CMA ENGINEERS, INC. CIVIL | ENVIRONMENTAL | STRUCTURAL

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CMA

August 23, 2023

Jason Garnham, Director of Planning and Development Town of Kittery 200 Rogers Rd. Kittery, ME 03904

RE: Town of Kittery, Planning Board Services
 35 Badgers Island West Residential Conversion Drainage Review #3
 Tax Map 1, Lot 32
 CMA #591.156

Jason:

CMA Engineers has received the following information for Assignment #156, review #3 of the plans and drainage analysis for a redevelopment at 35 Badgers Island West.

- 1) Drainage Analysis, Site Development, 35 Badgers Island West, Kittery, Maine, Prepared for Hampshire Development by Ambit Engineering, dated January 19, 2023 and Amended July 26, 2023.
- Residential Conversion, 35 Badgers Island West, Kittery, Maine, Amended Site Plan, Preliminary Plan Application and supporting documentation dated January 1, 2023 and last revised August 3, 2023.
- 3) Response letter to CMA Engineers' comments dated July 13, 2023. We note that the response letter appears to have been generated prior to the current set of plans and does not reflect entirely accurate responses.

The proposed project includes two building additions and a change in use to residential at 35 Badgers Island West, Tax Map 1, Lot 32 in the Mixed-Use Badgers Island Zoning District (MU-BI), the Resource Protection Overlay (OZ-RP) and the Shoreland Overlay Zone (OZ-SL-250'). The application has been prepared by Ambit Engineering, Inc. of Portsmouth, NH on behalf of the owner B.I.W. Group, LLC.

Previously, a revetment repair was approved and completed with the exception of the landscaping installation. The project includes two proposed additions to the existing office building with heated driveways and a change in use to residential. Ten residential units are proposed. Access is from Badgers Island West. The development is located on a 1.35-acre lot. Utilities are proposed to be installed underground. Water supply is proposed off a Kittery Water District (KWD) main from Badgers Island West to the north and wastewater disposal is proposed off a Town sewer main also from Badgers Island West to the south.

We have the following comments that relate to the plans:

<u>Sheet C1 – Existing Conditions Plan</u>

- 1. Label the size and material of the water services and water main.
- 2. Is the 6' "ceptic" easement required to be maintained? It is not shown on any subsequent plans.

Sheet C2 – Shoreland Development Plan

- 1. It is not clear why the porous pavement walkway is an option and not definitively chosen or not.
- 2. The previous buffer planting area shows symbols that are not defined in a legend. The area could be hatched, or the symbols defined.

<u>Sheet C3 – Utility Plan</u>

- 1. The proposed sewer service contains a change in pipe direction at a three-foot drop in elevation and is serviced by a cleanout. This is not a conventional design and has the potential to easily become clogged. The design should include a manhole with a drop inlet.
- 2. The invert of the pipe at the cleanouts is labeled as 14.0' but 13.56' in the Sewer Plan and Profile on Sheet D4.

<u>Sheet C4 – Grading Plan</u>

- 1. The applicant is now proposing to replace DMH #1657, which is located in the Town right-of-way. Is this acceptable to the department of public works? What are the details of the pipe connections?
- 2. Amend the leader "Pipe to be terminated at DMH 1657..." The manhole is being replaced. The pipe should be removed or filled in and capped to the roof leader tie-in point.
- 3. The new 18" pipe at DMH 1657 is called out as 18" CPP on the drainage structure table. Is this correct or should it be HDPE? Is the existing 12" pipe in HDPE or CPP?

<u>Sheet C5 – Demolition Plan</u>

- 1. The leader "Tree line to be cut back to landscape plan limits" should be removed or clarified. There is no cutting back of the tree line shown on the landscape plan.
- 2. Call out DMH #1657 to be removed.

<u>Sheet D4 – Details</u>

- 1. The proposed sewer service contains a change in pipe direction at a three-foot drop in elevation and is serviced by a cleanout. This is not a conventional design and has the potential to easily become clogged. The design should include a manhole with a drop inlet.
- 2. The invert of the pipe at the cleanouts is labeled as 13.56' in the Sewer Plan and Profile but 14.0' on Sheet C3.

The lighting plan has been removed from the plan set. Is there no outside lighting proposed? The response letter indicates that the information previously requested "will be provided. Lighting will be building mounted. A significant number of lights will be in ceiling areas." Please clarify.

We have the following comments on the drainage analysis:

- 1. All nodes shown on the pre- and post-construction routing diagrams should be shown on their respective subcatchment plans.
- 2. The pipe at CB 1 is modelled as 24" but is shown as 18" on the plans. The invert is modelled as 7.63' but shown as 7.53' on the plans. Please clarify.
- 3. Proposed piping is modelled as CMP (with square edged headwalls) when the plans call for HDPE pipe. Please clarify.
- 4. The Inspection & Long-Term Maintenance Plan should be updated to remove permeable pavers since these are not shown on the plans.



Should you have any questions, please do not hesitate to call.

Very truly yours, CMA ENGINEERS, INC.

Jodie Bray Strickland, P.E. Project Manager

cc: John Chagnon, P.E., Ambit Engineering





200 Griffin Road, Unit 3, Portsmouth, NH 03801 Phone (603) 430-9282 Fax 436-2315

24 August 2023

Jason Garnham, Director of Planning and Development Town of Kittery 200 Rogers Road, Kittery, ME 03904

Re: Preliminary Site Plan Review Application; Conversion to Residential Tax Map 1, Lot 32, 35 Badgers Island West <u>Response to CMA Comments</u>

Dear Jason:

On behalf of BIW Inc. we hereby submitted revised plans and supporting material for **Final Site Plan Review Approval** to address the comments from the August 23, 2023, review letter from CMA Engineers. The specific comments, repeated below with our responses in **bold** text, are as follows:

Sheet C1 – Existing Conditions Plan

1. Label the size and material of the water services and water main. There is an 8-inch sprinkler line servicing the building. Currently the domestic is tapped from that line inside the building. This is not an uncommon arrangement and has operated and has been approved. A new domestic line will be run, if required. The water services will be fully coordinated with the Water District.

2. Is the 6' "ceptic" easement required to be maintained? It is not shown on any subsequent plans. No, that is an old deed reference, no pipe was installed in that location, and no septic pipe will ever be installed in that location.

Sheet C2 – Shoreland Development Plan

1. It is not clear why the porous pavement walkway is an option and not definitively chosen or not. We would like to install that as porous but at this time did not want to do the destructive testing to ensure that the subsurface soil conditions were suitable. It's calculated as impervious in the chart. If a decision has to be made at this time we will.

2. The previous buffer planting area shows symbols that are not defined in a legend. The area could be hatched, or the symbols defined. The note labels it as "Previous Buffer Planting Area" – See Revised Plan. The Revised Plan has an entirely new design for that area; so, we can just delete that from the plan. We just wanted the Board to be able to see that we were not impacting a previously design that was not yet constructed on a portion of the site.

Sheet C3 – Utility Plan

1. The proposed sewer service contains a change in pipe direction at a three-foot drop in elevation and is serviced by a cleanout. This is not a conventional design and has the

potential to easily become clogged. The design should include a manhole with a drop inlet. Please see attached a PVC manhole to address the concern. Once approved we will update the plans.

2. The invert of the pipe at the cleanouts is labeled as 14.0' but 13.56' in the Sewer Plan and Profile on Sheet D4. Those numbers are correct. The <u>14.0 is the invert out at the building</u>, consistent on both sheets.

Sheet C4 - Grading Plan

1. The applicant is now proposing to replace DMH #1657, which is located in the Town rightof-way. Is this acceptable to the department of public works? What are the details of the pipe connections? **The reviewer previously commented...** *"Has the condition of the existing structures (namely DMH 1657) been assessed? What size is DMH 1657? Is there room in the structure for two additional pipe penetrations?"* This was a good comment as the review indicated that the DMH needed to be replaced.

2. Amend the leader "Pipe to be terminated at DMH 1657..." The manhole is being replaced. The pipe should be removed or filled in and capped to the roof leader tie-in point. **Done, the pipe will be removed.**

3. The new 18" pipe at DMH 1657 is called out as 18" CPP on the drainage structure table. Is this correct or should it be HDPE? Is the existing 12" pipe in HDPE or CPP? **HDPE**.

Sheet C5 – Demolition Plan

1. The leader "Tree line to be cut back to landscape plan limits" should be removed or clarified. There is no cutting back of the tree line shown on the landscape plan. Both the C5 Plan and the L1 Landscape Plan have been clarified.

2. Call out DMH #1657 to be removed. **Done.**

Sheet D4 – Details

1. The proposed sewer service contains a change in pipe direction at a three-foot drop in elevation and is serviced by a cleanout. This is not a conventional design and has the potential to easily become clogged. The design should include a manhole with a drop inlet. Please see attached a PVC manhole to address the concern. Once approved we will update the plans.

2. The invert of the pipe at the cleanouts is labeled as 13.56' in the Sewer Plan and Profile but 14.0' on Sheet C3. Those numbers are correct. The <u>14.0 is the invert out at the building</u>, consistent on both sheets.

The lighting plan has been removed from the plan set. Is there no outside lighting proposed? The response letter indicates that the information previously requested "will be provided. Lighting will be building mounted. A significant number of lights will be in ceiling areas." Please clarify. **The Lighting Plan is Sheet 3 of the Architectural Package – page 113 of the digital Planning Board Packet.**

We have the following comments on the drainage analysis:

1. All nodes shown on the pre- and post-construction routing diagrams should be shown on their respective subcatchment plans. **Done.**

2. The pipe at CB 1 is modelled as 24" but is shown as 18" on the plans. The invert is modelled as 7.63' but shown as 7.53' on the plans. Please clarify. **The 24" pipe has been changed to an 18" pipe in the model, and the elevations lowered.**

3. Proposed piping is modelled as CMP (with square edged headwalls) when the plans call for HDPE pipe. Please clarify. **The pipes have been revised in the model.**

4. The Inspection & Long-Term Maintenance Plan should be updated to remove permeable pavers since these are not shown on the plans. **Done.**

The model was re-run, the results are consistent with the previous submission.

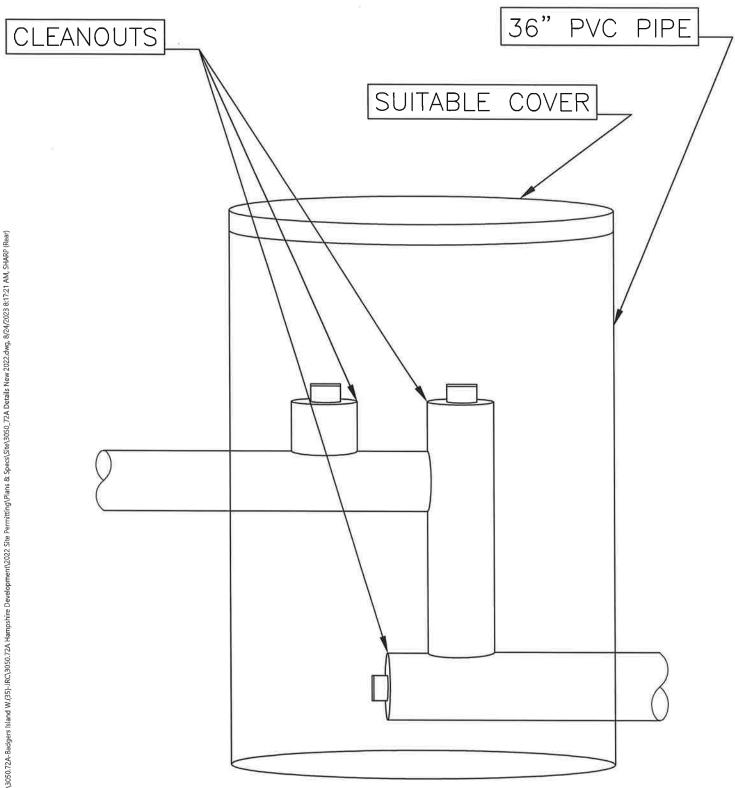
We look forward to our in-person presentation at tonight's Planning Board meeting. Thank you for your time and attention to this proposal.

