL BE PROPERLY ANCHORED. EROSION CONTROL MIX MUST BE APPLIED WITH A MINIMUM 4 INCH THICKNESS.

MULCH SHALL NOT BE SPREAD ON TOP OF SNOW. THE SNOW WILL BE REMOVED DOWN TO A ONE INCH

DEPTH OR LESS PRIOR TO APPLICATION.

AFTER EACH DAY OF FINAL GRADING, THE AREA WILL BE PROPERLY STABILIZED WITH ANCHORED HAY OR

STRAW OR EROSION CONTROL MATTING.

AN AREA SHALL BE CONSIDERED TO HAVE BEEN STABILIZED WHEN EXPOSED SURFACES HAVE BEEN EITHER MULCHED OR ADEQUATELY ANCHORED SO THAT GROUND SURFACE IS NOT VISIBLE THOUGH THE MULCH. BETWEEN THE DATES OF NOVEMBER 1 AND APRIL 15, ALL MULCH SHALL BE ANCHORED BY EITHER MULCH NETTING, ASPHALT EMULSION CHEMICAL, TRACKING OR WOOD CELLULOSE FIBER. THE COVER WILL BE CONSIDERED SUFFICIENT WHEN THE GROUND SURFACE IS NOT VISIBLE THROUGH THE MULCH. AFTER NOVEMBER 1ST, MULCH AND ANCHORING OF ALL EXPOSED SOIL SHALL OCCUR AT THE END OF EACH

4. SOIL STOCKPILES

FINAL GRADING WORKDAY.

STOCKPILES OF SOIL OR SUBSOIL WILL BE MULCHED FOR OVER WINTER PROTECTION WITH HAY OR STRAW AT TWICE THE NORMAL RATE OR WITH A FOUR-INCH LAYER OF EROSION CONTROL MIX. THIS WILL BE DONE WITHIN 24 HOURS OF STOCKING AND REESTABLISHED PRIOR TO ANY RAINFALL OR SNOWFALL. ANY SOIL STOCKPILE WILL NOT BE PLACED (EVEN COVERED WITH MULCH) WITHIN 100 FEET FROM ANY NATURAL RESOURCES.

SEEDIN

BETWEEN THE DATES OF OCTOBER 15 AND APRIL 1, LOAM OR SEED WILL NOT BE REQUIRED. DURING PERIODS OF ABOVE FREEZING TEMPERATURES FINISHED AREAS SHALL BE FINE GRADED AND EITHER PROTECTED WITH MULCH OR TEMPORARILY SEEDED AND MULCHED UNTIL SUCH TIME AS THE FINAL TREATMENT CAN BE APPLIED. IF THE DATE IS NOVEMBER 1ST AND IF THE EXPOSED AREA HAS BEEN LOAMED, FINAL GRADED WITH A UNIFORM SURFACE, THEN THE AREA MAY BE DORMANT SEEDED AT A RATE OF 3 TIMES HIGHER THAN SPECIFIED FOR PERMANENT SEED AND THEN MULCHED.

DORMANT SEEDING MAY BE SELECTED TO BE PLACED PRIOR TO THE PLACEMENT OF MULCH AND EROSION CONTROL BLANKETS. IF DORMANT SEEDING IS USED FOR THE SITE, ALL DISTURBED AREAS SHALL RECEIVE 4' OF LOAM AND SEED AT AN APPLICATION RATE OF 5LBS/1,000 S.F. ALL AREAS SEEDED DURING THE WINTER WILL BE INSPECTED IN THE SPRING FOR ADEQUATE CATCH. ALL AREAS INSUFFICIENTLY VEGETATED (LESS THAN 75% CATCH) SHALL BE REVEGETATED BY REPLACING LOAM, SEED AND MULCH.

IF DORMANT SEEDING IS NOT USED FOR THE SITE, ALL DISTURBED AREAS SHALL BE REVEGETATED IN THE SPRING.

OVERWINTER STABILIZATION

MAINE EROSION AND SEDIMENT CONTROL BMP (3/2003)

1. STABILIZATION OF DITCHES AND CHANNELS

ALL STONE-LINED DITCHES AND CHANNELS MUST BE CONSTRUCTED AND STABILIZED BY NOVEMBER 15. ALL GRASS-LINED DITCHES AND CHANNELS MUST BE CONSTRUCTED AND STABILIZED BY SEPTEMBER 1. IF A DITCH OR CHANNEL IS NOT GRASS-LINED BY SEPTEMBER 1, THEN ONE OF THE FOLLOWING ACTIONS TO STABILIZE THE DITCH FOR LATE FALL AND WINTER MUST BE TAKEN.

