Wood Island Feasibility Study

April 2009

B. Seawall Assessment Appendix

Katherine Andruchuk, Seth Lizotte

Wood Island Feasibility Study

April 2009

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Wood Island	Feasibility	Study
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April 2009

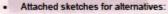
Communication Records

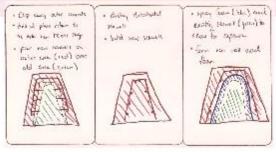
		Date:	January 20, 2009
Communication Record		Time:	4:30 pm - 5:00 pm
Meeting with:	Professor Gress		1.22
Group Members Present:	Seth Lizotte, Katie Andruchek		
Topic of Meeting:	Seawall Rehabilitation		
Type of Communication:	Office Visit		

Minutes

- Discussed the issues occurring with the seawall at Wood Island. Possible issues include effects of chemical action
 of seawater constituents on cement hydration products, alkali-aggregate expansion, crystallization pressure of
 salts within concrete if one face of the structure is subject to wetting and others to drying conditions, frost action in
 cold climates, corrosion of embedded steel in reinforced members, and physical erosion due to wave action.
- A core of the wall is needed to be cut and polished to determine the exact state of the existing seawall and in turn determine the effective rehabilitation
- Four distinct alternatives were determined, each dependant on the evaluation of the core sample (*please see attached sketches, in additional notes, for clarification):
 - Chip away deteriorated outer concrete, drill and place rebar into existing concrete to tie into newly formed rebar cage, pour new concrete around existing sound concrete, use chipped material for pathway construction.
 - Destroy existing seawall, use material for pathway construction, replace with a cast-in-place, pre-cast, or retaining wall with earthen back-fill seawall.
 - Chip away brittle outer damaged area, spray foam around existing seawall to allow for expansion due to
 existing chemical decay, form rebar cage around foam, pour new outer shell for seawall.
 - Leave structure as-is.

Cother Information Relevant Contact Information: Name: Professor David Gress Email: David.Gress@unh.edu Phone: 603. 862. 1410 Additional Notes: Need to obtain sample from Krystian and follow-up with Professor Gress to evaluate sample





CIE 788 Project Planning and Design Wood Island Life Saving Station

Communication Record		Date:	January 23, 2009
		Time:	10:30 am – 11:00 am
Meeting with:	Professor Gress		
Group Members Present:	Seth Lizotte, Katie Andruchuk		
Topic of Meeting:	Seawall Rehabilitation		
Type of Communication:	e of Communication: Office Visit		
Minutes			

- A piece of core from the seawall was shown to Professor Gress and determined that from the looks of it, it was a still a good piece of concrete and might have had help being taken off the wall seeing it's relatively large size
- . Discussed using interlocking concrete blocks as a seawall alternative and received company advertisements from a magazine in order to look further into this type of possibility
- . The piece of core from the seawall was left with Professor Gress and discussion about getting one of his students to cut the core into three sections in order to polish the pieces of seawall occurred
- . Once the core has been cut and polished the three of us will meet in order to closely look at the concrete under a microscope

Other Information

Relevant Contact Information:

Name:	Professor David Gress	
Email:	David.Gress@unh.edu	
Phone:	603. 862. 1410	

Additional Notes:

- Received email from Professor Gress following the meeting to check out WWW.stonestrong.com and www.tensarcorp.com/ss9
- January 27th, was informed by one of Professor Gress' students that the core has been cut and is ready to be polished

CIE 788 Project Planning and Design Wood Island Life Saving Station

Meeting with: Group Members Pro Topic of Meeting:	Professor Gress Seth Lizotte, Katie Andruchuk Seawall Rehabilitation			
Fopic of Meeting:	<u> </u>			
Type of Communication				
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determine th It was deter concrete wa which indica Reaction tak To determin then radioac glow a brigh ASR, though Alternatives, alternative w series of stir needed for tharrier, woul Further rese	of seawall were cut and polished by Katie and e extent of weathering on the samples. In the wall structure. Also, a tes carbonation. Dark rings around the aggreging place which is detrimental to the structure ethe extent of ASR within the samples, a same tive uranyl acetate was added. Under Ultravict green, indicating ASR was present and activities and the company of the fractured areas of the serups and then cast at least 4 inches of new come rehabilitation, under the assumption that the determinement of the serups and the cast at least 4 inches of new come rehabilitation, under the assumption that the determinement of the serups and the cast at least 4 inches of new come rehabilitation, under the assumption that the determinement of the serups and the done to determine the extent of determined the extent of determined the serups and the done to determine the extent of determined the serups and the done to determine the extent of determined the serups and the done to determine the extent of determined the serups and the done to determine the extent of determined the serups and the done to determine the extent of determined the serups and the done to determine the extent of determined the serups and the done to determine the extent of determined the serups and the determined the serups are serups and the serups are serups and the serups are serups and the serups are serups at least 4 inches of the serups are serups and the serups are ser	acking in the samples brownish haze is seen ate within the sample long term. ple was fractured to o plet Lighting, the radio. e. The samples we extend and it seems that the teawall, and encase the norete onto the surface is seawall is not in even Concrete Institute granages to the samples awall.	parallel to the face of the nat the face of the samples indicate Alkali-Silica btain an untouched face and active uranyl acetate would amined did show signs of the cheapest and easiest existing structure with a le. Also, the amount of steel ryday use as a protective uidelines.	
Name: Profes	sor David Gress	X /887.55		
Email: David.	Gress@unh.edu			
Phone: 603. 8	32. 1410			