Please contact me if you have any questions or concerns regarding this application.

Sincerely,

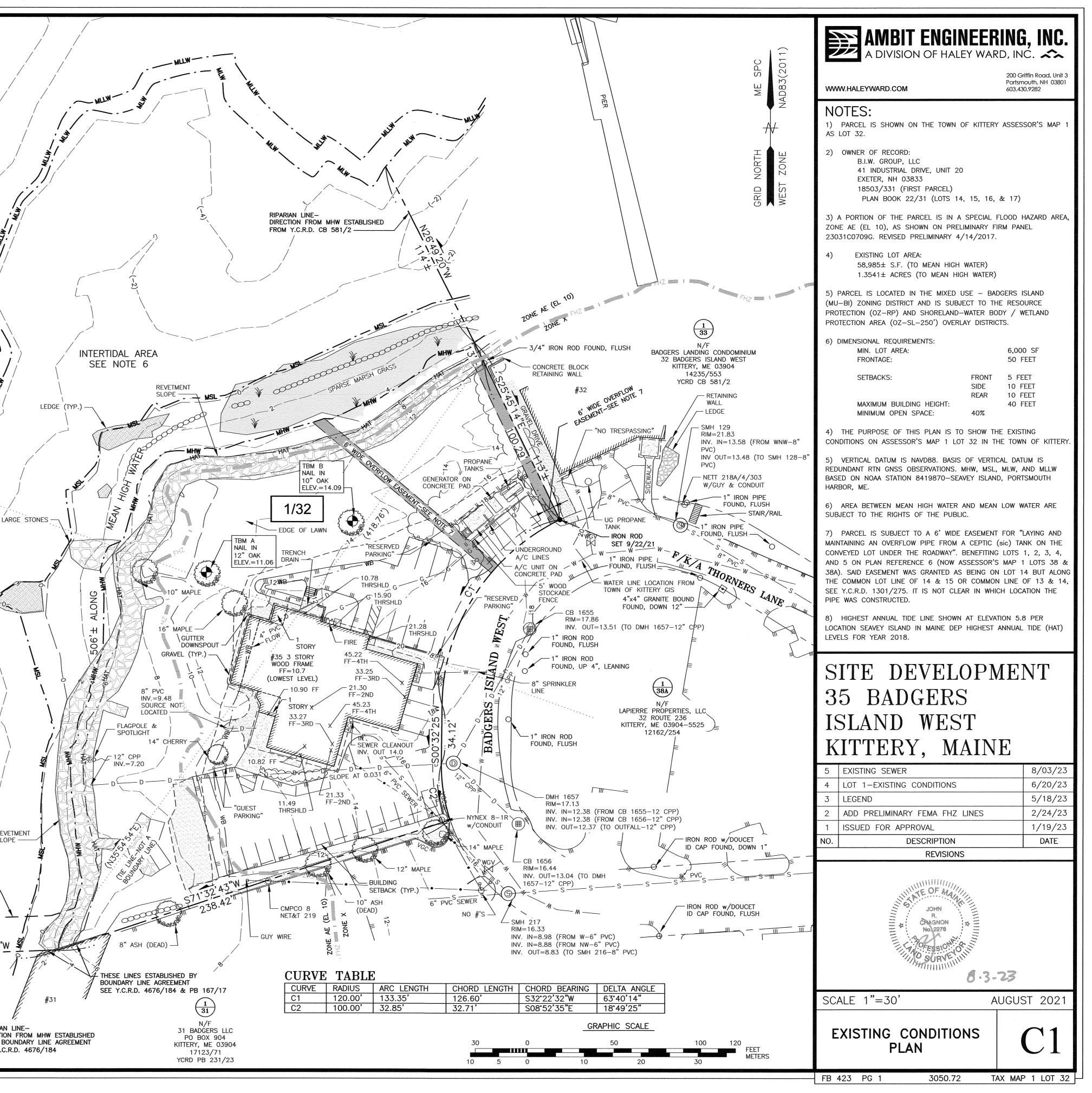
John R. Chagnon, PE Ambit Engineering – Haley Ward CC: Project Team

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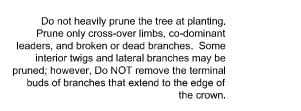
SCHEMATIC MANHOLE

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Acres				BADGERS ISLAND WEST, KITTERY, YORK COUNTY, MAINE CLIENT ISLAND PROPERTIES, LLC PREPARED BY EASTERLY
			103	SURVEY, INC. DATED SEPTEMBER 17, 2002, FINAL REVISION / DATE SEPTEMBER 30, 2002. Y.C.R.D. PLAN BOOK 581,
		Freemans Pt O	Cem.	PAGE 1.
	Water Sho	Maine/NH		2) LAND TITLE SURVEY WEATHERVANE LOBSTER –
		Nobles Badge	ers I	SEAFOODS, THORNERS LANE, BADGERS ISLAND, KITTERY MAINE. PREPARED BY CIVIL CONSULTANTS. DATED AUGUST
	INTERCHANGE 7			21, 1996, FINAL REVISION SEPTEMBER 20, 1996. Y.C.R.D.
	EWOOD	AVE STATE	morial	PLAN BOOK 231/23.
	St Catherine			 3) LOCATION OF A PORTION OF THE TOWN ROAD KNOWN AS BADGERS ISLAND WEST ON BADGERS ISLAND, KITTERY
		SITE		MAINE, FOR THE TOWN OF KITTERY, MAINE. PREPARED BY
				DOUCET SURVEY, INC. DATED AUGUST 26, 1994, FINAL REVISION DATE SEPTEMBER 15, 1995. Y.C.R.D. PLAN BOOK
LOCATION		SCALE 1"=2,	000'	
				4) BOUNDARY PLAN OF LAND, CHARLES & MARYANN D. / PATTEN, KITTERY, MAINE. PREPARED BY THOMAS F. MORAN, / INC. DATED MAY 17, 1982. Y.C.R.D. PLAN BOOK 118/37.
		END:		
N/F RP		NOW OR FORMERLY RECORD OF PROBATE		5) GAGNER / SEWARD PROPERTY LINE EVALUATION SURVEYED SITE PLAN, KITTERY, MAINE. PREPARED BY
YCRI		YORK COUNTY REGISTRY OF DEEDS	\setminus	KIMBALL CHASE. DATED SEPTEMBER 16, 1987. Y.C.R.D. PLAN BOOK 167/17.
$\begin{pmatrix} 11\\ 21 \end{pmatrix}$)	MAP 11 / LOT 21		
		BOUNDARY BUILDING SETBACK		6) PLAN OF LOTS, BADGERS ISLAND, KITTERY, MAINE OWNED BY JOSEPH W. THORNER. PREPARED BY JOHN W.
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	MSL MLW	MEAN SEA LEVEL MEAN LOW WATER		
	MLLW	MEAN LOWER LOW WATER		
	HAT	MAINE DEP HIGHEST ANNUAL TIDE LINE		
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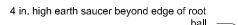
Landscape Notes

- 1. Design is based on drawings by Ambit Engineering dated 2023-06-27. Drawings may require adjustment due to actual field conditions.
- 2. This plan is FOR REVIEW purposes ONLY, NOT for Construction.
- Construction Documents will be provided upon request.
- 3. The contractor shall follow best management practices during construction and shall take all means necessary to stabilize and protect the site from erosion.
- 4. Erosion Control shall be in place prior to construction.
- 5. Erosion Control shall comply with State and Local Erosion & Sedimentation Control Practices
- 6. The Contractor shall verify layout and grades and inform the Landscape Architect or Client's Representative of any discrepancies or changes in layout and/or grade relationships prior to construction.
- 7. It is the contractor's responsibility to verify drawings provided are to the correct scale prior to any bid, estimate or installation. A graphic scale bar has been provided on each sheet for this purpose. If it is determined that the scale of the drawing is incorrect, the landscape architect will provide a set of drawings at the correct scale, at the request of the contractor.
- 8. Trees to Remain within the construction zone shall be protected from damage for the duration of the project by snow fence or other suitable means of protection to be approved by Landscape Architect or Client's Representative. Snow fence shall be located at the drip line or at the distance in feet from the trunk equal to the diameter of the tree caliper in inches, whichever is greater, and shall be expanded to include any and all surface roots. Do not fill or mulch on the trunk flare. Do not disturb roots. In order to protect the integrity of the roots, branches, trunk and bark of the tree(s) no vehicles or construction equipment shall drive or park in or on the area within the drip line(s) of the tree(s). Do not store any refuse or construction materials or portalets within the tree protection area.
- 9. Location, support, protection, and restoration of all existing utilities and appurtenances shall be the responsibility of the Contractor.
- 10. The Contractor shall verify exact location and elevation of all utilities with the respective utility owners prior to construction. Call DIGSAFE at 811 or 888-DIG-SAFE (1-888-344-7233.)
- 11. The Contractor shall procure any required permits prior to construction.
- 12. Prior to any landscape construction activities Contractor shall test all existing loam and loam from off-site intended to be used for lawns and plant beds using a thorough sampling throughout the supply. Soil testing shall indicate levels of pH, nitrates, macro and micro nutrients, texture, soluble salts, and organic matter. Contractor shall amend all soils to be used for lawns and plant beds per testing results' recommendations and review with Landscape Architect. All loam to be used on site shall be amended as approved by the Landscape Architect prior to placement.
- 13. Contractor shall notify landscape architect or owner's representative immediately if at any point during demolition or construction a site condition is discovered which may negatively impact the completed project. This includes, but is not limited to, unforeseen drainage problems, unknown subsurface conditions, and discrepancies between the plan and the site. If a Contractor is aware of a potential issue and does not bring it to the attention of the Landscape Architect or Owner's Representative immediately, they may be responsible for the labor and materials associated with correcting the problem.
- 14. The Contractor shall furnish and plant all plants shown on the drawings and listed thereon. All plants shall be nursery-grown under climatic conditions similar to those in the locality of the project. Plants shall conform to the botanical names and standards of size, culture, and quality for the highest grades and standards as adopted by the American Association of Nurserymen, Inc. in the American Standard of Nursery Stock, American Standards Institute, Inc. 230 Southern Building, Washington, D.C. 20005.
- 15. A complete list of plants, including a schedule of sizes, quantities, and other requirements is shown on the drawings. In the event that quantity discrepancies or material omissions occur in the plant materials list, the planting plans shall govern.
- 16. All plants shall be legibly tagged with proper botanical name.
- 17. Owner or Owner's Representative will inspect plants upon delivery for conformity to Specification requirements. Such approval shall not affect the right of inspection and rejection during or after the progress of the work. The Owner reserves the right to inspect and/or select all trees at the place of growth and reserves the right to approve a representative sample of each type of shrub, herbaceous perennial, annual, and ground cover at the place of growth. Such sample will serve as a minimum standard for all plants of the same species used in this work.
- 18. No substitutions of plants may be made without prior approval of the Owner or the Owner's Representative for any reason.
- 19. All landscaping shall be provided with the following:
- a. Outside hose attachments spaced a maximum of 150 feet apart,
- b. An underground irrigation system, or
- c. A temporary irrigation system designed for a two-year period of plant establishment.
- 21. If an automatic irrigation system is installed, all irrigation valve boxes shall be located within planting bed areas.
- 22. The contractor is responsible for all plant material from the time their work commences until final acceptance. This includes but is not limited to maintaining all plants in good condition, the security of the plant material once delivered to the site, watering of plants, including seeding and weeding. Plants shall be appropriately watered prior to, during, and after planting. It is the Contractor's responsibility to provide clean water suitable for plant health from off site, should it not be available on site.
- 23. All disturbed areas will be dressed with 6" of loam and planted as noted on the plans or seeded except plant beds. Plant beds shall be prepared to a depth of 12" with 75% loam and 25% compost.
- 24. Trees, ground cover, and shrub beds shall be mulched to a depth of 2" with one-year-old, well-composted, shredded native bark not longer than 4" in length and ½" in width, free of woodchips and sawdust. Mulch for ferns and herbaceous perennials shall be no longer than 1" in length. Trees in lawn areas shall be mulched in a 5' diameter min. saucer. Color of mulch shall be black.
- 25. Drip strip shall extend to 6" min. beyond roof overhang and shall be edged with 3/16" thick metal edger.
- 26. In no case shall mulch touch the stem of a plant nor shall mulch ever be more than 3" thick total (including previously applied mulch) over the root ball of any plant.
- 27. Secondary lateral branches of deciduous trees overhanging vehicular and pedestrian travel ways shall be pruned up to a height of 8' to allow clear and safe passage of vehicles and pedestrians under tree canopy. Shrubs and ornamental plantings adjacent to vehicular travel way shall not exceed three feet in height where sightlines would be blocked. If pruning is necessary to maintain the required maximum height, plants shall be pruned to a natural form and shall not be sheared.
- 28. Snow shall be stored a minimum of 5' from shrubs and trunks of trees.
- 29. The Landscape Contractor shall guarantee all lawns and plant materials for a period of not fewer than two years. Dead, dying, or diseased planting shall be removed and replaced within the growing seasor
- 30. Landscape Architect is not responsible for the means and methods of the Contractor.