SOD LINING: A DITCH OR CHANNEL MUST BE LINED WITH PROPERLY INSTALLED SOD BY OCTOBER 1. PROPER INSTALLATION INCLUDES: PINNING THE SOD ONTO THE SOIL WITH WIRE PINS, ROLLING THE SOD TO GUARANTEE CONTACT BETWEEN THE SOD AND UNDERLYING SOIL, WATERING THE SOD TO PROMOTE ROOT GROWTH INTO THE DISTURBED SOIL, AND ANCHORING THE SOD AT THE BASE OF THE DITCH WITH JUTE OR PLASTIC MESH TO PREVENT THE SOD FROM SLOUGHING DURING FLOW CONDITIONS.

STONE LINING: A DITCH OR CHANNEL MUST BE LINED WITH STONE RIPRAP BY NOVEMBER 15. A REGISTERED PROFESSIONAL ENGINEER MUST DETERMINE THE STONE SIZE AND LINING THICKNESS NEEDED TO WITHSTAND THE ANTICIPATED FLOW VELOCITIES AND FLOW DEPTHS WITHIN THE DITCH. IF NECESSARY, THE CONTRACTOR WILL REGRADE THE DITCH PRIOR TO PLACING THE STONE LINING TO PREVENT THE STONE LINING FROM REDUCING THE DITCH'S CROSS—SECTIONAL AREA.

2. STABILIZATION OF DISTURBED SLOPES

ALL STONE—COVERED SLOPES MUST BE CONSTRUCTED AND STABILIZED BY NOVEMBER 15. ALL SLOPES TO BE VEGETATED MUST BE SEEDED AND MULCHED BY SEPTEMBER 1. THE DEPARTMENT WILL CONSIDER ANY AREA HAVING A GRADE GREATER THAN 15% TO BE A SLOPE. IF A SLOPE TO BE VEGETATED IS NOT STABILIZED BY SEPTEMBER 1, THEN ONE OF THE FOLLOWING ACTIONS MUST BE TAKEN TO STABILIZE THE SLOPE FOR LATE FALL AND WINTER.

TEMPORARY VEGETATION AND EROSION CONTROL MATS: BY OCTOBER 1, THE DISTURBED SLOPE MUST BE SEEDED WITH WINTER RYE AT A SEEDING RATE OF 3 POUNDS PER 1,000 SQUARE FEET FOLLOWED BY INSTALLATION OF EROSION CONTROL MATS OR ANCHORED MULCH OVER THE SEEDING. IF THE RYE FAILS TO GROW AT LEAST THREE INCHES OR FAILS TO COVER AT LEAST 75% OF THE SLOPE BY NOVEMBER 1, THEN THE CONTRACTOR WILL COVER THE SLOPE WITH A LAYER OF EROSION CONTROL MIX OR STONE RIPRAP AS DESCRIBED IN THE FOLLOWING STANDARDS.

SOD: THE DISTURBED SLOPE MUST BE STABILIZED WITH PROPERLY INSTALLED SOD BY OCTOBER 1. PROPER INSTALLATION INCLUDES THE CONTRACTOR PINNING THE SOD ONTO THE SLOPE WITH WIRE PINS, ROLLING THE SOD TO GUARANTEE CONTACT BETWEEN THE SOD AND UNDERLYING SOIL, AND WATERING THE SOD TO PROMOTE ROOT GROWTH INTO THE DISTURBED SOIL. THE CONTRACTOR WILL NOT USE LATE—SEASON SOD INSTALLATION TO STABILIZE SLOPES HAVING A GRADE GREATER THAN 33% (3H:1V) OR HAVING GROUNDWATER SEEPS ON THE SLOPE FACE.

EROSION CONTROL MIX: EROSION CONTROL MIX MUST BE PROPERLY INSTALLED BY NOVEMBER 15. THE CONTRACTOR WILL NOT USE EROSION CONTROL MIX TO STABILIZE SLOPES HAVING GRADES GREATER THAT 50% (2H:1V) OR HAVING GROUNDWATER SEEPS ON THE SLOPE FACE.

STONE RIPRAP: PLACE A LAYER OF STONE RIPRAP ON THE SLOPE BY NOVEMBER 15. THE DEVELOPMENT'S OWNER WILL HIRE A REGISTERED PROFESSIONAL ENGINEER TO DETERMINE THE STONE SIZE NEEDED FOR STABILITY ON THE SLOPE AND TO DESIGN A FILTER LAYER TO BE INSTALLED BENEATH THE RIPRAP.