CIE 788 Project Planning and Design Wood Island Life Saving Station

Communication Record		Date:	February 10, 2009
		Time:	1:30 pm – 1:50 pm
Meeting with:	Professor Gress		
Group Members Present:	Seth Lizotte, Katie Andruchuk		
Topic of Meeting:	Seawall Rehabilitation		
Type of Communication:	Phone Conversation		
	Minutes		

Minutes

- Since laboratory testing is complete and results found, the next step for looking into capping the existing structure would be to develop a mix design for the proposed cap
- Upon talking to Professor Gress, various specifications pertinent to a cap were needed, such as:
 - · Greater or equal to 4000 psi strength
 - . Less than or equal to 0.4 w/c ratio
 - High quality Type II cement paste (sulfate exposure)
 - %" maximum aggregate size or 3/8" maximum aggregate size for a pump mix if needed (ASTM 33 reference and Appendices in EB001 for ASR)
 - · High range water reducer if designed slump needed to increase for placement
- . It was recommended to look through EB001: Portland Cement Association: Design and Control of Concrete Mixtures, to arrive at further specifications needed to be met for the capping process
- . The idea of placing riprap from crushed seawall in a cage as a means for wave energy dissipation was discussed and will deem reason to look into

Other Information Relevant Contact Information: Professor David Gress Name: Email: David.Gress@unh.edu 603. 862. 1410 Phone: **Additional Notes:**

CIE 788

Project Planning and Design

Communication Record		Date:	February 19, 2009
		Time:	5:30 pm – 6:00 pm
Meeting with:	Redi-Rock ® Representative		
Group Members Present:	Seth Lizotte, Katie Andruchuk		
Topic of Meeting:	Precast Alternatives for Seawall Rehabilitation		
Type of Communication: Conversation with Redi-Rock ® Representative			
Minutes			

- · Received a product catalog with information pertaining to the various alternatives which we could use
- Representative said due to environmental conditions present at the site, it would be his recommendation that the product used would be the 2400 lb "Big Block."
 - The "Big Block" is 18" high, 46" wide, and 36" deep
 - This would require 3'- 4' of porous fill (gravel and pea stone) behind it to serve as a means for water to not build up behind the wall and pose further freeze-thaw action
- During the conversation, he called an individual out of his office and calculated that for materials alone (delivery and placement of "Big Blocks, "excluding the footings, demolition of the existing wall, and porous backfill), it would cost approximately \$210,000 for a 6' tall, 420' wall to be constructed

Other Information

Relevant Contact Information:

Company Name:	Redi-Rock: a Carroll Company	700
Website:	www.REDIROCK.com	
Phone:	603. 863.1000	

Additional Notes:

Due to the cost of the Redi-Rock ® alternative, it was decided to not look into that system further as a means
for the seawall rehabilitation of Wood Island.

CIE 788 Project Planning and Design

Communication Record Meeting with: Robert G. Armando, President of Tectonics, Inc. Group Members Present: Seth Lizotte Topic of Meeting: Seawall Rehabilitation Type of Communication: Email Conversations Minutes

. The following are excerpts from emails between Robert Armando and myself:

"I surveyed the project using Google Earth Pro and was able to measure the existing wall at 600 LF - if your 420 LF stated plans the exclusion of certain parts of the existing wall, I wouldn't recommend that - the ideal concrete installation would include a poured n place base slab pinned to the rock and a precast superstructure bolted together. All operations can be done using amphibious barges.

I think you have some data on the rock depth and composition in your report - if not, I would need that. Some pictures of the existing facility and the seawall would also be helpful.