Trees less than 3" in caliper shall be staked with three stakes per tree, spaced evenly around the trunk with 12 gauge wire. Plastic hose sections shall be used at attachment to trees. Each wire shall be flagged with a visual marker 5' long min, wooden stakes shall be used to anchor the wires. Stakes shall be driven at least 12" outside the edge of the planting pit into stable soil. Remove all staking NO LATER than the end of the first growing season after planting.

> Mark the north side of the tree in the nursery. Rotate the tree to face north at the site whenever possible.

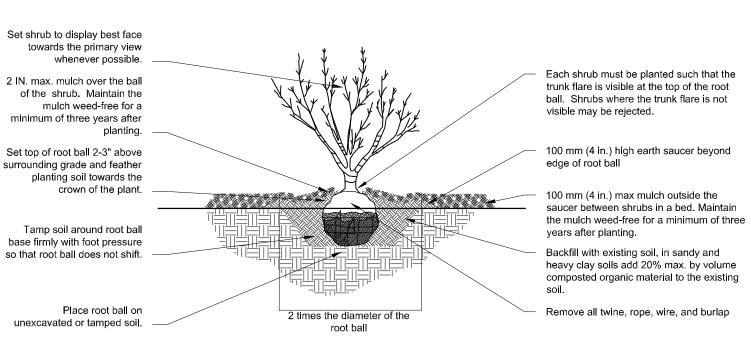


2 IN. max. Mulch. Do NOT place mulch in contact with tree trunk. Maintain the mulch weed-free for a minimum of three years after planting.

Tamp soil around root ball base firmly with foot pressure so that root ball does not shift.







Shrub Planting Detail Scale NTS

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Plant Li	st			
TREES				
Symbol	Botanical Name	Common Name	Quantity	Size
Am	Amelanchier grandiflora 'Autumn Brilliance'	Autumn Brilliance Serviceberry	5	8-10' h
Ar	Acer rubrum 'October Glory'	October Glory Red Maple	5	3" cal.
Ex	Existing tree to remain	Existing tree to remain		0 001.
Ham	Hamamelis x 'Arnold Promis'	Arnold Promise Witch Hazel	2	7-8' ht
Mal	Malus 'Tina'	Tina Crabapple	2	2.5" ca
QR	Quercus rubra	Northern Red Oak	3	3" cal
UA	Ulmus americana 'Princeton'	Princeton Elm	4	3" cal
SHRUBS				
Symbol	Botanical Name	Common Name	Quantity	Size
CL	Clethra alnifolia 'Hummingbird'	Hummingbird Clethra	192	3 gal
Bux	Buxus 'Green Gem'	Green Gem Boxwood	15	5 gal
Hs	Hibiscus syriacus 'Blue Satin'	Blue Satin Rose of Sharon	2	5-6 HT
HY	Hydrangea macrophylla 'All Summer Beauty'	All Summer Beauty Hydrangea (Blue	13	3 gal
HYI	Hydrangea a. 'Incrediball'	Incrediball Hydrangea	4	5 gal
HyP	Hydrangea paniculata 'Limelight'	Limelight Hydrangea	2	10 gal
IG	llex glabra 'Shamrock'	Shamrock Inkberry	36	5 gal
JC	Juniperus communis	Common Juniper	80	3 gal
MP	Myrica pennsylvanica	Bayberry	65	5 gal
RH	Rhododendron chionoides	Chioniodes Rhododendron	4	5 gal
RHUS	Rhus aromatica 'Grow Low'	Grow Low Sumac	227	3 gal
ROS	Rosa 'Blush Knockout'	Blush Knockout Rose	7	3 gal
ROS	Rosa 'Apricot Drift'	Apricot Drift Rose	7	3 gal
SP	Spirea latifolia "Pink Mountain'	Pink Mountain Spirea	135	3 gal
PERENNI	ALS, GROUNDCOVERS, VINES and ANNUALS			
Symbol	Botanical Name	Common Name	Quantity	Size
DAY	Daylily mix	Mixed Daylilies	25	1 gal
HAL	Hakonecloa aurea		86	1 gal
N	Nepeta 'Junior Walker'	Junior Walker Catmint	10	1 gal

the existing soil.

noted above.

Mulch Ring

(8FT.) diam.

preferred

2 times the diameter of the root ball

- Permeable area in which tree is

to be planted shall be no less than

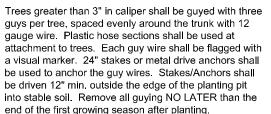
a 3' wide radius from the base of

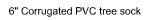
the tree

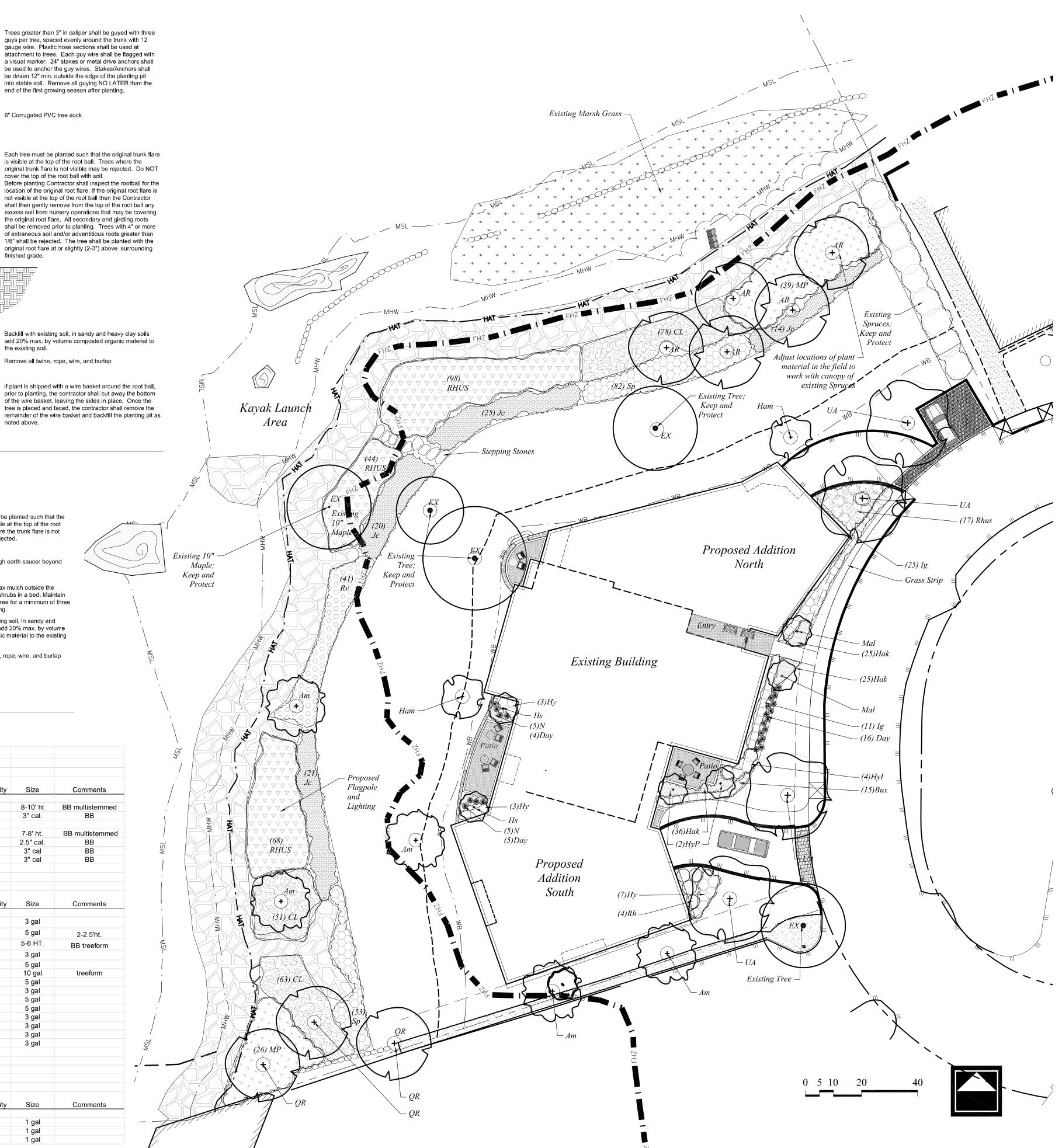
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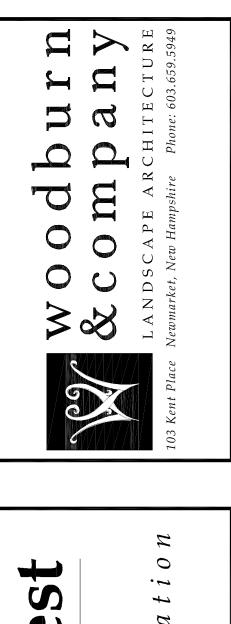
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finished grade.