3. STABILIZATION OF DISTURBED SOILS

TEMPORARY VEGETATION: BY OCTOBER 1, SEED THE DISTURBED SOIL WITH WINTER RYE AT A SEEDING RATE OF 3-LBS PER 1,000 SQUARE FEET, LIGHTLY MULCH THE SEEDED SOIL WITH HAY OR STRAW AT 75-LBS PER 1,000 SQUARE FEET, AND ANCHOR THE MULCH WITH PLASTIC NETTING. MONITOR GROWTH OF THE RYE OVER THE NEXT 30 DAYS. IF THE RYE FAILS GROW AT LEAST THREE INCHES OR COVER AT LEAST 75% OF THE DISTURBED SOIL BEFORE NOVEMBER 1, THEN MULCH THE AREA FOR OVERWINTER PROTECTION AS FOLLOWS.

MULCH: BY NOVEMBER 15, MULCH THE DISTURBED SOIL BY SPREADING HAY OR STRAW AT A RATE OF AT LEAST 150-LBS PER 1,000 SQUARE FEET ON THE AREA SO THAT NO SOIL IS VISIBLE THROUGH THE MULCH. IMMEDIATELY AFTER APPLYING THE MULCH, ANCHOR THE MULCH WITH PLASTIC NETTING TO PREVENT WIND FROM MOVING THE MULCH OFF THE DISTURBED SOIL.

MAINTENANCE

MAINTENANCE MEASURES SHALL BE APPLIED AS NEEDED DURING THE ENTIRE CONSTRUCTION SEASON. AFTER EACH RAINFALL, SNOW STORM OR PERIOD OF THAWING AND RUNOFF, THE SITE CONTRACTOR SHALL PERFORM A VISUAL INSPECTION OF ALL INSTALLED EROSION CONTROL MEASURES AND PERFORM REPAIRS AS NEEDED TO INSURE THEIR CONTINUOUS FUNCTION. FOLLOWING THE TEMPORARY AND OR FINAL SEEDING AND MULCHING, THE CONTRACTOR SHALL IN THE

SPRING INSPECT AND REPAIR ANY DAMAGES AND/OR BARE SPOTS. AN ESTABLISHED VEGETATIVE COVER MEANS A MINIMUM OF 85% TO 90% OF AREAS VEGETATED WITH VIGOROUS GROWTH.

STABILIZATION SCHEDULE BEFORE WINTER

SEPTEMBER 15 ALL DISTURBED AREAS MUST BE SEEDED AND MULCHED.

ALL SLOPES MUST BE STABILIZED, SEEDED AND MULCHED.

ALL GRASS—LINED DITCHES AND CHANNELS MUST BE STABILIZED WITH

MULCH OR AN EROSION CONTROL BLANKET.

OCTOBER 1 IF THE SLOPE IS STABILIZED WITH AN EROSION CONTROL BLANKET AND SEEDED.

ALL DISTURBED AREAS TO BE PROTECTED WITH ANNUAL GRASS MUST BE SEEDED

AT A SEEDING RATE OF 3-LBS PER 1,000 SQUARE FEET AND MULCHED.

NOVEMBER 15 ALL STONE-LINED DITCHES AND CHANNELS MUST BE CONSTRUCTED AND STABILIZED.

SLOPES THAT ARE COVERED WITH RIPRAP MUST BE CONSTRUCTED BY THIS DATE.

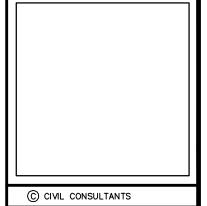
DECEMBER 1 ALL DISTURBED AREAS WHERE GROWTH OF VEGETATION FAILS TO BE AT LEAST

THREE INCHES TALL OR AT LEAST 75% OF THE DISTURBED SOIL IS COVERED BY VEGETATION, MUST BE PROTECTED FOR OVER—WINTER.

NOTE: THE DATES GIVEN ARE FOR PROJECTS IN SOUTH—CENTRAL MAINE.