Also the new structure could be installed in front of the existing wall - the existing could remain, but be knocked down to below the new back of wall grade saving the cost of demolition and removal of material

After I review your study, I will send you some drawings and a 3-d model - download Sketchup 7 Free to be able to view my files. I will also do a conceptual cost estimate after I make some calls to suppliers and potential subs in the area for pricing."

"Got your report - the REDI-Rock system will be about \$500per LF of wall - figuring a 6 foot height which is 4 layers of Redi-Block This would amount to approximately 1/3 of the final cost. The demolition, anchored base slab, tie backs and fill in place would be the other 2/3 and the Means and Methods selected will make or break the project.

The cost to do the entire project right with a lasting value would be about \$650,000 - if you do the entire 600 foot perimeter area to preserve the integrity of the wall system the cost would be about \$900,000. This very preliminary cost estimate is using a poured in place base slab and a precast superstructure.

Cost can be a function of the weather and tidal surges - work would have to be performed at low tide cycles using amphibious barges.

I would carry a 15% contingency to cover possibility of extraordinary weather and the possibility of storm damage he the system while under construction.

CIE 788

Project Planning and Design

Communication Record		Date:	March 23, 2009
		Time:	3:30 – 4:00pm
Meeting with:	Robert G. Armando, President of Tectonics, Inc.		
Group Members Present:	Seth Lizotte		
Topic of Meeting:	Seawall Rehabilitation		
Type of Communication: Phone Conversation			
Minutes			

- \$1500 per linear foot to cast in place footing with tie anchors and bedrock grinding, place
 precast seawall, and grind existing seawall as backfill material behind new wall
- Footing will consist of an 18" thick concrete slab with the precast wall sitting 8" back from front of slab which will dissipate energy from breaking waves onto wall
- The footing will be placed partially into the existing bedrock and the bedrock will be of a
 constant elevation with one or two steps for each face (north and south)
- Recommended to do entire 600' length of wall to provide complete protection and will in turn
 have the seawall last up to 50 year design life
- Weepholes will be cast every 8' along the footing to allow for drainage. Perforated pipe will lie
 behind the wall and direct water towards the weepholes, with filter-fabric covering it and then
 covered with backfill of the demoed wall to allow for proper drainage.
- The footing, having been placed into the bedrock, will withstand a great amount of undermining forces, solving the existing problem
- · Work can only be done in low tide and done with an amphibious barge
- 1" to 1 ½" tiebolts will be used every 8' and made of high strength coil bolt inserts with steel
 plates which will tie into buttresses and into slab, which tie into the rebar present in the footing
- Recommended 4000-5000 psi concrete for footing and precast seawall
- Grinder to be used for bedrock and existing seawall for use as backfill material and should be done prior to placement of footing
- · Look into mobile-mix concrete trucks on an amphibious barge for footing placement

Other Information	
Relevant Contact Information:	
Name:	Robert G. Armando
Email:	rga@tectonicsystems.com
Phone:	631-220-1098
Additional Notes:	

CIE 788

Project Planning and Design

Communication Record		Date:	April 23, 2009	
		Time:	12:00 – 12:30pm	
Meeting with:	Robert G. Armando, President of Tectonics, Inc.			
Group Members Present:	Seth Lizotte			
Topic of Meeting:	Seawall Rehabilitation			
Type of Communication:	Phone Conversation			
Minutes				

Cast in Place Seawall

- · Would need a battered seawall
- . 36" width at bottom tapering to 12" width at top, 72" height
- · Formwork needs to be heavily barricaded to prevent blowout
- Would need C or U shapped tie reinforcing bar
- · Foundation in the same fashion and size as the precast alternative
- · Batter helps to resist wave force moment
- Cast in place concrete including formwork and reinforcing bar would be approximately \$1000
 per cubic yard, includes use of water proof cement
- Would need a 40-50 ton amphibious barge to transport 6 ft wide grinder for existing wall and then to transport concrete trucks
- Tie anchors would be grouted into bedrock and stick up at least 18" above foundation
- Would have flexible construction joints every 10-15 ft with water seal
- Epoxy coated reinforcing bar to be used
- 100 +/- year design life

Demolition

- Grinder cost about \$3000 per day
- Demolition and grinding would take about 5 work days
- Would leave material on site and in place as gravel since transporting off-site would be very
 expensive relative to grinder cost
- Mobilization/ Demobilization cost \$1000
- · Would need to torch off tie backs as they are found to prevent grinder damage
- Could use material for sustainable pathway material

Other Information				
Relevant Contact Information:				
Name:	Robert G. Armando			
Email:	roa@tectonicsystems.com			
Phone:	631-220-1098			
Additional Notes:				

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