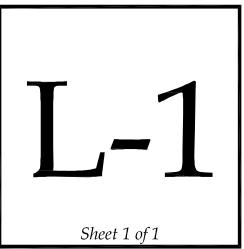






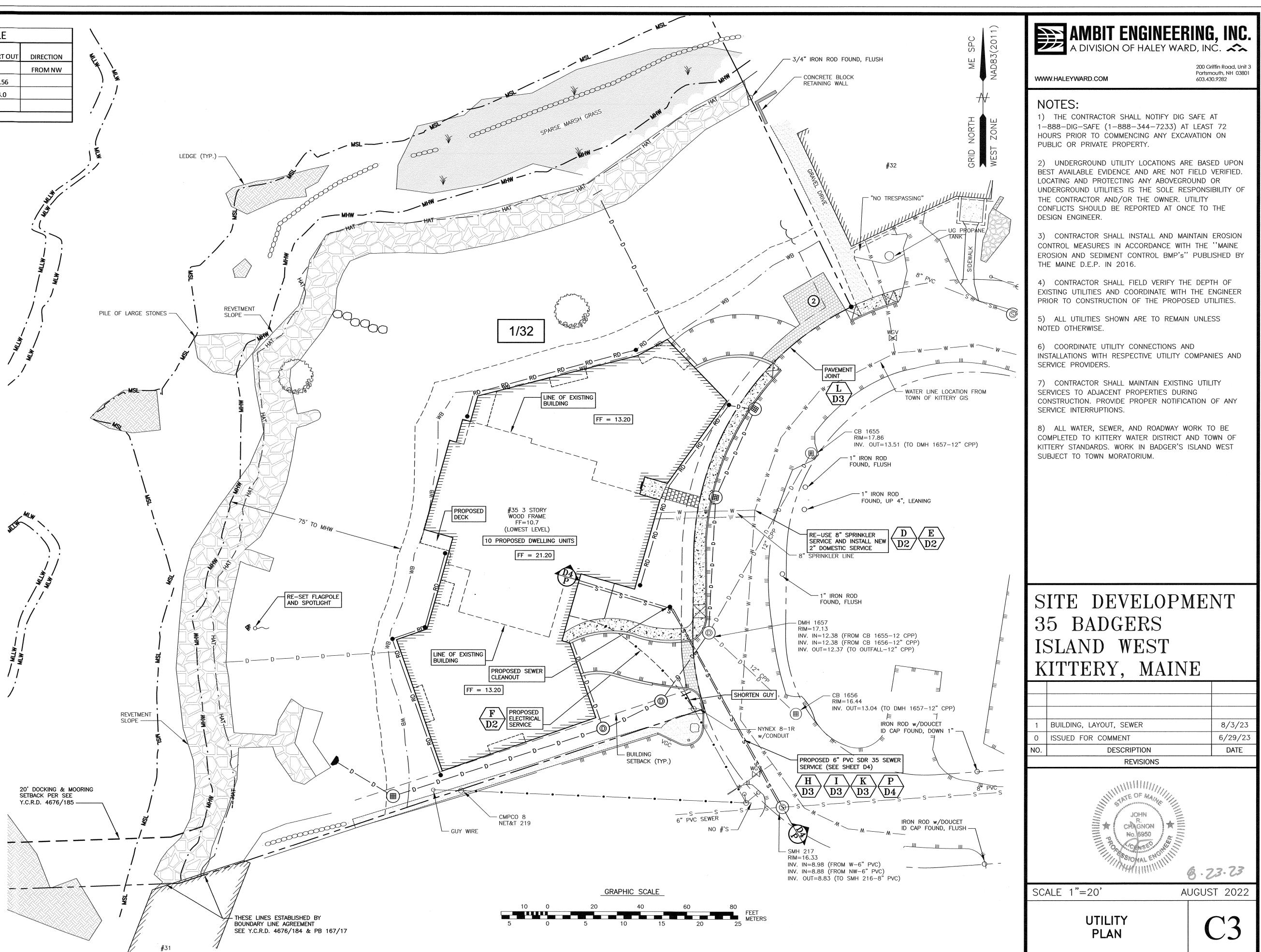


Checked By: Scale:	RW
Caalar	
Scale:	1"=20'-0"
Date: ²⁰²³⁻⁰⁵⁻²⁵ for PB ISSUED SU Revisions:	2023-06-29



© 2023 Woodburn & Company Landscape Architecture, LLC

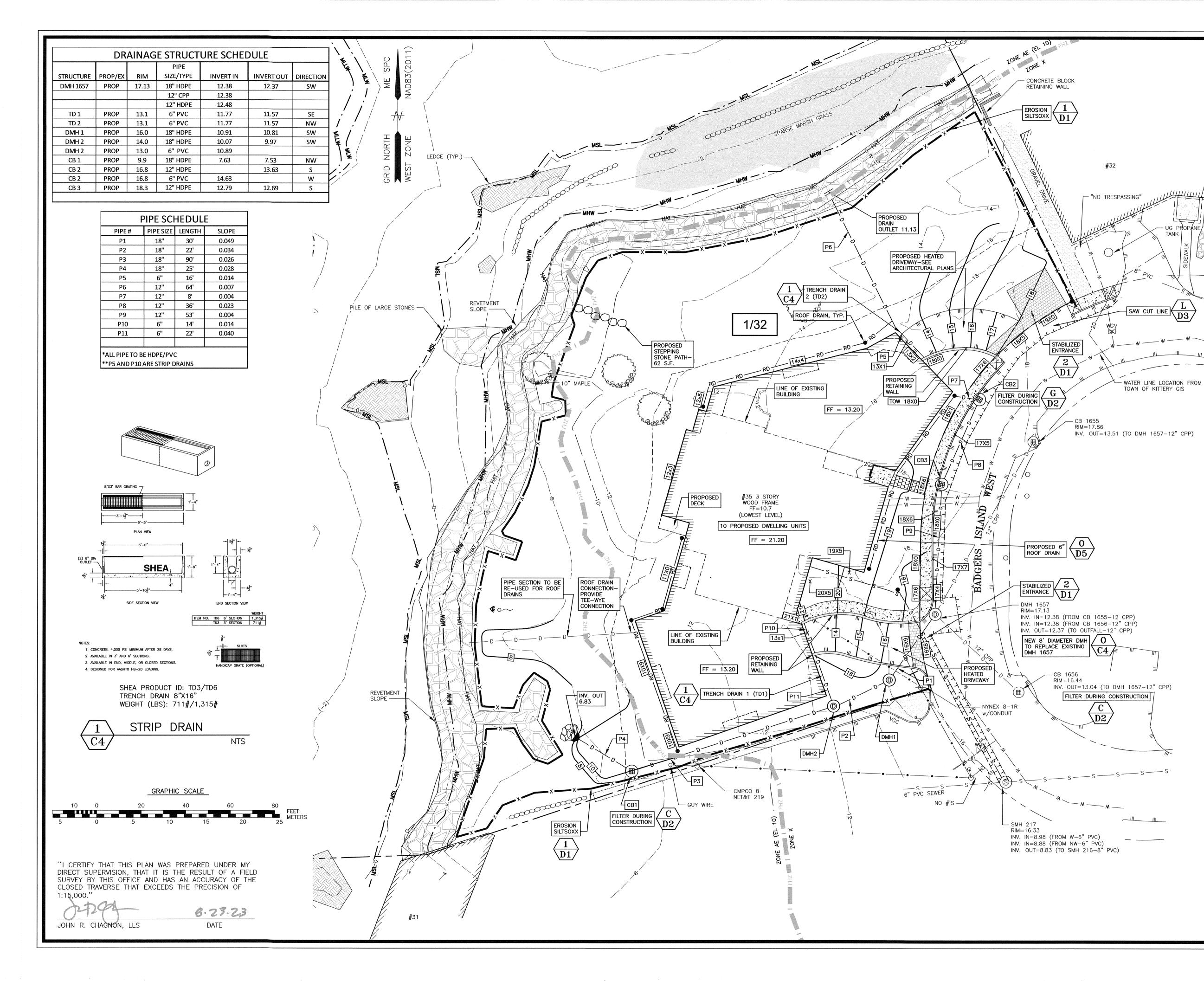
PIPE							
STRUCTURE	PROP/EX	RIM	SIZE/TYPE	INVERT IN	INVERT OUT	DIRECTION	
SMH 217	EX	16.33	6" PVC	8.80		FROM NW	
DROP	PROP	N/A	6" PVC	13.56	10.56		
BLDG.	PROP	N/A	4" PVC		14.0		
	11(01	,,,					



3050.72 TAX

TAX MAP 1 LOT 32

FB 423 PG 1 305





WWW.HALEYWARD.COM

200 Griffin Road, Unit 3 Portsmouth, NH 03801 603.430.9282

NOTES:

UG PROPANE

D3

1) THE CONTRACTOR SHALL NOTIFY DIG SAFE AT 1-888-DIG-SAFE (1-888-344-7233) AT LEAST 72 HOURS PRIOR TO COMMENCING ANY EXCAVATION ON PUBLIC OR PRIVATE PROPERTY.

2) UNDERGROUND UTILITY LOCATIONS ARE BASED UPON BEST AVAILABLE EVIDENCE AND ARE NOT FIELD VERIFIED. LOCATING AND PROTECTING ANY ABOVEGROUND OR UNDERGROUND UTILITIES IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND/OR THE OWNER. UTILITY CONFLICTS SHOULD BE REPORTED AT ONCE TO THE DESIGN ENGINEER.

3) CONTRACTOR SHALL INSTALL AND MAINTAIN EROSION CONTROL MEASURES IN ACCORDANCE WITH THE "MAINE EROSION AND SEDIMENT CONTROL BMP's" PUBLISHED BY THE MAINE D.E.P. IN 2016.

4) TOTAL PROJECT DISTURBED AREA 41,535 S.F.

5) VERTICAL DATUM IS NAVD88. BASIS OF VERTICAL DATUM IS REDUNDANT RTN GNSS OBSERVATIONS.

SITE DEVELOPMENT 35 BADGERS ISLAND WEST KITTERY, MAINE

2	GENERAL NOTES	8/23/23
1	BUILDING, LAYOUT, SAWCUTS	8/3/23
0	ISSUED FOR COMMENT	6/29/23
NO.	DESCRIPTION	DATE
	REVISIONS	
	SURVENING SURVENING	A H
12 I		ST 2022

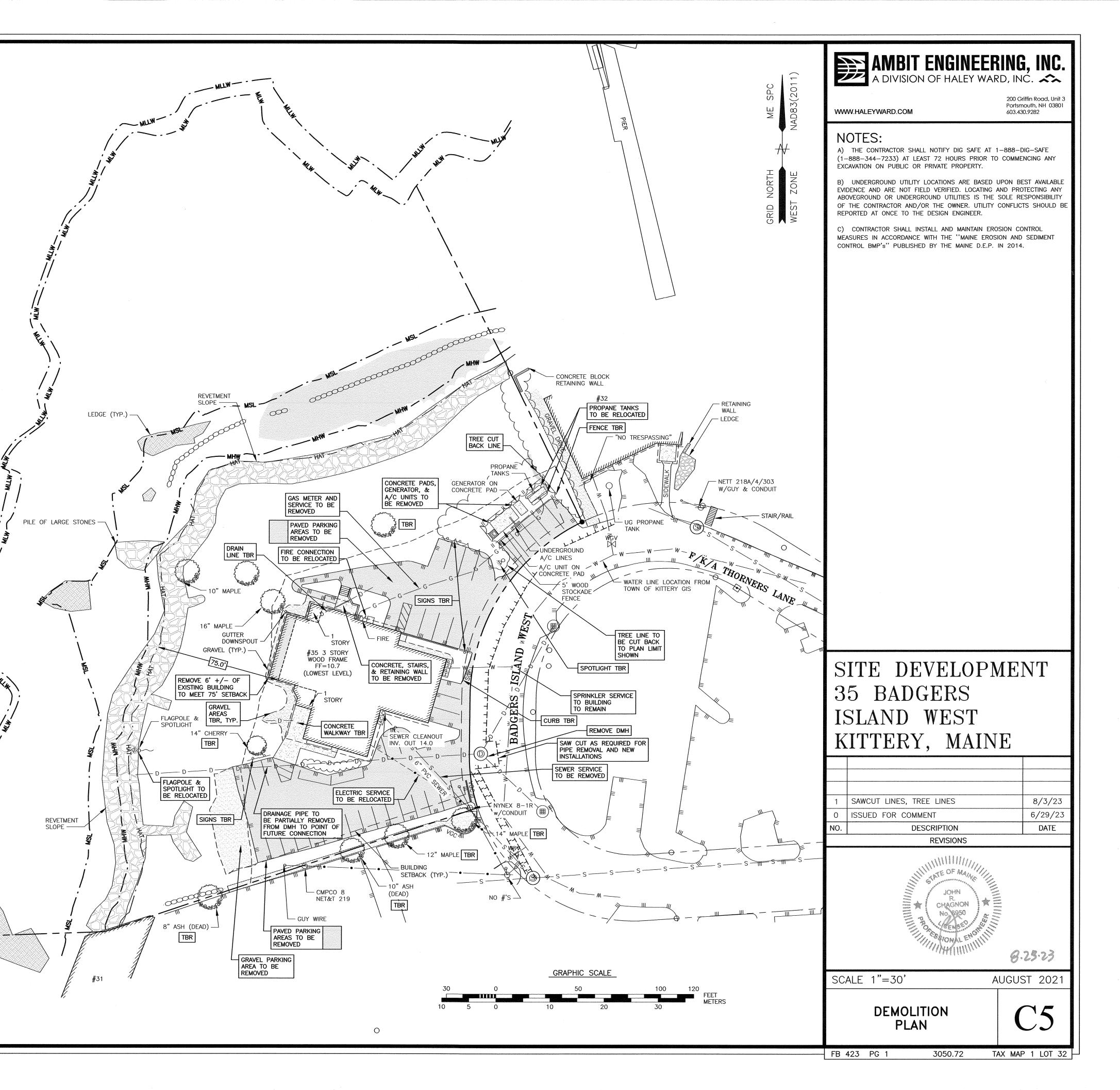


FB 423 PG 1 3050.72 <u>΄</u> ΄Δ

DEMOLITION NOTES:

- A) THE LOCATIONS OF UNDERGROUND UTILITIES ARE APPROXIMATE AND THE LOCATIONS ARE NOT GUARANTEED BY THE OWNER OR THE DESIGNER. IT IS THE CONTRACTORS' RESPONSIBILITY TO LOCATE UTILITIES AND ANTICIPATE CONFLICTS. CONTRACTOR SHALL REPAIR EXISTING UTILITIES DAMAGED BY THEIR WORK AND RELOCATE EXISTING UTILITIES THAT ARE REQUIRED TO BE RELOCATED PRIOR TO COMMENCING ANY WORK IN THE IMPACTED AREA OF THE PROJECT.
- B) ALL MATERIALS SCHEDULED TO BE REMOVED SHALL BECOME THE PROPERTY OF THE CONTRACTORS UNLESS OTHERWISE SPECIFIED. THE CONTRACTOR SHALL DISPOSE OF ALL MATERIALS OFF-SITE IN ACCORDANCE WITH ALL FEDERAL, STATE, AND LOCAL REGULATIONS, ORDINANCES AND CODES. THE CONTRACTOR SHALL COORDINATE REMOVAL, RELOCATION, DISPOSAL, OR SALVAGE OF UTILITIES WITH THE OWNER AND APPROPRIATE UTILITY COMPANY.
- C) ANY EXISTING WORK OR PROPERTY DAMAGED OR DISRUPTED BY CONSTRUCTION/ DEMOLITION ACTIVITIES SHALL BE REPLACED OR REPAIRED TO THE ORIGINAL EXISTING CONDITIONS BY THE CONTRACTOR AT NO ADDITIONAL COST TO THE OWNER.
- D) THE CONTRACTOR SHALL VERIFY LOCATION OF ALL EXISTING UTILITIES AND CALL DIG SAFE AT LEAST 72 HOURS PRIOR TO THE COMMENCEMENT OF ANY DEMOLITION/CONSTRUCTION ACTIVITIES.
- E) SAWCUT AND REMOVE PAVEMENT ONE FOOT OFF PROPOSED EDGE OF PAVEMENT TRENCH IN AREAS WHERE PAVEMENT IS TO BE REMOVED.
- F) IT IS THE CONTRACTOR'S RESPONSIBILITY TO FAMILIARIZE THEMSELVES WITH THE CONDITIONS OF ALL THE PERMIT APPROVALS.
- G) THE CONTRACTOR SHALL OBTAIN AND PAY FOR ADDITIONAL CONSTRUCTION PERMITS, NOTICES AND FEES NECESSARY TO COMPLETE THE WORK AND ARRANGE FOR AND PAY FOR ANY INSPECTIONS AND APPROVALS FROM THE AUTHORITIES HAVING JURISDICTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ADDITIONAL AND OFF-SITE DISPOSAL OF MATERIALS REQUIRED TO COMPLETE THE WORK.
- H) THE CONTRACTOR SHALL REMOVE AND DISPOSE OF ALL EXISTING STRUCTURES, CONCRETE, UTILITIES, VEGETATION, PAVEMENT, AND CONTAMINATED SOIL WITHIN THE WORK LIMITS SHOWN UNLESS SPECIFICALLY IDENTIFIED TO REMAIN. ANY EXISTING DOMESTIC / IRRIGATION SERVICE WELLS IN THE PROJECT AREA IDENTIFIED DURING THE CONSTRUCTION AND NOT CALLED OUT ON THE PLANS SHALL BE BROUGHT TO THE ATTENTION OF THE OWNER AND ENGINEER FOR PROPER CAPPING / RE-USE.
- I) ALL WORK WITHIN THE TOWN OF KITTERY RIGHT OF WAY SHALL BE COORDINATED WITH THE TOWN OF KITTERY DEPARTMENT OF PUBLIC WORKS (DPW).
- J) REMOVE TREES AND BRUSH AS REQUIRED FOR COMPLETION OF WORK. CONTRACTOR SHALL GRUB AND REMOVE ALL STUMPS WITHIN LIMITS OF WORK AND DISPOSE OF OFF-SITE IN ACCORDANCE WITH FEDERAL, STATE, AND LOCAL LAWS AND REGULATIONS.
- K) CONTRACTOR SHALL PROTECT ALL PROPERTY MONUMENTATION THROUGHOUT DEMOLITION AND CONSTRUCTION OPERATIONS. SHOULD ANY MONUMENTATION BE DISTURBED, THE CONTRACTOR SHALL EMPLOY A LAND SURVEYOR TO REPLACE THEM.
- L) PROVIDE INLET PROTECTION BARRIERS AT ALL CATCH BASINS WITHIN CONSTRUCTION LIMITS AND MAINTAIN FOR THE DURATION OF THE PROJECT. INLET PROTECTION BARRIERS SHALL BE HIGH FLOW SILT SACK BY ACF ENVIRONMENTAL OR APPROVED EQUAL. INSPECT BARRIERS WEEKLY AND AFTER EACH RAIN OF 0.25 INCHES OR GREATER. CONTRACTOR SHALL COMPLETE A MAINTENANCE INSPECTION REPORT AFTER EACH INSPECTION. SEDIMENT DEPOSITS SHALL BE REMOVED AFTER EACH STORM EVENT OR MORE OFTEN IF WARRANTED OR FABRIC BECOMES CLOGGED. EROSION CONTROL MEASURES SHALL BE INSTALLED PRIOR TO THE START OF ANY CLEARING OR DEMOLITION ACTIVITIES.
- M) THE CONTRACTOR SHALL PAY ALL COSTS NECESSARY FOR TEMPORARY PARTITIONING, BARRICADING, FENCING, SECURITY AND SAFELY DEVICES REQUIRED FOR THE MAINTENANCE OF A CLEAN AND SAFE CONSTRUCTION SITE.
- N) ANY CONTAMINATED MATERIAL REMOVED DURING THE COURSE OF THE WORK WILL REQUIRE HANDLING IN ACCORDANCE WITH MEDEP REGULATIONS. CONTRACTOR SHALL HAVE A HEALTH AND SAFETY PLAN IN PLACE, AND COMPLY WITH ALL APPLICABLE PERMITS, APPROVALS, AUTHORIZATIONS, AND REGULATIONS

PISCATAQUA RIVER (TIDAL)





Existing Subcatchments Plan

B.I.W. GROUP, LLC 35 BADGERS ISLAND WEST KITTERY, MAINE JOB NUMBER: 5010135.3050.72A SCALE: 1" = 100' SUBMITTED: 08-24-2023

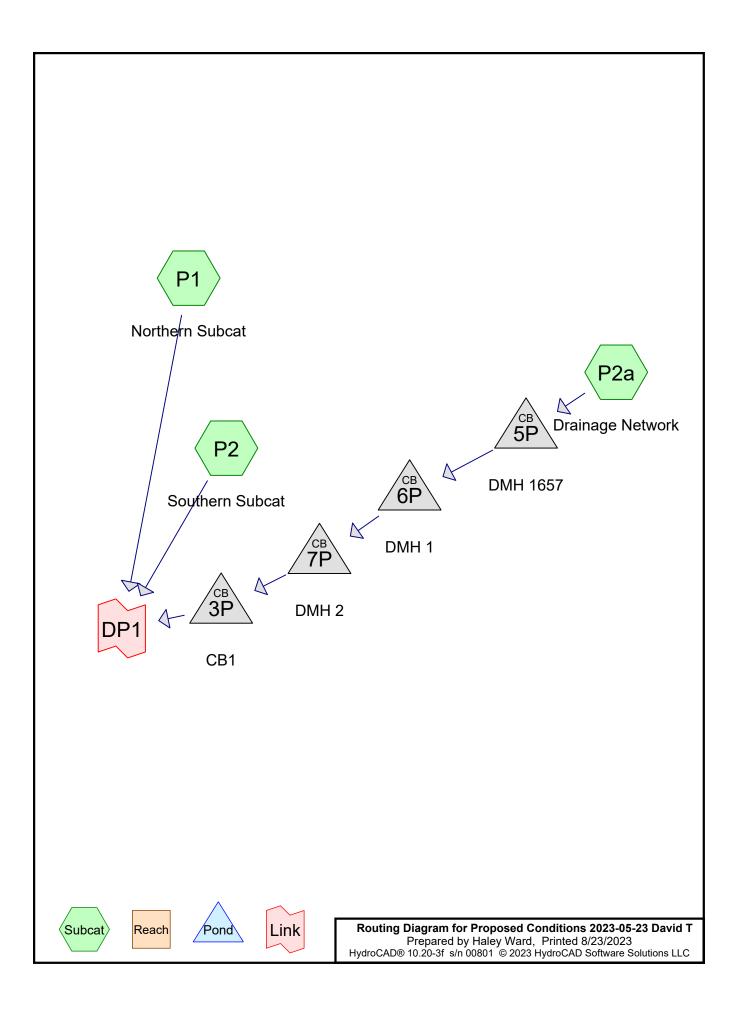




Proposed Subcatchments Plan

B.I.W. GROUP, LLC 35 BADGERS ISLAND WEST KITTERY, MAINE JOB NUMBER: 5010135.3050.72A SCALE: 1" = 100' SUBMITTED: 08-24-2023





Project Notes

Defined 5 rainfall events from output (39) IDF

Event#	Event	Storm Type	Curve	Mode	Duration	B/B	Depth	AMC	
	Name				(hours)		(inches)		
1	2-yr	Type II 24-hr		Default	24.00	1	3.20	2	
2	10-yr	Type II 24-hr		Default	24.00	1	4.86	2	
3	25-yr	Type II 24-hr		Default	24.00	1	6.16	2	

Rainfall Events Listing (selected events)

Proposed Conditions 2023-05-23 David T Prepared by Haley Ward HydroCAD® 10.20-3f s/n 00801 © 2023 HydroCAD Software Solutions LLC

Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
0.984	80	>75% Grass cover, Good, HSG D (P1, P2, P2a)
0.109	96	Gravel surface, HSG D (P1, P2)
0.925	98	Paved parking, HSG D (P1, P2a)
0.335	98	Roofs, HSG D (P1, P2, P2a)
0.097	98	Water Surface, 0% imp, HSG D (P1)
0.924	98	Water Surface, HSG D (P1, P2)
0.004	77	Woods, Good, HSG D (P1)
3.378	93	TOTAL AREA

Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
0.000	HSG B	
0.000	HSG C	
3.378	HSG D	P1, P2, P2a
0.000	Other	
3.378		TOTAL AREA

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HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchmen
0.000	0.000	0.000	0.984	0.000	0.984	>75% Grass cover, Good	P1, P2, P2a
0.000	0.000	0.000	0.109	0.000	0.109	Gravel surface	P1, P2
0.000	0.000	0.000	0.925	0.000	0.925	Paved parking	P1, P2a
0.000	0.000	0.000	0.335	0.000	0.335	Roofs	P1, P2, P2a
0.000	0.000	0.000	0.924	0.000	0.924	Water Surface	P1, P2
0.000	0.000	0.000	0.097	0.000	0.097	Water Surface, 0% imp	P1
0.000 0.000	0.000 0.000	0.000 0.000	0.004 3.378	0.000 0.000	0.004 3.378	Woods, Good TOTAL AREA	P1

Ground Covers (all nodes)

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Pipe Listing	g (all nodes)

Line#	Node	In-Invert	Out-Invert	Length	Slope	n	Width	Diam/Height	Inside-Fill	Node
	Number	(feet)	(feet)	(feet)	(ft/ft)		(inches)	(inches)	(inches)	Name
1	3P	7.53	6.83	25.1	0.0279	0.013	0.0	18.0	0.0	
2	5P	12.37	10.91	30.0	0.0487	0.013	0.0	18.0	0.0	
3	6P	10.81	10.07	22.0	0.0336	0.013	0.0	18.0	0.0	
4	7P	9.97	7.63	90.0	0.0260	0.013	0.0	18.0	0.0	