PROJECT MAINTENANCE ITEMS:

THE DEVICES THAT WILL REQUIRE MAINTENANCE FOR THIS PROJECT ARE: SILT FENCE AND LEVEL SPREADERS



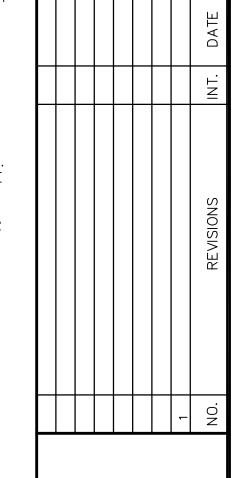


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EN PIGS FLY

DUTE 1/ LEWIS ROA

TTERY, MAINE

PREPARED FOR:

W AND RON SIEGEL

WHEN
460 US ROUT
KITTEF
PRE
ANDREW A



DRAWN JAA CALC.

DATE 7 SEPTEMBER 2010

CHECKED

APPROVED

SCALE NOT TO SCALE

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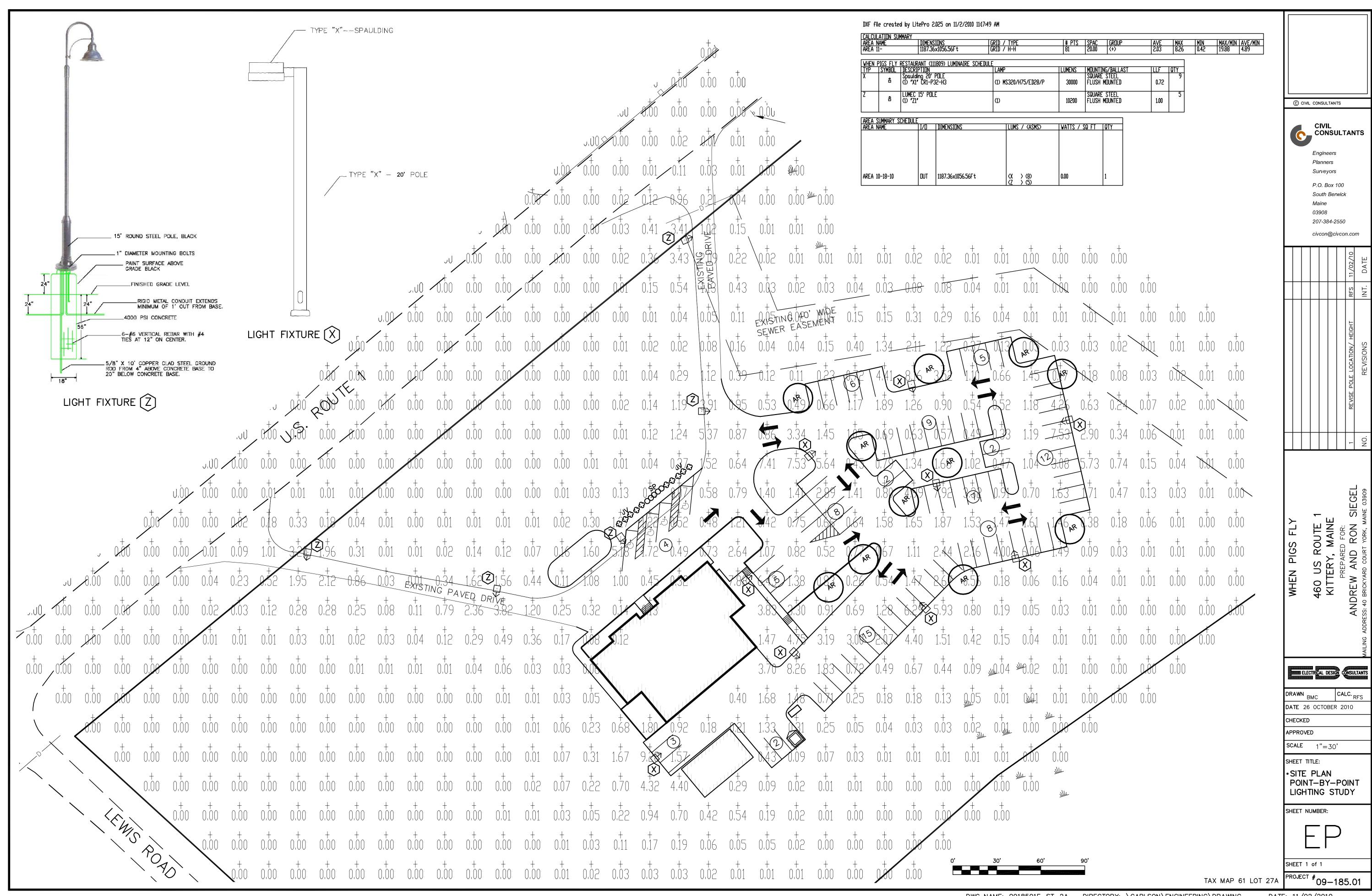
• CONSTRUCTION DETAILS

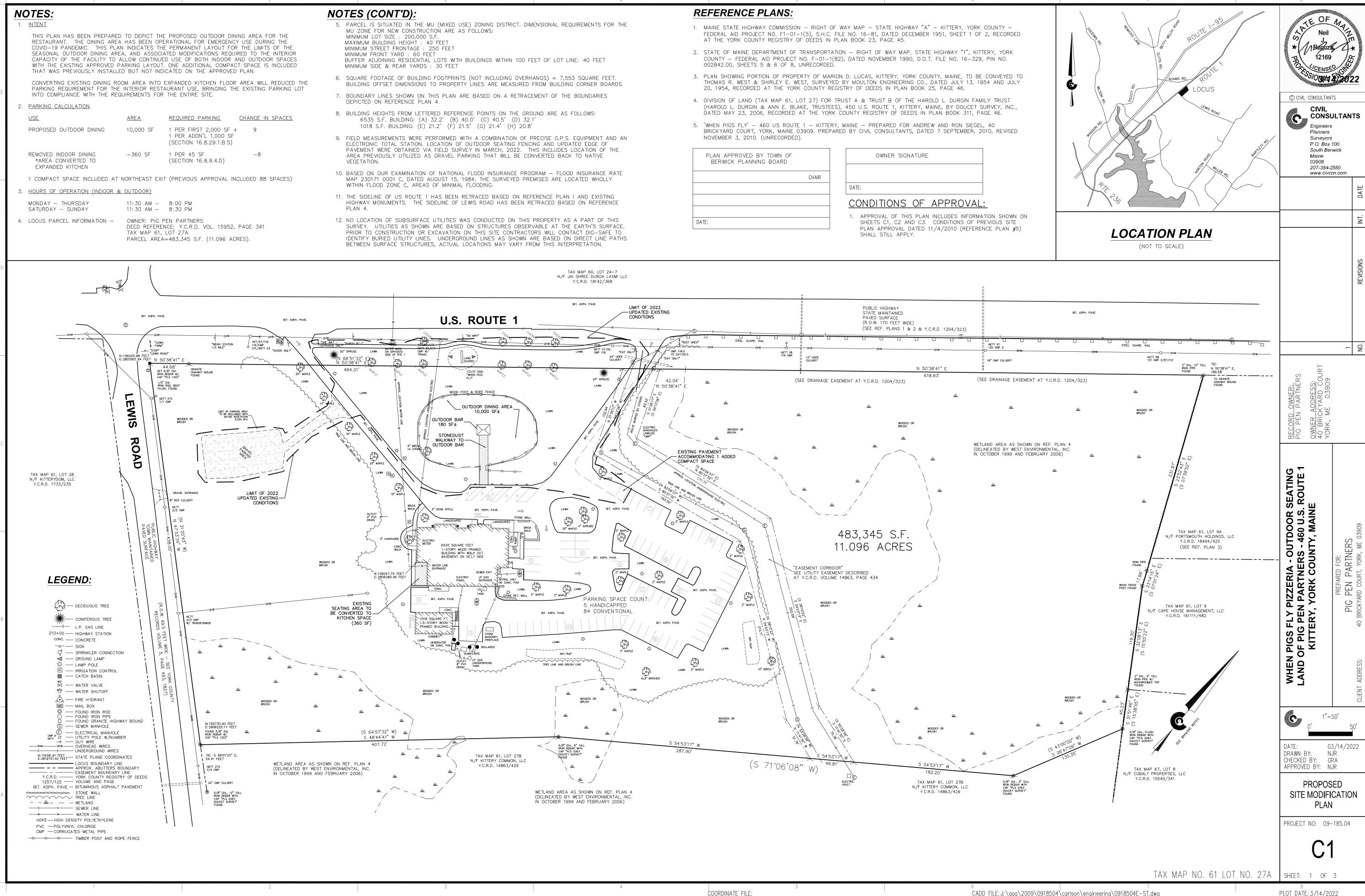
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