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment P1: Northern Subcat	Runoff Area=65,505 sf 51.78% Impervious Runoff Depth=2.26"
Flow Length=585'	Slope=0.0374 '/' Tc=7.2 min CN=91 Runoff=5.48 cfs 0.283 af
Subcatchment P2: Southern Subcat	Runoff Area=30,379 sf 54.47% Impervious Runoff Depth=2.17" Tc=5.0 min CN=90 Runoff=2.63 cfs 0.126 af
Subcatchment P2a: Drainage Network	Runoff Area=51,243 sf 87.19% Impervious Runoff Depth=2.75"
Flow Length=411'	Slope=0.0155 '/' Tc=6.7 min CN=96 Runoff=4.89 cfs 0.270 af
Pond 3P: CB1	Peak Elev=8.62' Inflow=4.89 cfs 0.270 af
18.0" Round	Culvert n=0.013 L=25.1' S=0.0279 '/' Outflow=4.89 cfs 0.270 af
Pond 5P: DMH 1657	Peak Elev=13.46' Inflow=4.89 cfs 0.270 af
18.0" Round	Culvert n=0.013 L=30.0' S=0.0487 '/' Outflow=4.89 cfs 0.270 af
Pond 6P: DMH 1	Peak Elev=11.90' Inflow=4.89 cfs 0.270 af
18.0" Round	Culvert n=0.013 L=22.0' S=0.0336 '/' Outflow=4.89 cfs 0.270 af
Pond 7P: DMH 2	Peak Elev=11.06' Inflow=4.89 cfs 0.270 af
18.0" Round	Culvert n=0.013 L=90.0' S=0.0260 '/' Outflow=4.89 cfs 0.270 af
Link DP1:	below 1,000.00 cfs Inflow=12.81 cfs 0.679 af Primary=12.81 cfs 0.679 af Secondary=0.00 cfs 0.000 af
Total Runoff Area = 3.378 a	c Runoff Volume = 0.679 af Average Runoff Depth = 2.41"

otal Runoff Area = 3.378 ac Runoff Volume = 0.679 af Average Runoff Depth = 2.41" 35.33% Pervious = 1.193 ac 64.67% Impervious = 2.184 ac

Summary for Subcatchment P1: Northern Subcat

Runoff = 5.48 cfs @ 11.98 hrs, Volume= 0.283 af, Depth= 2.26" Routed to Link DP1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type II 24-hr 2-yr Rainfall=3.20"

Area (sf)	CN D	escription			
3,5	70	96 G	ravel surfa	ace, HSG D)	
27,6	40	98 W	/ater Surfa	ice, HSG D		
4,2	06	98 W	/ater Surfa	ice, 0% imp	o, HSG D	
1	73	77 W	/oods, Goo	od, HSG D		
2,7	71	98 R	oofs, HSG	D		
23,6	39	80 >	75% Grass	s cover, Go	ood, HSG D	
3,5	06	98 P	aved parki	ng, HSG D		
65,5	05	91 W	/eighted A	verage		
31,5	88	48	8.22% Per	vious Area		
33,9	17	5	1.78% Imp	ervious Are	ea	
Tc Ler	ngth	Slope	Velocity	Capacity	Description	
(min) (f	eet)	(ft/ft)	(ft/sec)	(cfs)		
7.2	585 (0.0374	1.35		Lag/CN Method,	

Summary for Subcatchment P2: Southern Subcat

- [49] Hint: Tc<2dt may require smaller dt
- Runoff = 2.63 cfs @ 11.95 hrs, Volume= 0.126 af, Depth= 2.17" Routed to Link DP1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type II 24-hr 2-yr Rainfall=3.20"

A	rea (sf)	CN	Description			
	1,183	96	Gravel surfa	ace, HSG D)	
	12,607	98	Water Surfa	ace, HSG D)	
	3,939	98	Roofs, HSG	6 D		
	12,650	80	>75% Gras	s cover, Go	ood, HSG D	
	30,379	90	Weighted A	verage		
	13,833		45.53% Pervious Area			
	16,546		54.47% Impervious Area			
Та	l a ra artha	Clana	Valasity	Consolt	Description	
	Length	Slope	,	Capacity	Description	
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)		
5.0					Direct Entry,	

Summary for Subcatchment P2a: Drainage Network

Runoff	=	4.89 cfs @	11.97 hrs,	Volume=	0.270 af,	Depth= 2	.75"
Routed	l to Pon	d 5P : DMH 16	657				

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type II 24-hr 2-yr Rainfall=3.20"

A	rea (sf)	CN [Descriptior	ı	
	7,903	98 F	Roofs, HS	GD	
	6,564				bod, HSG D
	36,776			king, HSG D	
	51,243	96 V	Veighted A	Average	
	6,564			rvious Area	l
	44,679	8	37.19% lm	pervious Ar	ea
				-	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.7	411	0.0155	1.03		Lag/CN Method,
				Summary	/ for Pond 3P: CB1
[79] War	rning: Sub	merged	Pond 7P F	Primary dev	ice # 1 OUTLET by 0.97'
Inflow A	rea =	1.176	ac, 87.19	% Impervio	us, Inflow Depth = 2.75" for 2-yr event
Inflow	=	4.89 ct	fs @ 11.9	97 hrs, Volu	ume= 0.270 af
Outflow	=	4.89 ct	ís @ 11.9	97 hrs, Volu	ume= 0.270 af, Atten= 0%, Lag= 0.0 min
Primary			fs @ 11.9	97 hrs, Volu	ume= 0.270 af
Route	ed to Link	DP1 :			
Devilian				0 00 7	
				pan= 0.00-7	/2.00 hrs, dt= 0.05 hrs
	ev= 8.62' ev= 9.90'	@ 11.97	nis		
	ev- 9.90				
Device	Routing		Invert (Dutlet Devic	es
#1	Primary			8.0" Roun	
					PP, square edge headwall, Ke= 0.500
					Invert= 7.53' / 6.83' S= 0.0279 '/' Cc= 0.900
			r	n= 0.013 Co	orrugated PE, smooth interior, Flow Area= 1.77 sf
Dulina	0.451		70 -4- 0	11.07 hrs. 1	IV/-9 COL (Error Discharge)
				11.97 nrs F s @ 3.52 fps	HW=8.60' (Free Discharge)
—ı-cu			// 0 Cl	s @ 3.52 ips	5)

Summary for Pond 5P: DMH 1657

 Inflow Area =
 1.176 ac, 87.19% Impervious, Inflow Depth =
 2.75" for 2-yr event

 Inflow =
 4.89 cfs @
 11.97 hrs, Volume=
 0.270 af

 Outflow =
 4.89 cfs @
 11.97 hrs, Volume=
 0.270 af, Atten= 0%, Lag= 0.0 min

 Primary =
 4.89 cfs @
 11.97 hrs, Volume=
 0.270 af

 Routed to Pond 6P : DMH 1
 0.270 af

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Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 13.46' @ 11.97 hrs Flood Elev= 17.13'

Device	Routing	Invert	Outlet Devices
	Primary		18.0" Round Culvert L= 30.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 12.37' / 10.91' S= 0.0487 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=4.76 cfs @ 11.97 hrs HW=13.44' (Free Discharge) -1=Culvert (Inlet Controls 4.76 cfs @ 3.52 fps)

Summary for Pond 6P: DMH 1

[79] Warning: Submerged Pond 5P Primary device # 1 OUTLET by 0.97'

Inflow Area =	1.176 ac, 8	7.19% Impervious, Inflo	ow Depth = 2.75" for 2-yr event	
Inflow =	4.89 cfs @	11.97 hrs, Volume=	0.270 af	
Outflow =	4.89 cfs @	11.97 hrs, Volume=	0.270 af, Atten= 0%, Lag= ().0 min
Primary =	4.89 cfs @	11.97 hrs, Volume=	0.270 af	
Routed to Pond	17P:DMH2			

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 11.90' @ 11.97 hrs Flood Elev= 14.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	10.81'	18.0" Round Culvert L= 22.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 10.81' / 10.07' S= 0.0336 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=4.76 cfs @ 11.97 hrs HW=11.88' (Free Discharge) **1=Culvert** (Inlet Controls 4.76 cfs @ 3.52 fps)

Summary for Pond 7P: DMH 2

[79] Warning: Submerged Pond 6P Primary device # 1 INLET by 0.23'

Inflow Area	a =	1.176 ac, 8	87.19% Imp	ervious, In	flow Depth =	2.75"	for 2-y	r event
Inflow	=	4.89 cfs @	11.97 hrs,	Volume=	0.270) af	-	
Outflow	=	4.89 cfs @	11.97 hrs,	Volume=	0.270) af, At	ten= 0%,	Lag= 0.0 min
Primary	=	4.89 cfs @	11.97 hrs,	Volume=	0.270) af		-
Routed	to Pond	3P:CB1						

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 11.06' @ 11.97 hrs Flood Elev= 13.00' Proposed Conditions 2023-05-23 David T

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Device	Routing	Invert	Outlet Devices
#1	Primary	9.97'	18.0" Round Culvert L= 90.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 9.97' / 7.63' S= 0.0260 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=4.76 cfs @ 11.97 hrs HW=11.04' (Free Discharge) **1=Culvert** (Inlet Controls 4.76 cfs @ 3.52 fps)

Summary for Link DP1:

Inflow Area =	3.378 ac, 64.67% Impervious, Inflow D	Depth = 2.41" for 2-yr event
Inflow =	12.81 cfs @ 11.97 hrs, Volume=	0.679 af
Primary =	12.81 cfs @_ 11.97 hrs, Volume=	0.679 af, Atten= 0%, Lag= 0.0 min
Secondary =	0.00 cfs $\overline{@}$ 0.00 hrs, Volume=	0.000 af

Primary outflow = Inflow below 1,000.00 cfs, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Subcatchment P1: Northern Subcat	Runoff Area=65,505 sf 51.78% Impervious Runoff Depth=3.85"
Flow Length=	-585' Slope=0.0374 '/' Tc=7.2 min CN=91 Runoff=9.03 cfs 0.482 af
Subcatchment P2: Southern Subcat	Runoff Area=30,379 sf 54.47% Impervious Runoff Depth=3.74" Tc=5.0 min CN=90 Runoff=4.40 cfs 0.217 af
Subcatchment P2a: Drainage Network Flow Length=	
Pond 3P: CB1	Peak Elev=9.07' Inflow=7.59 cfs 0.431 af
18.0" R	ound Culvert n=0.013 L=25.1' S=0.0279 '/' Outflow=7.59 cfs 0.431 af
Pond 5P: DMH 1657	Peak Elev=13.91' Inflow=7.59 cfs 0.431 af
18.0" R	ound Culvert n=0.013 L=30.0' S=0.0487 '/' Outflow=7.59 cfs 0.431 af
Pond 6P: DMH 1	Peak Elev=12.35' Inflow=7.59 cfs 0.431 af
18.0" R	ound Culvert n=0.013 L=22.0' S=0.0336 '/' Outflow=7.59 cfs 0.431 af
Pond 7P: DMH 2	Peak Elev=11.51' Inflow=7.59 cfs 0.431 af
18.0" R	ound Culvert n=0.013 L=90.0' S=0.0260 '/' Outflow=7.59 cfs 0.431 af
Link DP1:	below 1,000.00 cfs Inflow=20.73 cfs 1.130 af Primary=20.73 cfs 1.130 af Secondary=0.00 cfs 0.000 af
Total Runoff Area = 3.3	78 ac Runoff Volume = 1.130 af Average Runoff Depth = 4.02"

otal Runoff Area = 3.378 ac Runoff Volume = 1.130 af Average Runoff Depth = 4.02" 35.33% Pervious = 1.193 ac 64.67% Impervious = 2.184 ac

Summary for Subcatchment P1: Northern Subcat

Runoff = 9.03 cfs @ 11.98 hrs, Volume= 0.482 af, Depth= 3.85" Routed to Link DP1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type II 24-hr 10-yr Rainfall=4.86"