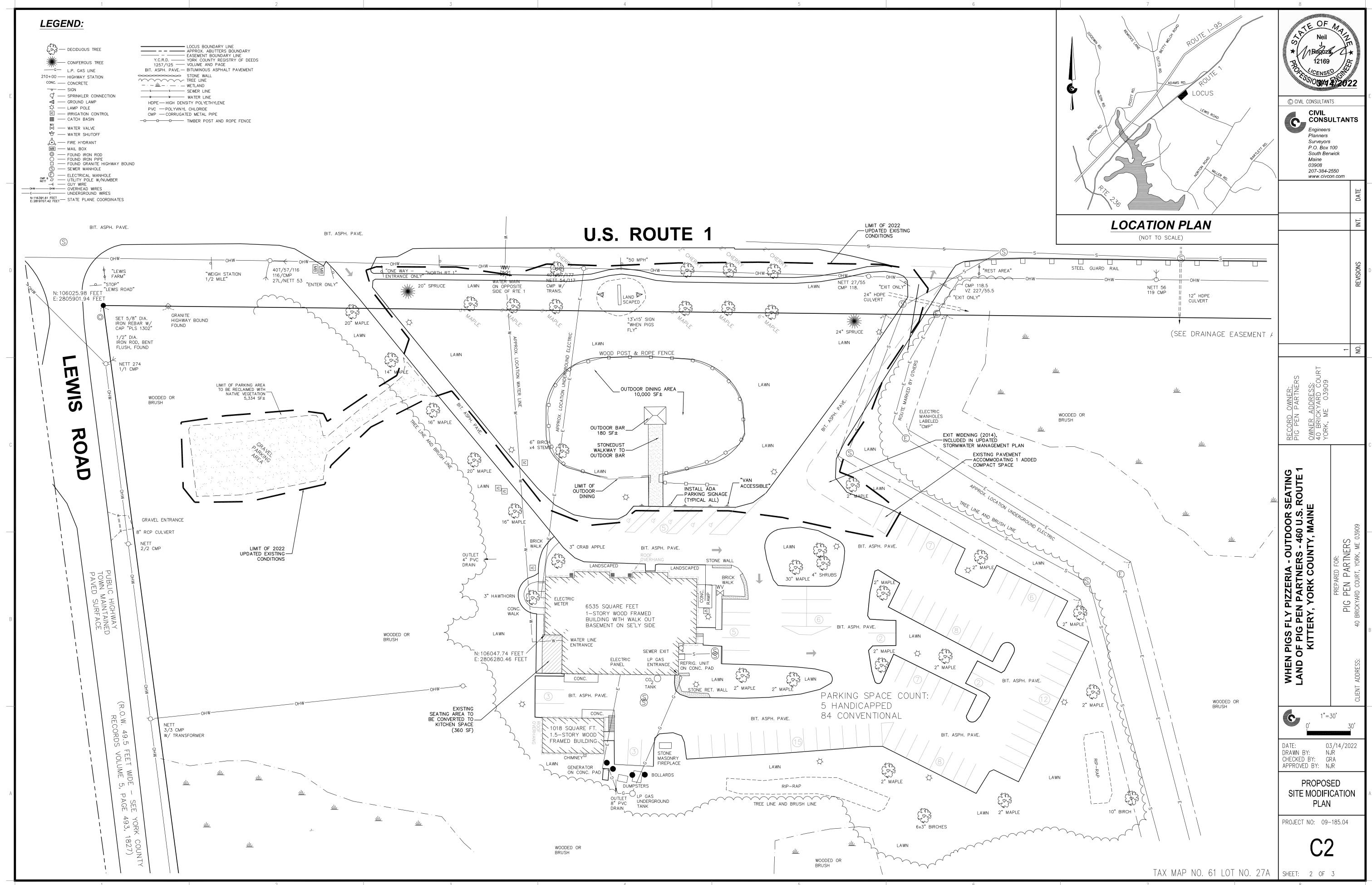
SHEET C5 of

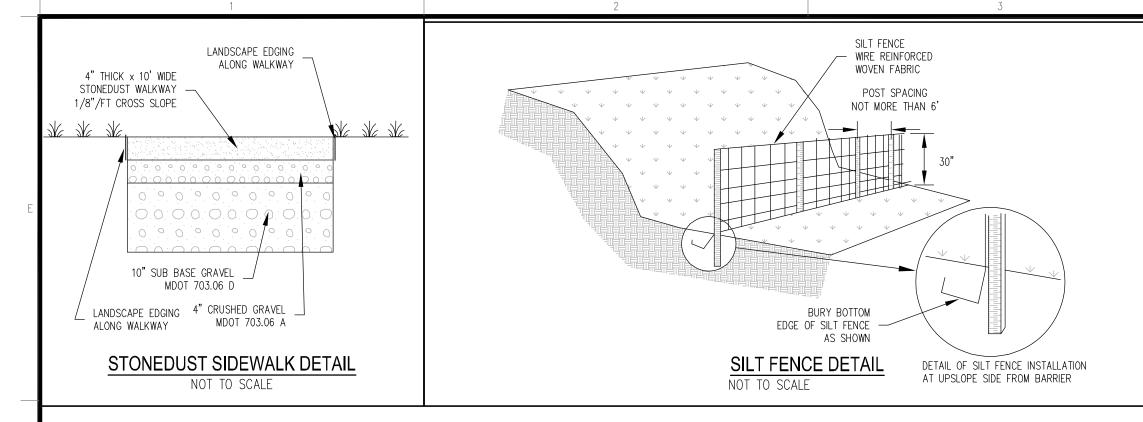
DATE: 09/07/2010

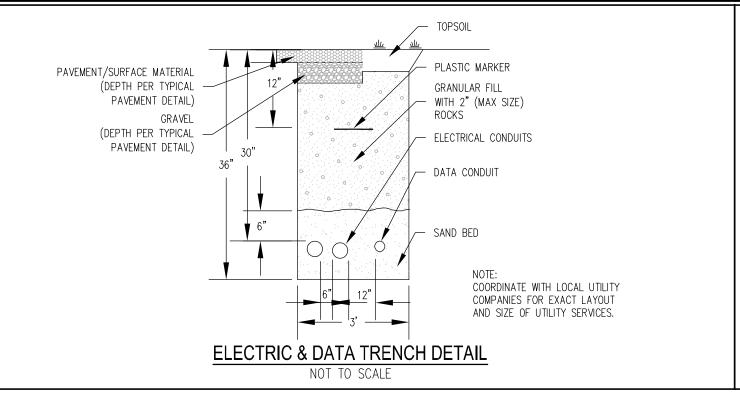
TAX MAP 61 LOT 27A PROJECT #09-185.01









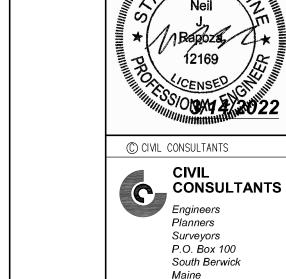




HANDICAP VAN ACCESIBLE SIGN

NOT TO SCALE





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03908

SEATING . ROUTE 1 IE WHEN PIGS FLY PIZZERIA - OUTDOOR LAND OF PIG PEN PARTNERS - 460 U.S KITTERY, YORK COUNTY, MAIN

NOT TO SCALE

DRAWN BY: NJŔ CHECKED BY: GRA

APPROVED BY: NJR

NOTES AND **DETAILS**

PROJECT NO: 09-185.04

TAX MAP NO. 61 LOT NO. 27A | SHEET: 3 OF 3

EROSION AND SEDIMENT CONTROL PRACTICES

- 1. NO SOIL SHALL BE DISTURBED DURING THE PERIOD OF MARCH 1 THROUGH APRIL 15, NOR DURING ANY OTHER PERIOD WHEN SOILS ARE SATURATED DUE TO RAIN OR SNOW MELT, UNLESS DIRECTED BY ENGINEER.
- 2. DISTURBED SOILS SHALL BE STABILIZED WITHIN ONE (1) WEEK FROM THE TIME IT WAS LAST ACTIVELY WORKED USING TEMPORARY OR PERMANENT MEASURES SUCH AS PLACEMENT OF RIPRAP, MULCH OR EROSION CONTROL BLANKET, OR OTHER COMPARABLE MEASURES.
- 3. HAY OR STRAW MULCH SHALL BE APPLIED AT A RATE OF AT LEAST ONE (1) BALE PER 500 SQUARE FEET (1-2 TONS PER ACRE).
- SHALL BE ANCHORED WITH NETTING, PEG OR TWINE, OR OTHER SUITABLE METHOD AND SHALL BE MAINTAINED UNTIL A CATCH OF VEGETATION IS ESTABLISHED OVER THE ENTIRE DISTURBED AREA.
- 5. IN ADDITION TO PLACEMENT OF RIPRAP, MULCH OR EROSION CONTROL BLANKETS, ADDITIONAL STEPS SHALL BE TAKEN WHERE NECESSARY IN ORDER TO PREVENT SEDIMENTATION OF THE WATER. EVIDENCE OF SEDIMENTATION INCLUDES VISIBLE GULLY EROSION, DISCOLORATION OF WATER BY SUSPENDED PARTICLES AND SLUMPING OF BANKS, SILT FENCES, STAKED HAY BALES AND OTHER SEDIMENTATION CONTROL MEASURES, WHERE PLANNED FOR, SHALL BE IN PLACE PRIOR TO COMMENCEMENT OF WORK, BUT SHALL ALSO BE INSTALLED WHEREVER NECESSARY DUE TO SEDIMENTATION.