A	rea (sf)	CN	Description				
	3,570	96	Gravel surfa	ace, HSG D)		
	27,640	98	Water Surfa	ace, HSG D)		
	4,206	98	Water Surfa	ace, 0% imp	o, HSG D		
	173	77	Woods, Go	od, HSG D			
	2,771	98	Roofs, HSG	6 D			
	23,639	80	>75% Gras	s cover, Go	ood, HSG D		
	3,506	98	Paved parking, HSG D				
	65,505	91	Weighted Average				
	31,588		48.22% Pervious Area				
	33,917		51.78% Imp	pervious Ar	ea		
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
7.2	585	0.0374	1.35		Lag/CN Method,		

Summary for Subcatchment P2: Southern Subcat

- [49] Hint: Tc<2dt may require smaller dt
- Runoff = 4.40 cfs @ 11.95 hrs, Volume= 0.217 af, Depth= 3.74" Routed to Link DP1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type II 24-hr 10-yr Rainfall=4.86"

Α	rea (sf)	CN	Description					
	1,183	96	Gravel surfa	ace, HSG D				
	12,607	98	Water Surfa	ace, HSG D				
	3,939	98	Roofs, HSG	6 D				
	12,650	80	>75% Gras	s cover, Go	od, HSG D			
	30,379	90	Weighted Average					
	13,833		45.53% Pervious Area					
	16,546		54.47% Impervious Area					
_		-						
	Length	Slope	,	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.0					Direct Entry,			

Summary for Subcatchment P2a: Drainage Network

Runoff	=	7.59 cfs @	11.97 hrs,	Volume=	0.431 af,	Depth= 4.39"
Routed	to Pond	d 5P : DMH 16	657			

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type II 24-hr 10-yr Rainfall=4.86"

Area (sf) CN Description						
7,903 98 Roofs, HSG D						
6,564 80 >75% Grass cover, Good, HSG D						
36,776 98 Paved parking, HSG D						
51,243 96 Weighted Average 6,564 12.81% Pervious Area						
44,679 87.19% Impervious Area						
Tc Length Slope Velocity Capacity Description						
(min) (feet) (ft/ft) (ft/sec) (cfs)						
6.7 411 0.0155 1.03 Lag/CN Method,						
Summary for Pond 3P: CB1						
[79] Warning: Submerged Pond 7P Primary device # 1 OUTLET by 1.41'						
Inflow Area = 1.176 ac, 87.19% Impervious, Inflow Depth = 4.39" for 10-yr event Inflow = 7.59 cfs @ 11.97 hrs, Volume= 0.431 af						
Outflow = 7.59 cfs @ 11.97 hrs , Volume= 0.431 af , Atten= 0%, Lag= 0.0 min						
Primary = 7.59 cfs @ 11.97 hrs, Volume= 0.431 af Routed to Link DP1 :						
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs						
Peak Elev= 9.07' @ 11.97 hrs						
Flood Elev= 9.90'						
Device Routing Invert Outlet Devices						
#1 Primary 7.53' 18.0" Round Culvert						
L= 25.1' CPP, square edge headwall, Ke= 0.500						
Inlet / Outlet Invert= 7.53' / 6.83' S= 0.0279 '/' Cc= 0.900						
n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf						
Primary OutFlow Max=7.38 cfs @ 11.97 hrs HW=9.03' (Free Discharge)						
Summary for Pond 5P: DMH 1657						

Summary for Pond 5P: DMH 1657

 Inflow Area =
 1.176 ac, 87.19% Impervious, Inflow Depth = 4.39" for 10-yr event

 Inflow =
 7.59 cfs @
 11.97 hrs, Volume=
 0.431 af

 Outflow =
 7.59 cfs @
 11.97 hrs, Volume=
 0.431 af, Atten= 0%, Lag= 0.0 min

 Primary =
 7.59 cfs @
 11.97 hrs, Volume=
 0.431 af

 Routed to Pond 6P : DMH 1
 0.431 af

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Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 13.91' @ 11.97 hrs Flood Elev= 17.13'

Device	Routing	Invert	Outlet Devices
#1	Primary	12.37'	18.0" Round Culvert L= 30.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 12.37' / 10.91' S= 0.0487 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=7.38 cfs @ 11.97 hrs HW=13.87' (Free Discharge) -1=Culvert (Inlet Controls 7.38 cfs @ 4.17 fps)

Summary for Pond 6P: DMH 1

[79] Warning: Submerged Pond 5P Primary device # 1 OUTLET by 1.41'

Inflow Area =	1.176 ac, 87.19% Impervious, Inflow I	Depth = 4.39" for 10-yr event				
Inflow =	7.59 cfs @ 11.97 hrs, Volume=	0.431 af				
Outflow =	7.59 cfs @_ 11.97 hrs, Volume=	0.431 af, Atten= 0%, Lag= 0.0 min				
Primary =	7.59 cfs @_ 11.97 hrs, Volume=	0.431 af				
Routed to Pond 7P : DMH 2						

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 12.35' @ 11.97 hrs Flood Elev= 14.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	10.81'	18.0" Round Culvert
	-		L= 22.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 10.81' / 10.07' S= 0.0336 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=7.38 cfs @ 11.97 hrs HW=12.31' (Free Discharge) **1=Culvert** (Inlet Controls 7.38 cfs @ 4.18 fps)

Summary for Pond 7P: DMH 2

[79] Warning: Submerged Pond 6P Primary device # 1 INLET by 0.67'

Inflow Area = 1.176 ac, 87.19% Impervious, Inflow Depth = 4.39" for 10-yr event 7.59 cfs @ 11.97 hrs, Volume= Inflow 0.431 af = 7.59 cfs @ 11.97 hrs, Volume= 0.431 af, Atten= 0%, Lag= 0.0 min Outflow = 7.59 cfs @ 11.97 hrs, Volume= 0.431 af Primary = Routed to Pond 3P : CB1

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 11.51' @ 11.97 hrs Flood Elev= 13.00' Proposed Conditions 2023-05-23 David T

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Device	Routing	Invert	Outlet Devices
#1	Primary	9.97'	18.0" Round Culvert L= 90.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 9.97' / 7.63' S= 0.0260 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=7.38 cfs @ 11.97 hrs HW=11.47' (Free Discharge) **1=Culvert** (Inlet Controls 7.38 cfs @ 4.18 fps)

Summary for Link DP1:

Inflow Area =	3.378 ac, 64.67% Impervious, Inflow	Depth = 4.02" for 10-yr event
Inflow =	20.73 cfs @ 11.97 hrs, Volume=	1.130 af
Primary =	20.73 cfs @ 11.97 hrs, Volume=	1.130 af, Atten= 0%, Lag= 0.0 min
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Primary outflow = Inflow below 1,000.00 cfs, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

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Runoff by SCS TR	2.00 hrs, dt=0.05 hrs, 1441 points 20 method, UH=SCS, Weighted-CN ns method - Pond routing by Stor-Ind method
Subcatchment P1: Northern Subcat	Runoff Area=65,505 sf 51.78% Impervious Runoff Depth=5.11"
Flow Length=585'	Slope=0.0374 '/' Tc=7.2 min CN=91 Runoff=11.79 cfs 0.641 af
Subcatchment P2: Southern Subcat	Runoff Area=30,379 sf 54.47% Impervious Runoff Depth=5.00" Tc=5.0 min CN=90 Runoff=5.77 cfs 0.291 af
Subcatchment P2a: Drainage Network	Runoff Area=51,243 sf 87.19% Impervious Runoff Depth=5.69"
Flow Length=411	Slope=0.0155 '/' Tc=6.7 min CN=96 Runoff=9.70 cfs 0.557 af
Pond 3P: CB1	Peak Elev=9.57' Inflow=9.70 cfs 0.557 af
18.0" Round	Culvert n=0.013 L=25.1' S=0.0279 '/' Outflow=9.70 cfs 0.557 af

- Pond 5P: DMH 1657
 Peak Elev=14.41'
 Inflow=9.70 cfs
 0.557 af

 18.0"
 Round Culvert
 n=0.013
 L=30.0'
 S=0.0487 '/'
 Outflow=9.70 cfs
 0.557 af

 Pond 6P: DMH 1
 Peak Elev=12.85'
 Inflow=9.70 cfs
 0.557 af

 18.0"
 Round Culvert
 n=0.013
 L=22.0'
 S=0.0336 '/'
 Outflow=9.70 cfs
 0.557 af
- Pond 7P: DMH 2 18.0" Round Culvert n=0.013 L=90.0' S=0.0260 '/' Outflow=9.70 cfs 0.557 af 18.0" Round Culvert n=0.013 L=90.0' S=0.0260 '/' Outflow=9.70 cfs 0.557 af

Link DP1:

below 1,000.00 cfs Inflow=26.88 cfs 1.489 af Primary=26.88 cfs 1.489 af Secondary=0.00 cfs 0.000 af

Total Runoff Area = 3.378 ac Runoff Volume = 1.489 af Average Runoff Depth = 5.29" 35.33% Pervious = 1.193 ac 64.67% Impervious = 2.184 ac

Summary for Subcatchment P1: Northern Subcat

Runoff = 11.79 cfs @ 11.98 hrs, Volume= 0.641 af, Depth= 5.11" Routed to Link DP1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr Rainfall=6.16"

A	rea (sf)	CN	Description				
	3,570	96	Gravel surfa	ace, HSG D)		
	27,640	98	Water Surfa	ace, HSG D)		
	4,206	98	Water Surfa	ace, 0% im	o, HSG D		
	173	77	Woods, Go	od, HSG D			
	2,771	98	Roofs, HSC	6 D			
	23,639	80	>75% Gras	s cover, Go	ood, HSG D		
	3,506	98	Paved parking, HSG D				
	65,505	91	Weighted Average				
	31,588		48.22% Pervious Area				
	33,917	51.78% Impervious Area					
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
7.2	585	0.0374	1.35		Lag/CN Method,		

Summary for Subcatchment P2: Southern Subcat

- [49] Hint: Tc<2dt may require smaller dt
- Runoff = 5.77 cfs @ 11.95 hrs, Volume= 0.291 af, Depth= 5.00" Routed to Link DP1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr Rainfall=6.16"

A	rea (sf)	CN	Description					
	1,183	96	Gravel surfa	ace, HSG D)			
	12,607	98	Water Surfa	ace, HSG D)			
	3,939	98	Roofs, HSG	6 D				
	12,650	80	>75% Gras	s cover, Go	ood, HSG D			
	30,379	90	Weighted Average					
	13,833		45.53% Pervious Area					
	16,546		54.47% Impervious Area					
Та	l a ra artha	Clana	Valasity	Consolt	Description			
	Length	Slope	,	Capacity	Description			
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)				
5.0					Direct Entry,			

Summary for Subcatchment P2a: Drainage Network

Runoff	=	9.70 cfs @	11.97 hrs,	Volume=	0.557 af,	Depth=	5.69"
Routed	to Pond	1 5P : DMH 16	357				

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Type II 24-hr 25-yr Rainfall=6.16"

Area	a (sf) CN	Descriptio	on		
	,903 98	,			
	,564 80		ass cover, Go	,	
36	,776 98	3 Paved pa	rking, HSG D)	
	,243 96				
	,564	-	ervious Area		
44	,679	87.19% lr	mpervious Are	ea	
		ope Velocit ft/ft) (ft/sec		Description	
6.7	411 0.0	155 1.0	3	Lag/CN Me	thod,
[79] Warnin	ıg: Submer	ged Pond 7P	Summary Primary devi	for Pond 3	
Outflow Primary	= 9.7 = 9.7	70 cfs @ 11 70 cfs @ 11 70 cfs @ 11	.97 hrs, Volu	ime= ime=	epth = 5.69" for 25-yr event 0.557 af 0.557 af, Atten= 0%, Lag= 0.0 min 0.557 af
Routing by Peak Elev= Flood Elev=	9.57' @ 1		Span= 0.00-7	2.00 hrs, dt=	0.05 hrs
Device Ro	outing	Invert	Outlet Device	es	
	imary	7.53'	Inlet / Outlet n= 0.013 Co	PP, square ec Invert= 7.53'	dge headwall, Ke= 0.500 / 6.83' S= 0.0279 '/' Cc= 0.900 , smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=9.44 cfs @ 11.97 hrs HW=9.51' (Free Discharge) -1=Culvert (Inlet Controls 9.44 cfs @ 5.34 fps)

Summary for Pond 5P: DMH 1657

 Inflow Area =
 1.176 ac, 87.19% Impervious, Inflow Depth =
 5.69" for 25-yr event

 Inflow =
 9.70 cfs @
 11.97 hrs, Volume=
 0.557 af

 Outflow =
 9.70 cfs @
 11.97 hrs, Volume=
 0.557 af, Atten= 0%, Lag= 0.0 min

 Primary =
 9.70 cfs @
 11.97 hrs, Volume=
 0.557 af

 Routed to Pond 6P : DMH 1
 0.557 af

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Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 14.41' @ 11.97 hrs Flood Elev= 17.13'

Device	Routing	Invert	Outlet Devices
#1	Primary	12.37'	18.0" Round Culvert L= 30.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 12.37' / 10.91' S= 0.0487 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=9.44 cfs @ 11.97 hrs HW=14.35' (Free Discharge) **1=Culvert** (Inlet Controls 9.44 cfs @ 5.34 fps)

Summary for Pond 6P: DMH 1

[79] Warning: Submerged Pond 5P Primary device # 1 INLET by 0.43'

Inflow Area =	1.176 ac, 8	87.19% Impervious,	Inflow Depth = 5.6	9" for 25-yr event
Inflow =	9.70 cfs @	11.97 hrs, Volume	e= 0.557 af	-
Outflow =	9.70 cfs @	11.97 hrs, Volume	e= 0.557 af,	Atten= 0%, Lag= 0.0 min
Primary =	9.70 cfs @	11.97 hrs, Volume	e= 0.557 af	-
Routed to	Pond 7P : DMH 2			

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 12.85' @ 11.97 hrs Flood Elev= 14.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	10.81'	18.0" Round Culvert
	ŗ		L= 22.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 10.81' / 10.07' S= 0.0336 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=9.44 cfs @ 11.97 hrs HW=12.79' (Free Discharge) **1=Culvert** (Inlet Controls 9.44 cfs @ 5.34 fps)

Summary for Pond 7P: DMH 2

[79] Warning: Submerged Pond 6P Primary device # 1 INLET by 1.15'

Inflow Area = 1.176 ac, 87.19% Impervious, Inflow Depth = 5.69"for 25-yr event 9.70 cfs @ 11.97 hrs, Volume= Inflow 0.557 af = 9.70 cfs @ 11.97 hrs, Volume= 0.557 af, Atten= 0%, Lag= 0.0 min Outflow = 9.70 cfs @ 11.97 hrs, Volume= Primary = 0.557 af Routed to Pond 3P : CB1

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Peak Elev= 12.01' @ 11.97 hrs Flood Elev= 13.00' Proposed Conditions 2023-05-23 David T

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Device	Routing	Invert	Outlet Devices
#1	Primary	9.97'	18.0" Round Culvert L= 90.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 9.97' / 7.63' S= 0.0260 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf
			n= 0.013 Condgated I E, shooth interior, Thow Area = 1.77 si

Primary OutFlow Max=9.44 cfs @ 11.97 hrs HW=11.95' (Free Discharge) —1=Culvert (Inlet Controls 9.44 cfs @ 5.34 fps)

Summary for Link DP1:

Inflow Area =	3.378 ac, 64.67% Impervious, Inflow	Depth = 5.29" for 25-yr event
Inflow =	26.88 cfs @ 11.97 hrs, Volume=	1.489 af
Primary =	26.88 cfs @ 11.97 hrs, Volume=	1.489 af, Atten= 0%, Lag= 0.0 min
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Primary outflow = Inflow below 1,000.00 cfs, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

INSPECTION & LONG-TERM MAINTENANCE PLAN FOR SITE DEVELOPMENT

AMBIT ENGINEERING, INC.

35 BADGERS ISLAND WEST KITTERY, ME

Introduction

The intent of this plan is to provide Hampshire Development (herein referred to as "owner") with a list of procedures that document the inspection and maintenance requirements of the stormwater management system for this development. Specifically, the proposed closed drainage network and associated drainage structures (collectively referred to as the "Stormwater Management System"). The contact information for the owner shall be kept current, and if there is a change of ownership of the property this plan must be transferred to the new owner.

The following inspection and maintenance program is necessary to keep the stormwater management system functioning properly and will help in maintaining a high quality of stormwater runoff to minimize potential environmental impacts. By following the enclosed procedures, the owner will be able to maintain the functional design of the stormwater management system and maximize its ability to remove sediment and other contaminants from site generated stormwater runoff.

<u>Annual Report</u>

The owner shall prepare an annual Inspection & Maintenance Report. The report shall include a summary of the system's maintenance and repair by transmission of the Inspection & Maintenance Log and other information as required. A copy of the report shall be delivered annually by July 1st to the Kittery Code Enforcement Officer.

Inspection & Maintenance Checklist/Log

The following pages contain the Stormwater Management System Inspection & Maintenance Requirements and a blank copy of the Stormwater Management System Inspection & Maintenance Log. These forms are provided to the owner as a guideline for performing the inspection and maintenance of the Stormwater Management System. This is a guideline and should be periodically reviewed for conformance with current practice and standards.

Stormwater Management System Components

The Stormwater Management System is designed to mitigate the quality of site-generated stormwater runoff. As a result, the design includes the following elements:

Non-Structural BMPs

Non-Structural best management practices (BMP's) include temporary and permanent measures that typically require less labor and capital inputs and are intended to provide protection against erosion of soils. Examples of non-structural BMP's on this project include but are not limited to:

- Temporary and Permanent mulching
- Temporary and Permanent grass cover
- Trees
- Shrubs and ground covers
- Miscellaneous landscape plantings
- Dust control
- Tree protection
- Topsoiling
- Sediment barriers
- Stabilized construction entrance
- Catch basin basket

Structural BMPs

Structural BMPs are more labor and capital-intensive structures or installations that require more specialized personnel to install. Examples on this project include but are not limited to:

• Storm Drains with Deep Sumps

Inspection and Maintenance Requirements

The following summarizes the inspection and maintenance requirements for the various BMP's that may be found on this project.

- 1. Grassed areas (until established): After each rain event of 0.5" or more during a 24-hour period, inspect grassed areas for signs of disturbance, such as erosion. If damaged areas are discovered, immediately repair the damage. Repairs may include adding new topsoil, lime, seed, fertilizer and mulch.
- 2. Plantings: Planting and landscaping (trees, shrubs) shall be monitored bi-monthly during the first year to insure viability and vigorous growth. Replace dead or dying vegetation with new stock and make adjustments to the conditions that caused the dead or dying vegetation. During dryer times of the year, provide weekly watering or irrigation during the establishment period of the first year. Make the necessary adjustments to ensure long-term health of the vegetated covers, i.e. provide more permanent mulch or compost or other means of protection.

- 3. Storm Drains and Trench Drains: Monitor accumulation of debris in catch basins and trench drains monthly or after significant rain events. Remove sediments when they accumulate within the outlet pipe. During construction, maintain inlet protection until all roadways and parking areas have been stabilized. Prior to the end of construction, inspect the drains and basins for accumulations and remove and clean by jet-vacuuming.
- 4. **Roof Drains:** Monitor roof drains for damage or clogging twice yearly. Check any outlets for erosion.

Pollution Prevention

The following pollution prevention activities shall be undertaken to minimize potential impacts on stormwater runoff quality. The Contractor is responsible for all activities during construction. The Owner is responsible thereafter.

Spill Procedures

Any discharge of waste oil or other pollutant shall be reported immediately to the Maine Department of Environmental Protection (Maine DEP). The Contractor/Owner will be responsible for any incident of groundwater contamination resulting from the improper discharge of pollutants to the stormwater system, and may be required by Maine DEP to remediate incidents that may impact groundwater quality. If the property ownership is transferred, the new owner will be informed of the legal responsibilities associated with operation of the stormwater system, as indicated above.

Sanitary Facilities

Sanitary facilities shall be provided during all phases of construction.

Material Storage

No on site trash facility is provided until homes are constructed. The contractors are required to remove trash from the site. Hazardous material storage is prohibited.

Material Disposal

All waste material, trash, sediment, and debris shall be removed from the site and disposed of in accordance with applicable local, state, and federal guidelines and regulations. Removed sediments shall be if necessary dewatered prior to disposal.

Invasive Species

Monitor the Stormwater Management System for signs of invasive species growth. If caught early, their eradication is much easier. The most likely places where invasions start is in wetter, disturbed soils or detention ponds. Species such as phragmites and purple loose-strife are common invaders in these wetter areas. If they are found, the owner shall refer to the Invasive Plants List created by the Maine Department of Agriculture, Conservation & Forestry or contact a wetlands scientist with experience in invasive species control to implement a plan of action for eradication. Measures that do not require the application of chemical herbicides should be the first line of defense.



Figure 1: Lythrum salicaria, Purple Loosestrife. Photo by Liz West. Figure 2: Phragmites australis. Photo by Le Loup Gris

CATCH BASIN BASKET CONSTRUCTION MAINTENANCE SHEET

INSPECTION REQUIREMENTS		
ACTION TAKEN	FREQUENCY	MAINTENANCE REQUIREMENTS
-Check for damage to basket -Remove sediment from basket	Within 24 hours of rainfall, Daily during extended rainfall	-Repair basket as necessary to prevent particles from reaching drainage system, or to prevent flooding. -Empty basket after every storm, or if clogged.

ROJECT NAME	
SPECTOR NAME	INSPECTOR CONTACT INFO
ATE OF INSPECTION	REASON FOR INSPECTION
	LARGE STORM EVENT
CORRECTIVE ACTION NEEDED?	DESCRIBE ANY PROBLEMS, NEEDED MAINTENANCE
IYES □NO	
ATE OF MAINTENANCE	PERFORMED BY
OTES	

CLOSED DRAINAGE STRUCTURE LONG-TERM MAINTENANCE SHEET

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INSPECTION REQUIREMENTS		
ACTION TAKEN	FREQUENCY	MAINTENANCE REQUIREMENTS
-Outlet Control Structures -Drain Manholes -Catch Basins and Trench Drains	Every other Month	Check for erosion or short-circuiting Check for sediment accumulation Check for floatable contaminants
-Drainage Pipes -Roof Drains	1 time per 2 years	Check for sediment accumulation/clogging, or soiled runoff. Check for erosion at outlets,

PROJECT NAME				
DATE OF INSPECTION	REASON FOR INSPECTION			
	LARGE STORM EVENT PERIODIC CHECK-IN			
IS CORRECTIVE ACTION NEEDED?	DESCRIBE ANY PROBLEMS, NEEDED MAINTENANCE			
□YES □NO				
DATE OF MAINTENANCE	PERFORMED BY			
NOTES				
IOTES				
NOTES				

STABILIZED CONSTRUCTION ENTRANCE CONSTRUCTION MAINTENANCE SHEET

INSPECTION REQUIREMENTS			
ACTION TAKEN	FREQUENCY	MAINTENANCE REQUIREMENTS	
ENTRANCE SURFACE -Check for sediment accumulation/clogging of stone -Check Vegetative filter strips	After heavy rains, as necessary	-Top dress pad with new stone. -Replace stone completely if completely clogged. -Maintain vigorous stand of vegetation.	
WASHING FACILITIES (if applicable) -Monitor Sediment Accumulation	As often as necessary	-Remove Sediments from traps.	

PROJECT NAME		
DATE OF INSPECTION	REASON FOR INSPECTION	
	LARGE STORM EVENT PERIODIC CHECK-IN	
IS CORRECTIVE ACTION NEEDED?	DESCRIBE ANY PROBLEMS, NEEDED MAINTENANCE	
DATE OF MAINTENANCE	PERFORMED BY	
NOTES		