- 6. MULCH OR OTHER TEMPORARY MEASURES SHALL BE MAINTAINED UNTIL THE SITE IS PERMANENTLY STABILIZED WITH VEGETATION OR OTHER PERMANENT CONTROL MEASURES AFTER WHICH TEMPORARY MEASURES WILL BE REMOVED.
- 7. PERMANENT RE-VEGETATION OF ALL DISTURBED AREAS, USING NATIVE PLANT MATERIAL WHEN POSSIBLE, SHALL OCCUR WITHIN 30 DAYS FROM THE TIME THE AREAS WERE LAST ACTIVELY WORKED, OR FOR FALL AND WINTER ACTIVITIES, BY JUNE 15, EXCEPT WHERE PRECLUDED BY THE TYPE OF ACTIVITY (E.G. RIRRAP, ROAD SURFACES, ETC.). THE VEGETATIVE COVER SHALL BE MAINTAINED.
- 4. IF MULCH IS LIKELY TO BE REMOVED DUE TO TO STEEP SLOPES OR WIND, IT 8. DISPOSAL OF COLLECTED DEBRIS MUST BE IN CONFORMANCE WITH MAINE SOLID WASTE LAW, TITLE 38 MRSA SECTION 1301 ET. SEQ.
 - 9. LIME AND FERTILIZER APPLICATION RATES SHALL NOT EXCEED THE FOLLOWING:

GROUND LIMESTONE: 3 TONS/ACRE (130 LBS./1000 S.F.) FERTILIZER, 10-10-10 OF EQUIVALENT: 600 LBS./ACRE (14 LBS./1000 S.F.)

FERTILIZER SHALL NOT BE APPLIED BEFORE START OF THE GROWING SEASON NOR AFTER SEPTEMBER 30. FERTILIZED AREAS SHALL BE MULCHED TO REDUCE OFF-SITE TRANSPORT OF NUTRIENTS UNTIL USED BY VEGETATIVE GROWTH.

SEEDING MIXTURE AND SCHEDULE:

SPREAD TOPSOIL UNIFORMLY 6" DEEP OVER AREAS TO BE RECLAIMED. THE FOLLOWING SEED MIXTURE SHALL BE USED:

LAWNS:

KENTUCKY BLUEGRASS 1.60 LBS./1000 S.F. PERENNIAL RYE GRASS 0.40 LBS./1000 S.F. TOTAL 2.00 LBS./1000 S.F.

APPLY LIME AND FERTILIZER AS SPECIFIED UNDER THE EROSION AND SEDIMENTATION CONTROL NOTES. WORK INTO THE TOP (4) INCHES OF SOIL PRIOR TO SEEDING. AFTER SEEDING, APPLY MULCH HAY AS SPECIFIED. ON FLAT AREAS AND NOT EXPOSED TO WIND, THE MULCH WILL BE ANCHORED BY WETTING DOWN. IN OTHER AREAS, JUTE NETTING SHALL BE USED FOR ANCHORAGE. THE ABOVE SEEDING SCHEDULE IS APPLICABLE IF SEEDING DURING THE GROWING SEASON (APRIL 15 TO JUNE 15 AND AUGUST 30 TO SEPTEMBER 30). BETWEEN JUNE 15 AND AUGUST 30, SEEDING WILL BE DELAYED UNTIL AUGUST 30. IF SOIL IS DISTURBED BETWEEN OCTOBER 1 AND NOVEMBER 1, DELAY SEEDING UNTIL NOVEMBER 1. AFTER NOVEMBER 1 AND BEFORE A SNOW COVER FORMS, THE SAME PROCEDURE WILL BE FOLLOWED EXCEPT THE SEED RATE WILL BE DOUBLED. AFTER SNOW COVER AND BEFORE APRIL 15, SEEDING WILL BE DELAYED UNTIL APRIL 15. HAY MULCH WILL BE APPLIED AT A RATE OF 150 LBS./1000 SQUARE FEET. THIS WILL BE ANCHORED BY NON-ASPHALTIC TACKIFIER SPRAYED ON LAWNS AND JUTE NETTING IN DRAINAGE WAYS AND OTHER AREAS.

